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## Evaluation of relative density effects on liquefiable sands using PM4Sand model

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To simulate the behaviour of saturated sands under cyclic loading, the PM4Sand constitutive model (version 3.1) formulated by Boulanger & Ziotopoulou [1], is used. The model can realistically reproduce the pore pressure build-up, accumulation of strain as well as triggering of liquefaction. The effect of different relative densities on liquefaction resistance is evaluated by comparing the results of a site response analysis performed on a soil column characterized by a saturated sand layer and subjected to given earthquake signals. The analyses are performed using the finite element code PLAXIS.

## Aim

To show the role that the relative density in the PM4Sand model plays on the liquefaction potential, through:

- cyclic DSS tests using a single stress point constitutive driver;
- finite element-based one-dimensional site response analyses.

### The constitutive model

The PM4Sand model is an elasto-plastic, stress-ratio-controlled, critical state compatible, bounding surface plasticity model, based on the Dafalias -Manzari model [2]. The PM4Sand model has become available in the geotechnical finite element software PLAXIS 2D [3].

#### Characteristics:

- 4 surfaces: the yield, bounding, dilation and critical state surface;
- current state defined by  $\xi_R$  (relative state parameter index equal to  $D_R$   $D_{R, CS}$ ), evolving with the mean effective stress and/or void ratio;

# Effect of the relative density in a one-dimensional site response analysis

- soil column with tied degrees of freedom as lateral boundary condition;
- 3 different outcrop motions (Loma Prieta, Kalamata and Northridge earthquakes):  $a_{max} = 0.3g$ , different frequency content and duration.

#### **Results and conclusion**

- the model is capable of accumulating shear strains and excess pore pressures while the effective vertical stress tends to zero, leading to liquefaction after 15 stress-controlled loading cycles (Figure 1);
- generated butterfly shape during final loops, when the stress state moves up and down along the failure envelope (Figure 1);
- the cyclic resistance ratio increases with increasing  $D_R$  (Figure 1);
- the loose and medium loose sands liquefy in the case of all three selected earthquakes (Figure 2);
- primary parameters to be calibrated are the shear modulus coefficient  $G_0$ , the relative density  $D_R$ , the contraction rate parameter  $h_{p0}$ ;
- default values are assumed for the secondary parameters.

## Effect of the relative density on Cyclic DSS tests

- undrained stress-controlled cyclic DSS tests;
- anisotropic consolidation with  $K_0$  equal to 0.5;
- initial vertical stress  $\sigma'_{v}$  equal to 100 kPa;
- different shear stress amplitude for different  $D_{\sf R}$  (Table 1), equal to the value that triggers liquefaction at 3% shear strain, applying 15 uniform cycles.

D <sub>R</sub>	(N <sub>1</sub> ) <sub>60</sub>	$G_0$	CRR <sub>M=7.5</sub> , 1atm	h <sub>p0</sub>
0.35	6	476	0.090	0.53
0.55	14	677	0.147	0.40
0.75	26	890	0.312	0.64

Table 1. PM4Sand primary parameters: values and cyclic resistance ratios for different  $D_R$ .



- the evolution of the pore pressure ratio, r<sub>u</sub>, shows that the onset of liquefaction occurs at different times based on the characteristics of the earthquake, but it seems to be independent from the relative density of the saturated sand (Figure 3);
- the dense sand does not liquefy: after an initial increase of r<sub>u</sub>, the excess pore pressures are partially dissipated (Figures 2 and 3).



Figure 2 - Maximum pore pressure ratio contours for Loma Prieta earthquake (a, d, g), Kalamata earthquake (b, e, h) and Northridge earthquake (c, f, i) for different relative densities.



Figure 3 - Comparison of the pore pressure ratio evolution for loose (left) and medium loose (center) and dense (right) sand, for different earthquake recordings.

Figure 1 - Results from cyclic DSS test for 3 different relative densities. Shear stress vs. shear strain (a), pore pressure vs. shear strain (b), shear stress vs. vertical effective stress (c).

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