

## P4 - PRESENTATION

T T E

# TALL TIMBER EXTENSION

*Design study for a new construction method  
in the city of Rotterdam*

### Student

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### Mentors

Structural Design | Karel Terwel

Computational Design | Michela Turrin

External supervisor | IMd Raadgevende Ingenieurs

# Structural extensions on existing buildings

# Lateral stability systems for tall structures

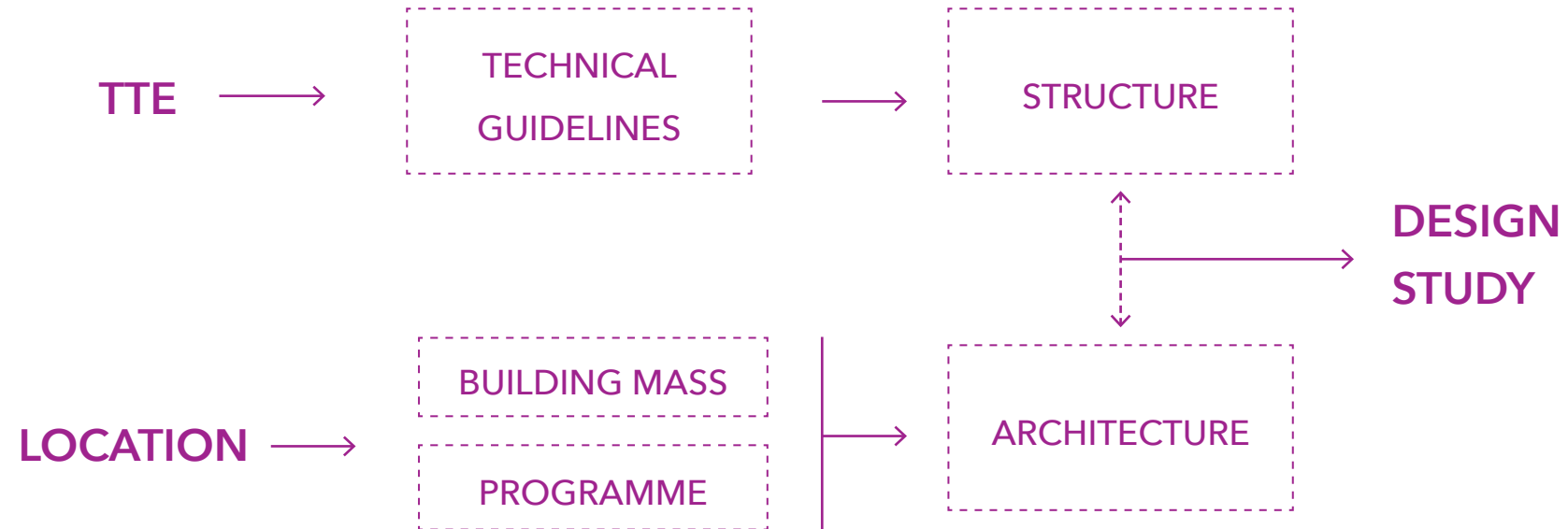
# Engineered timber products

# Sustainable structures

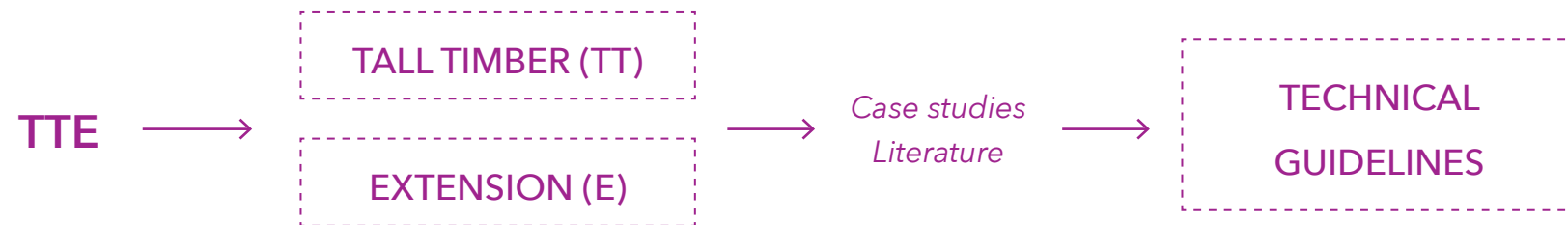
**HOW CAN WE DESIGN AN INTEGRATED TALL TIMBER  
EXTENSION (TTE) IN THE INTENDED LOCATION?**



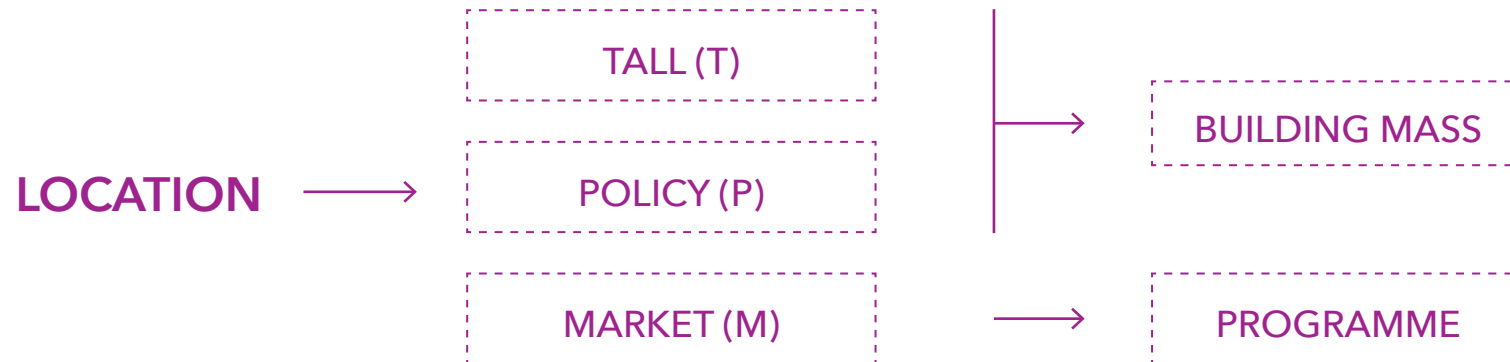
## PART 0 - RESEARCH FRAMEWORK



PART 1



## PART 2



PART 3

DESIGN STUDY



FLOOR

GRAVITY

LATERAL

EXISTING

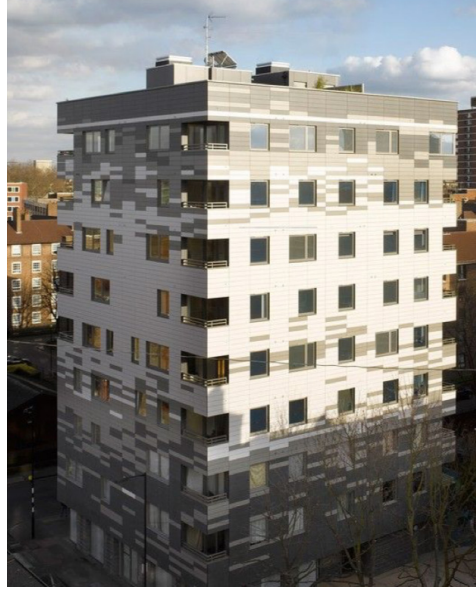


GUIDELINES

## TECHNICAL GUIDELINES

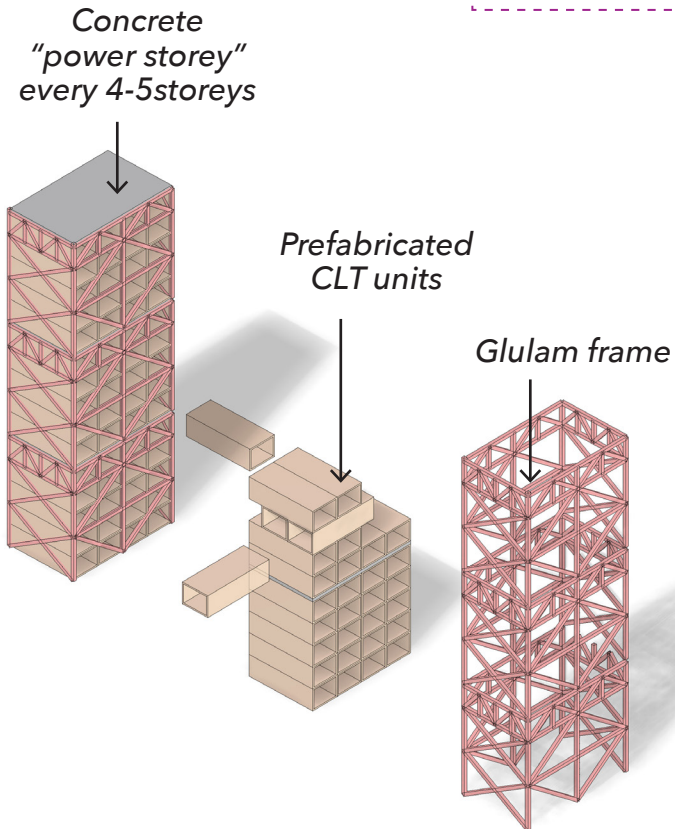
TALL TIMBER (TT)

**TO WHAT EXTENT IS A TALL TIMBER BUILDING (TT)  
TECHNICALLY FEASIBLE?**

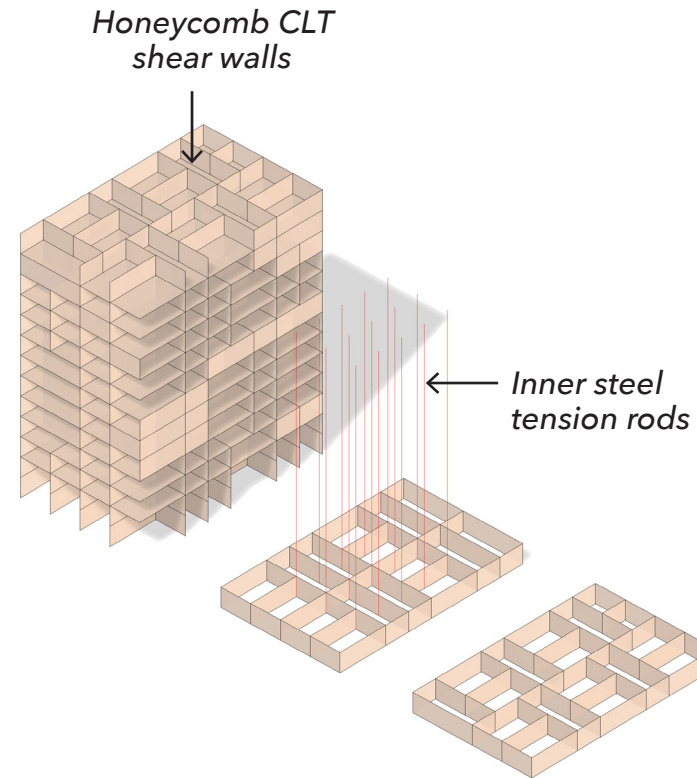




## ALL-TIMBER



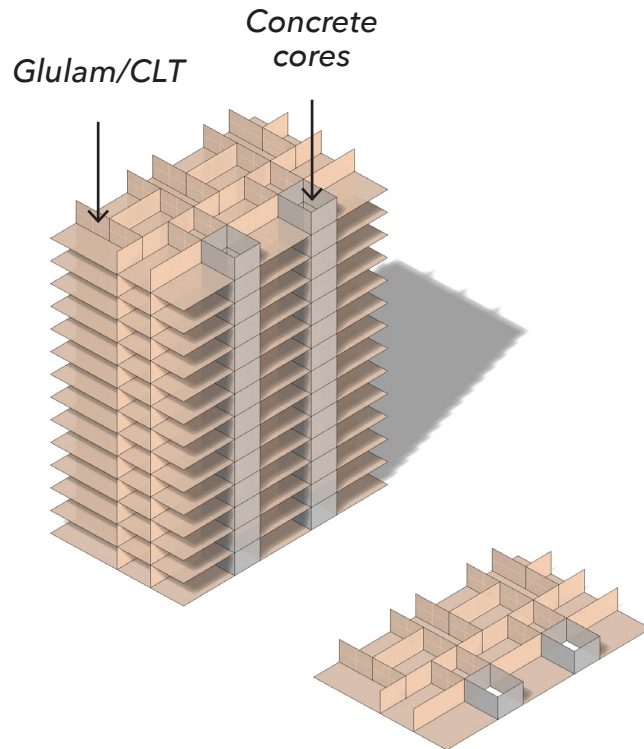
Precedent height: 53 meters  
 Lateral stability: Braced glulam frame  
 Vertical loading: CLT modules  
 Reinforcement: Concrete power storey  
 Example: The Treet (Norway)



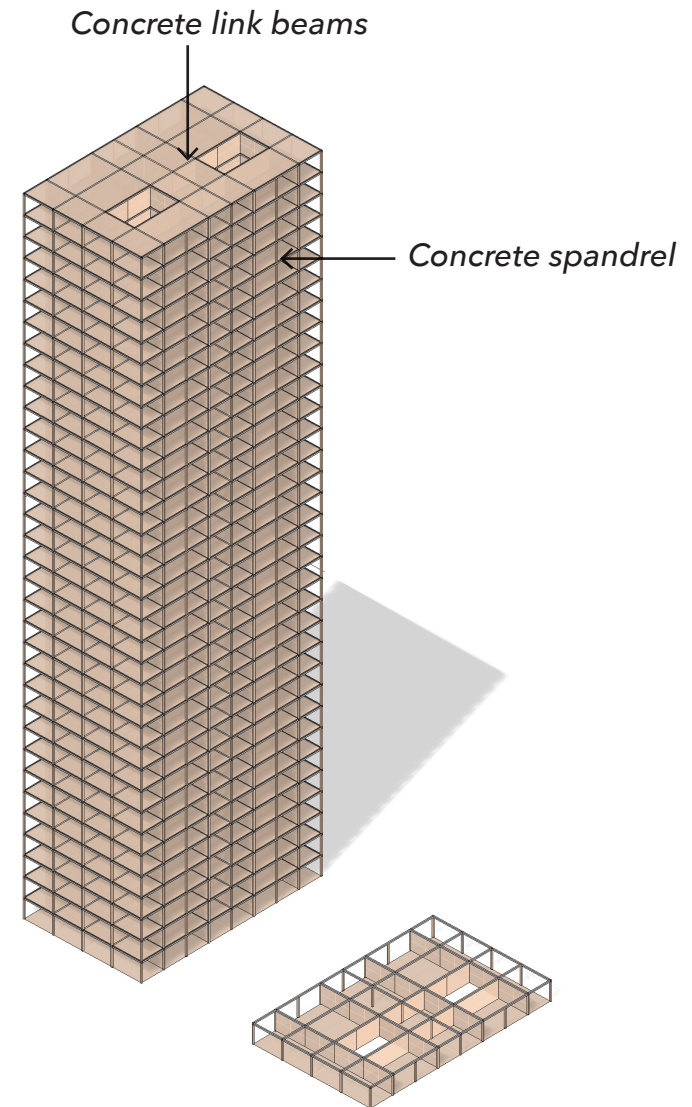
Precedent height: 32.2 meters  
 Lateral stability: CLT shear walls  
 Vertical loading: CLT walls  
 Reinforcement: Steel vertical tension rods or shear plates  
 Examples: Forte (Australia), Murray Groove (UK)



## TIMBER-CONCRETE



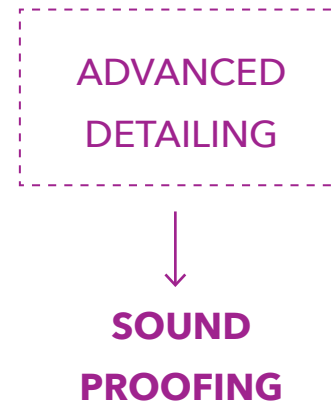
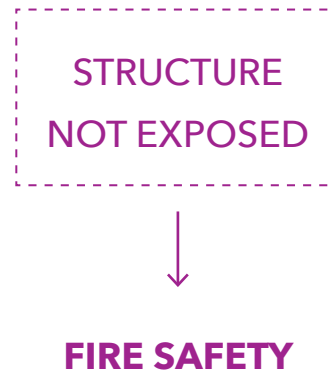
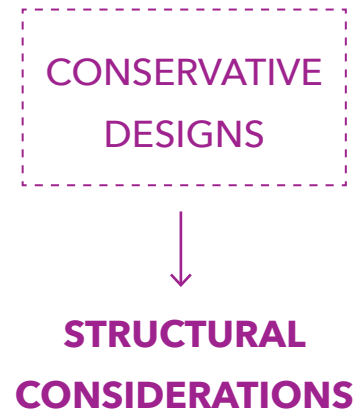
*Precedent height: 52.8 meters*  
*Lateral stability: Concrete cores*  
*Vertical loading: CLT walls/columns*  
*Example: UBC Brock Common (Canada)*



*Precedent height: 120 meters*  
*Lateral stability: CLT shear walls and outrigger glulam columns*  
*Vertical loading: CLT walls/columns*  
*Reinforcement: Concrete spandrel and link beams*  
*Example: Concept study by SOM (unbuilt)*

MARKETING

TALLEST - 53 M  
MOST SUSTAINABLE



# STRUCTURAL CONSIDERATIONS



LATERAL RESISTING SYSTEM  
DETAILING



GUIDELINES

UPLIFT

## LIGHTWEIGHT BUILDING

TIMBER

140KG/m<sup>3</sup>

THE TREET (MALO ET AL. 2016)

STEEL

160KG/m<sup>3</sup>

(CHO ET AL. 2004, YANG ET AL. 2004, HUANG ET AL. 2007)

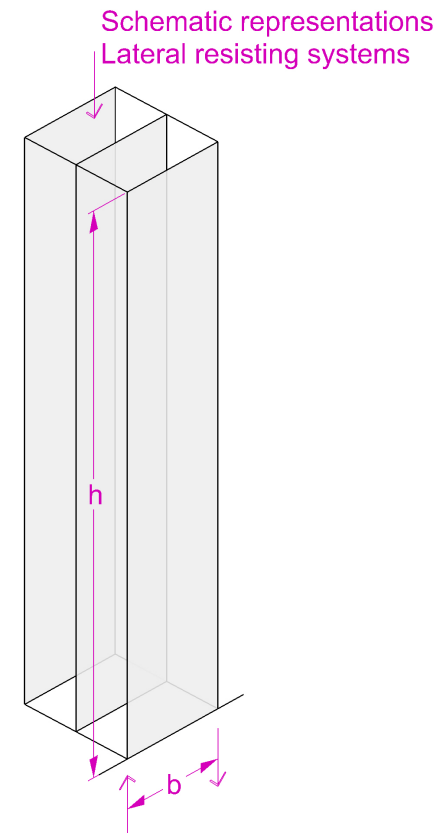
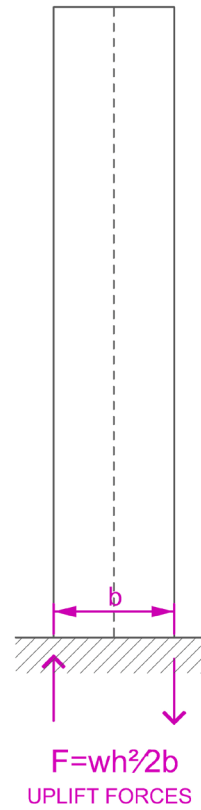
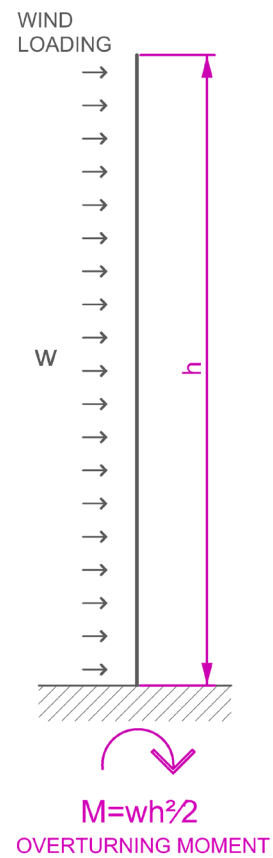
CONCRETE

300KG/m<sup>3</sup>

Estimated bulk density for a typical tall building  
(dead load by divided by gross volume)

## UPLIFT

HOWEVER, WIND LOADING REMAINS THE SAME

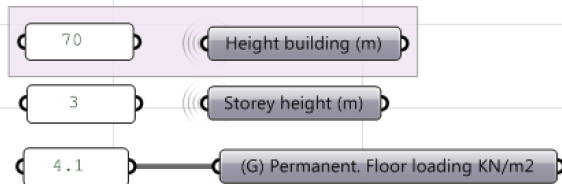


## AVOID UPLIFT

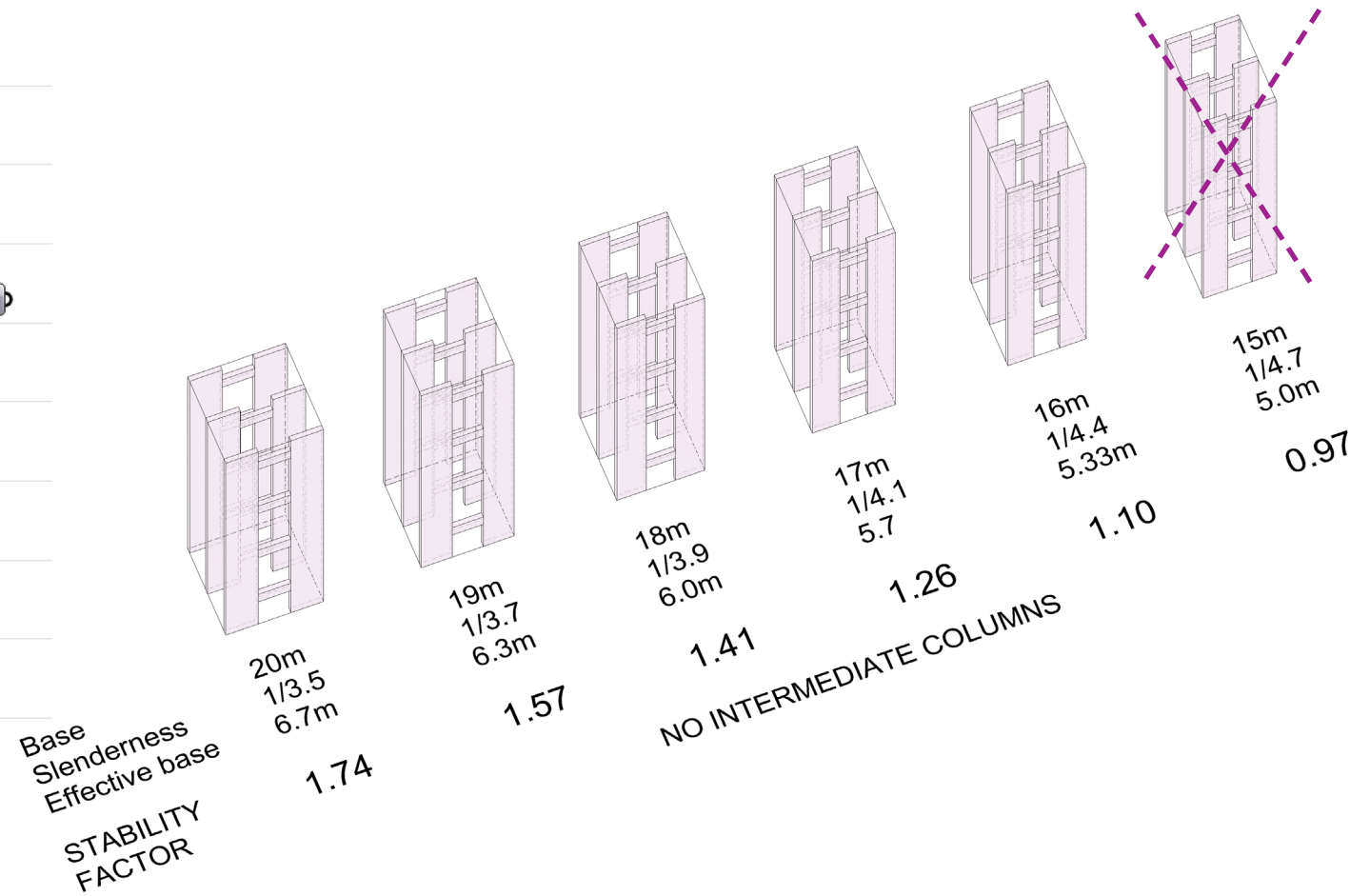
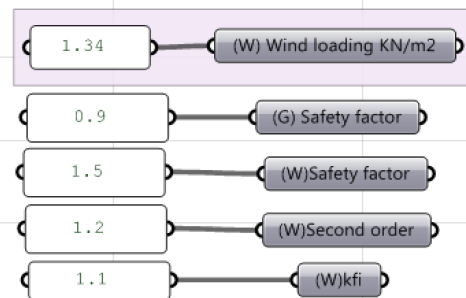
BASE DEPTH > 16 M

SLENDER RATIO < 1/4.4

### Constraints geometry



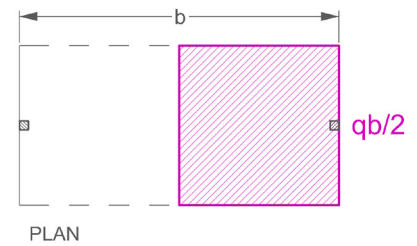
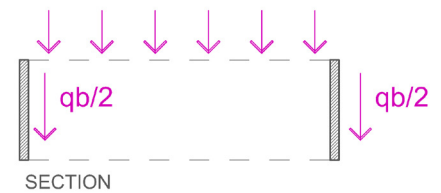
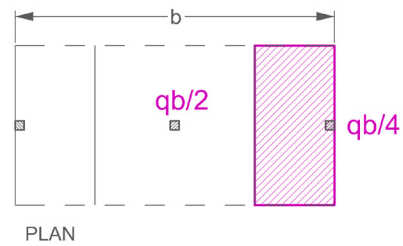
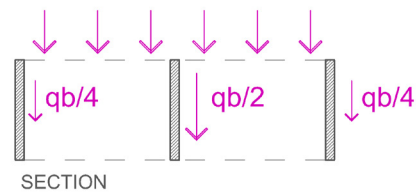
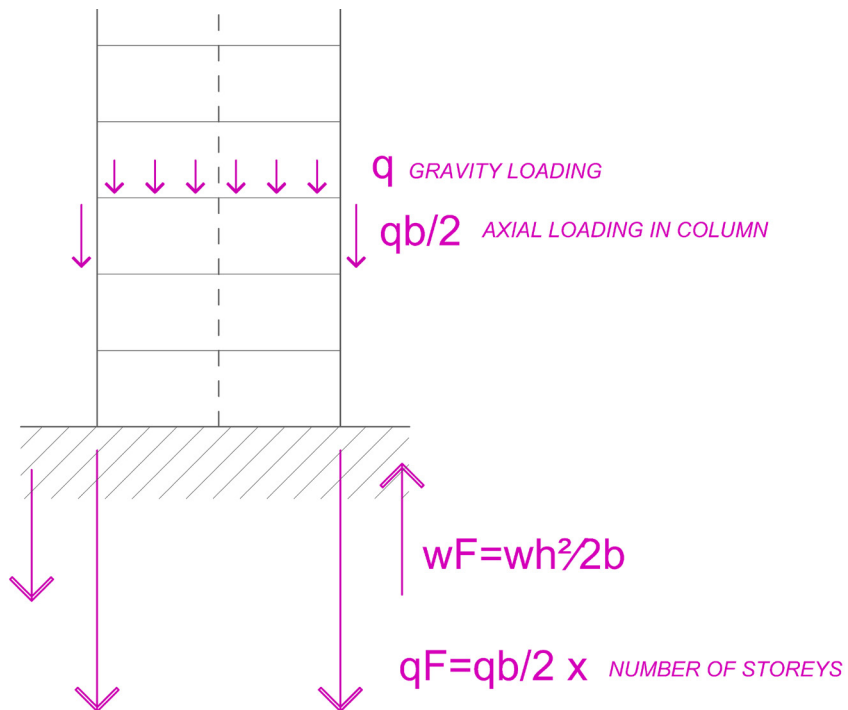
### Constraints calculations



## UPLIFT

MAXIMISE GRAVITY LOADING ON LATERAL RESISTING SYSTEM

NO INNER GRAVITY SYSTEM



CLT FLOOR SYSTEM  $< 6\text{m}$

(vibration controlled)

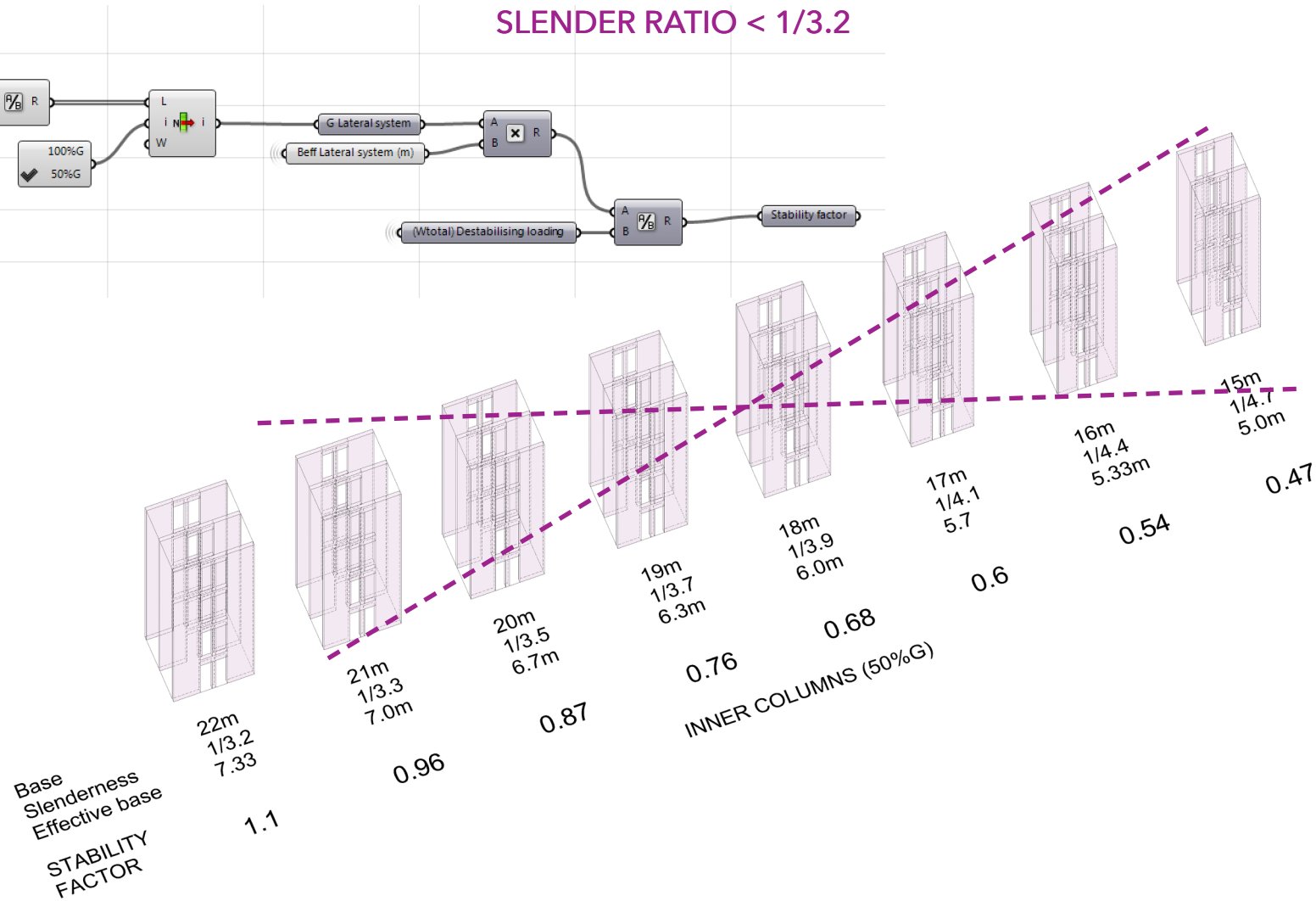


UPLIFT

INNER GRAVITY SYSTEM (50%G)

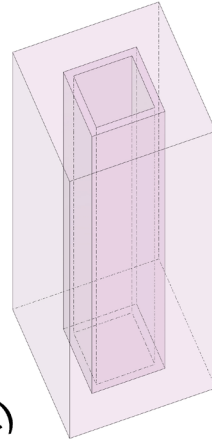
BASE DEPTH > 22 M

SLENDER RATIO < 1/3.2



UPLIFT

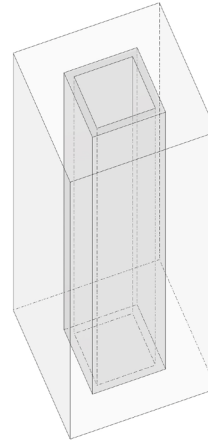
Base building  
20m (Constraint)



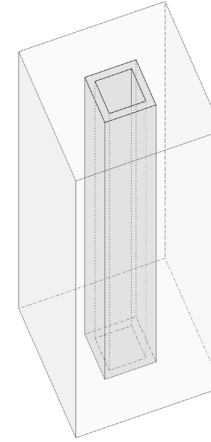
Effective base  
Weight

STABILITY

TIMBER  
5m  
140kg/m<sup>3</sup>  
1.25



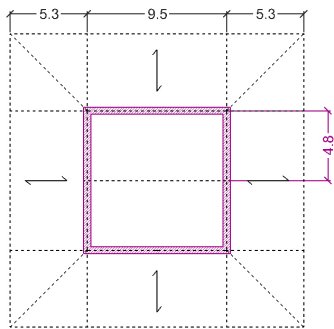
CONCRETE  
5m  
300kg/m<sup>3</sup>  
2.7



CONCRETE  
2.5m  
300kg/m<sup>3</sup>  
1.25

## UPLIFT

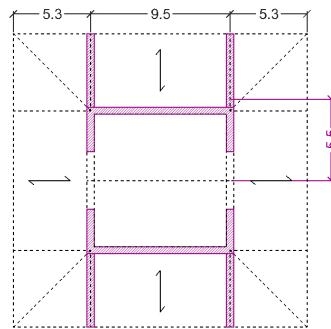
Up to 90% MORE EFFICIENT, engaging perimeter with wing shear walls



Beff = 4.8 m

1.25

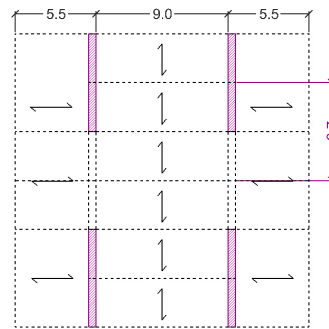
Core



Beff = 5.5 m

1.44

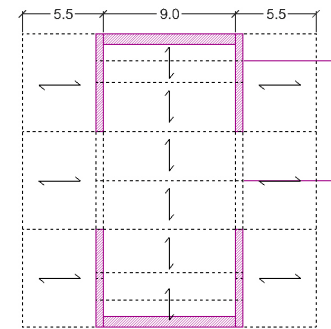
Double H



Beff = 6.7 m

1.74

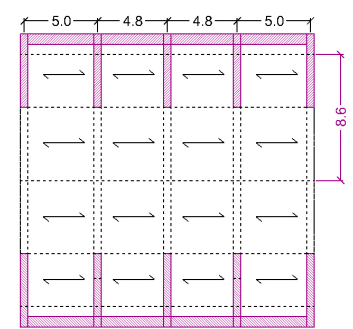
Coupled shear walls



Beff = 8.2 m

2.14

Double C



Beff = 8.6 m

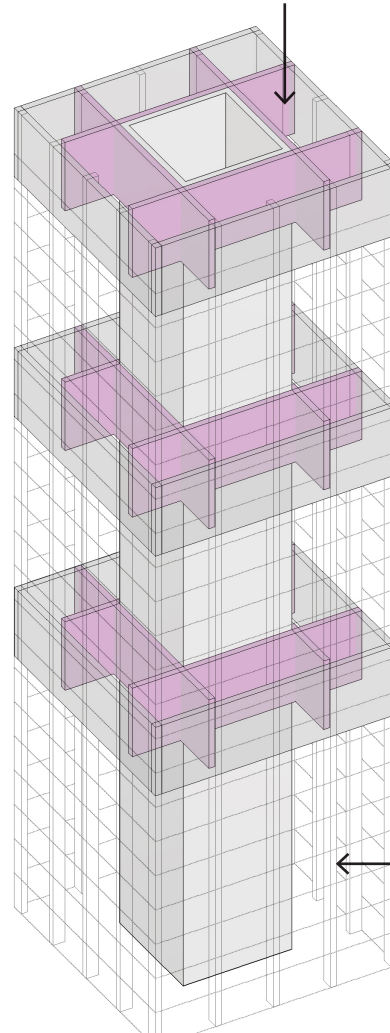
2.24

Double spine

UPLIFT

## OUTRIGGERS

*Concrete/CLT outrigger*

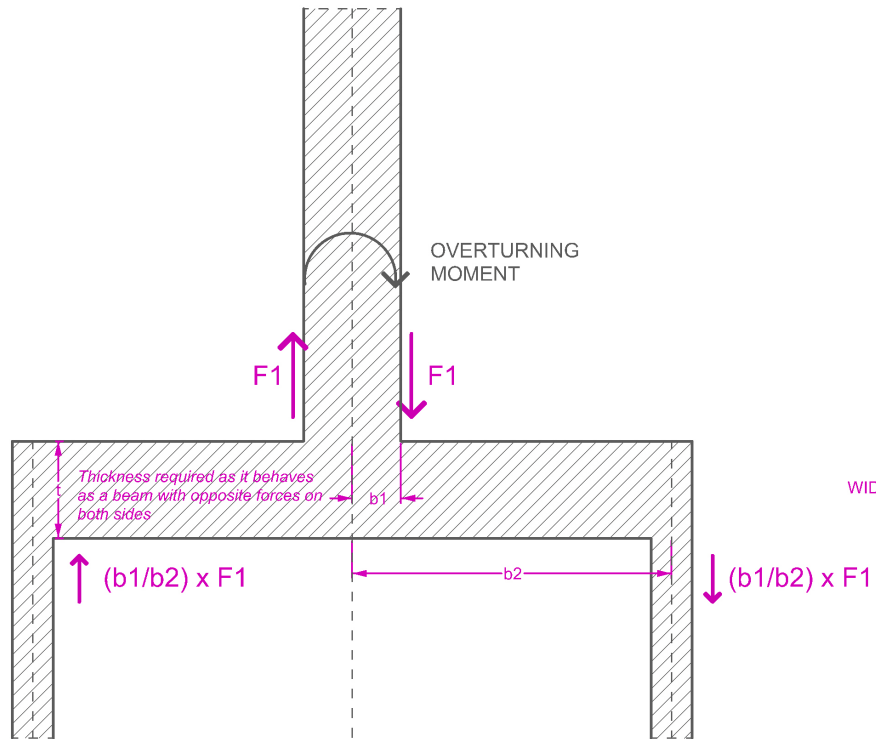


*Glulam columns  
CLT walls contribute to  
lateral resisting system*

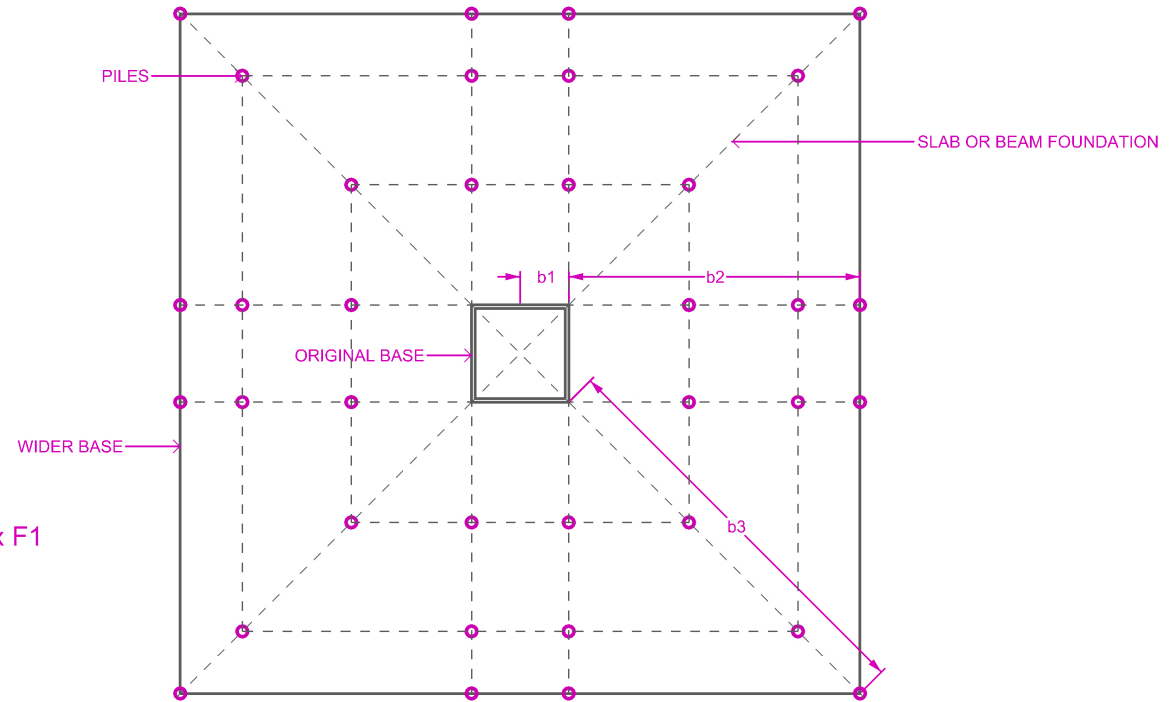
## UPLIFT IN FOUNDATION

### INCREASE FOUNDATION BASE

Grade beams or foundation slab



FOUNDATION SECTION

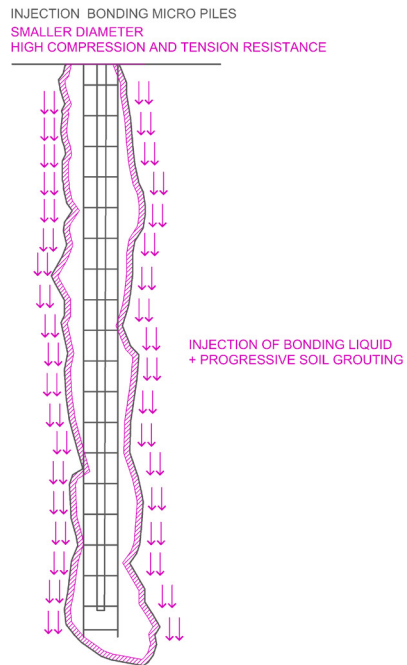
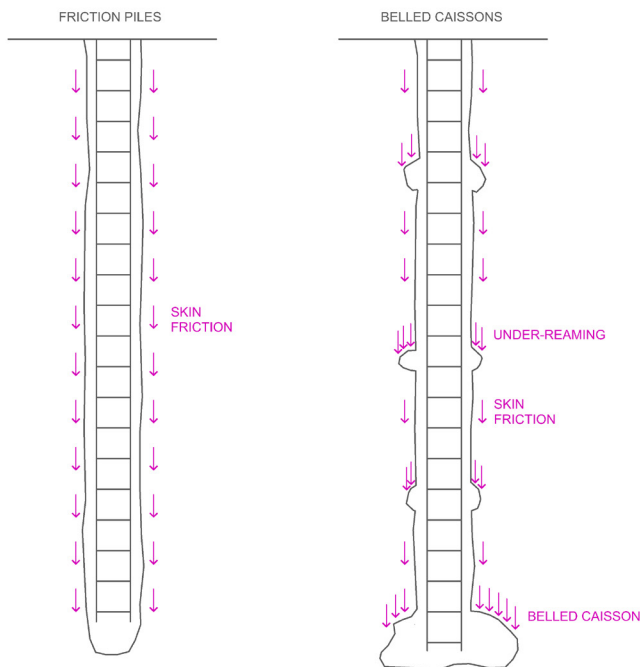


FOUNDATION PLAN

## UPLIFT IN FOUNDATION

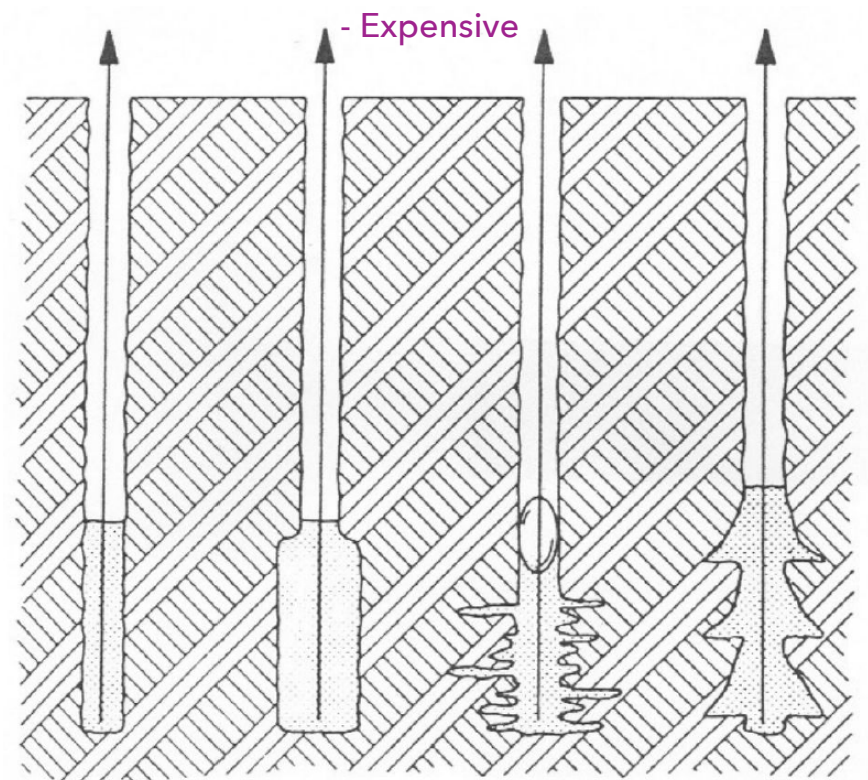
### FRICTION PILES

- Limited tension capacity (depending on the soil)
- Noise and vibrations
- Cheap



### MICRO PILES

- High tension capacity
- Low disturbances
- Expensive



UPLIFT

INCREASE  
EFFECTIVE BASE



**WING SHEAR WALLS  
OUTRIGGERS**

CONCENTRATE  
VERTICAL LOADING



**NO INNER  
COLUMNS**

INCREASE  
DEAD LOAD



**CONCRETE  
FLOOR**

CONTROL  
TENSION



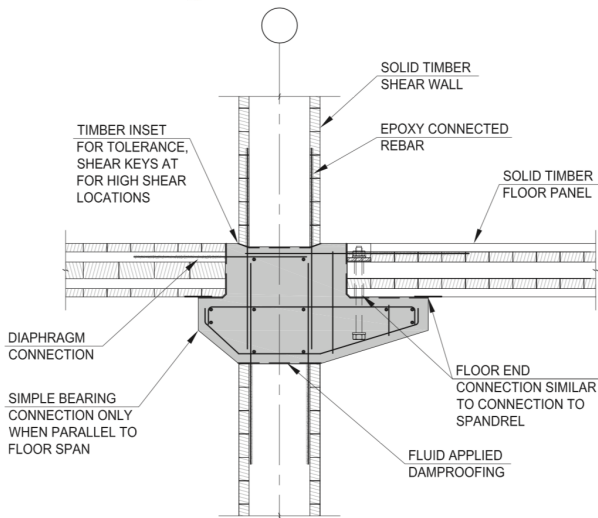
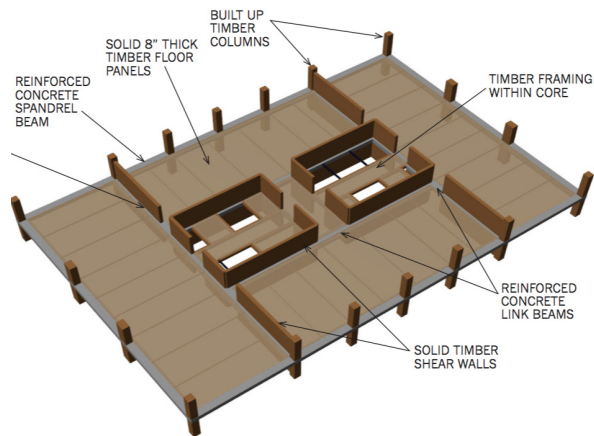
**STEEL REINFORCEMENT  
MICRO-PILES**

## COUPLING ELEMENTS

### FURTHER RESEARCH

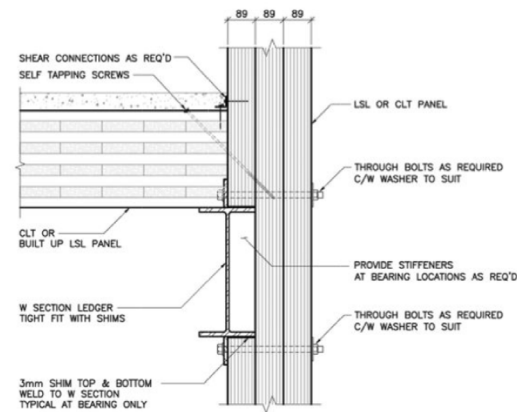
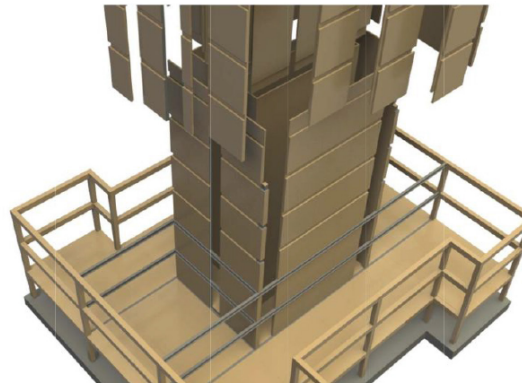
#### CONCRETE BEAM

(SOM, 2013)



#### STEEL BEAM

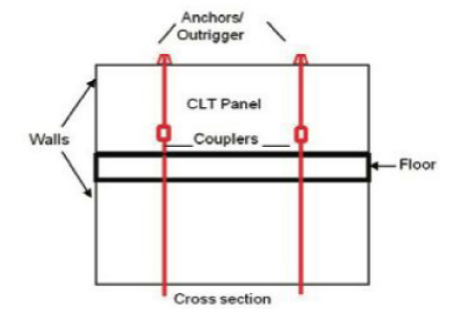
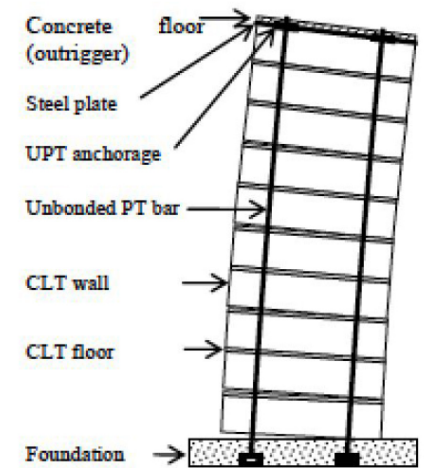
(MICHAEL GREEN, 2013)



#### POST-TENSIONED

#### CABLE INSIDE CLT

(VAN DE KUILLEN, 2010)

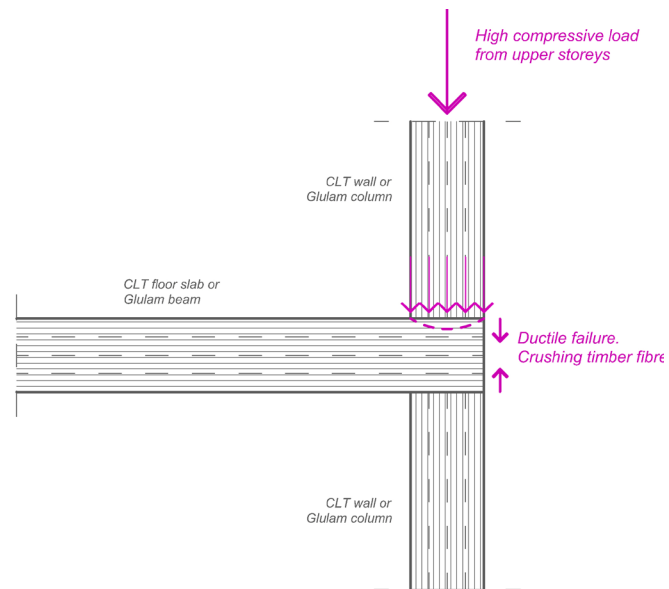




## DETAILING

### WEAK PERPENDICULAR TO FIBRE

#### PLATFORM METHOD

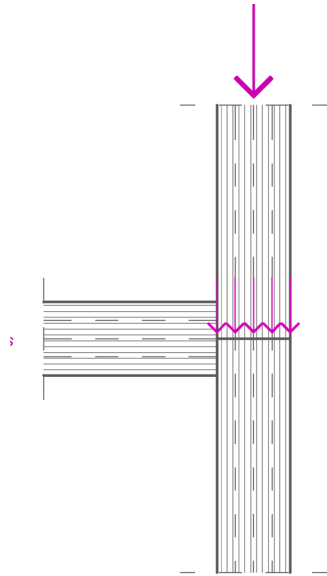


COMPRESSION PERPENDICULAR TO GRAIN

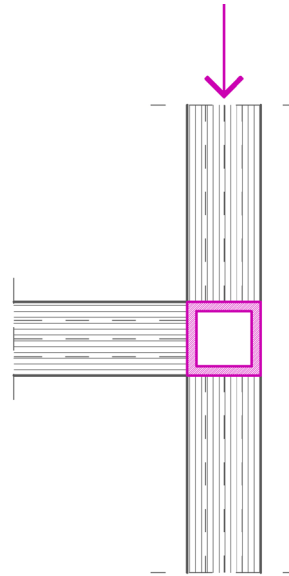
LIMITED TO 10 STOREYS

## DETAILING

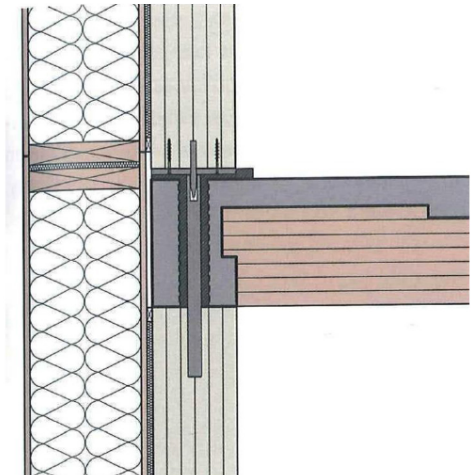
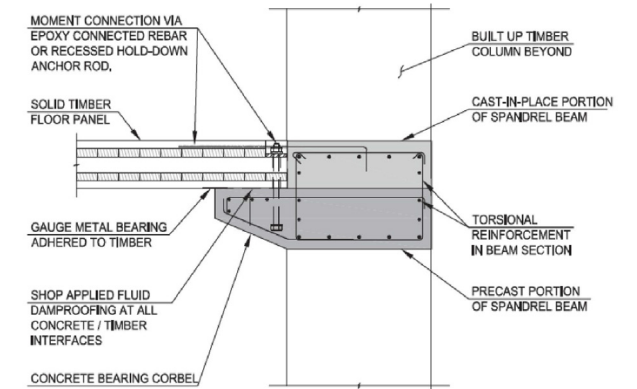
### VERTICAL CONTINUITY



### STEEL



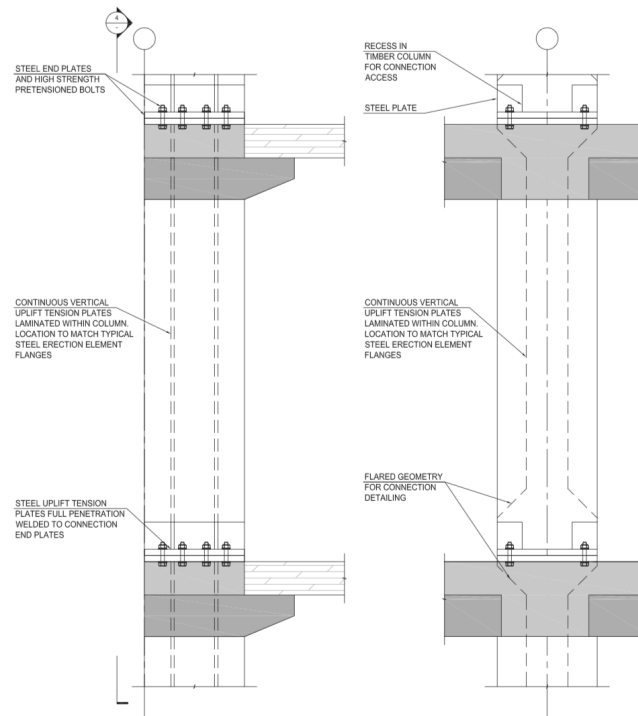
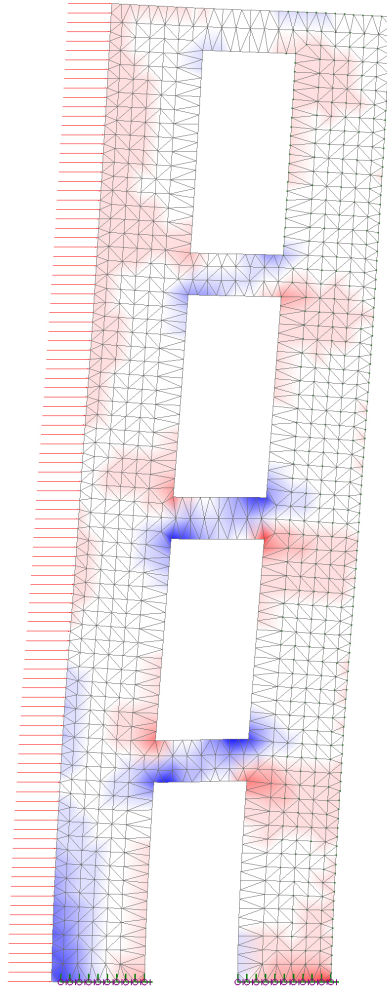
### CONCRETE



## TENSION AT THE BOTTOM

Timber has a fragile failure in tension

Reinforcement with Steel plates/rods inside timber elements



### *Steel tension rods inside CLT panels*

The use of steel tension rods inside CLT shear walls connecting the elements vertically from foundation to top can help in resisting wind-loading and uplift forces and anchoring the building to the concrete base, by absorbing tension forces. In addition, the use of these vertical rods eliminates the need of more complex load-transferring connectors between wall elements.

## FURTHER RESEARCH

WIND-INDUCED VIBRATIONS

DIFFERENTIAL SHORTENING



FULL-SCALE MOCK UP TESTS

(Above 10 storeys)

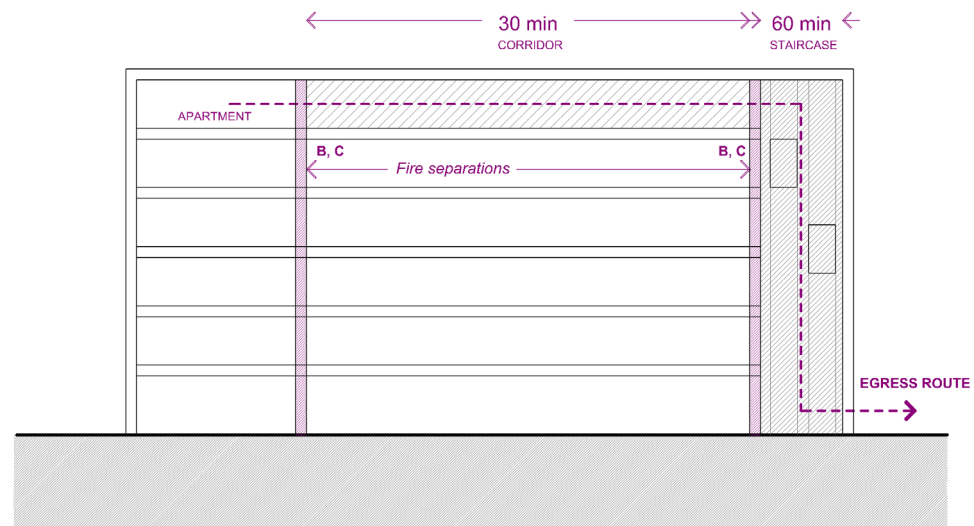
## FIRE RESISTANCE

(For buildings higher than 13m in The Netherlands)

STRUCTURE SHOULD PROVIDE 120M (structural capacity)\*

VERTICAL CIRCULATIONS > 60MIN (fire and smoke free)

HORIZONTAL CIRCULATIONS > 30MIN (smoke free)



\*can be reduced 30-60 minutes if automatic sprinklers

## FIRE CONTRIBUTION

### CLT

CONTRIBUTION TO FIRE

**D**

SMOKE PRODUCTION

**S1/S2**

FLAME DROPLET CLASS

**D0**

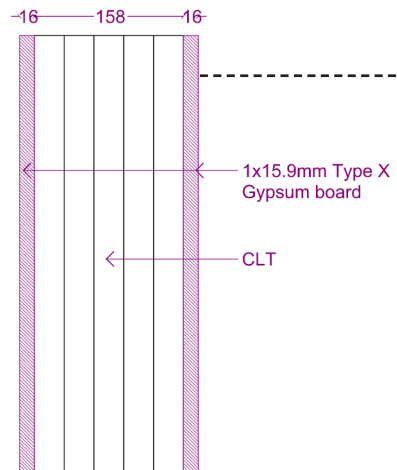
Cannot be used in fire separation  
compartments without additional  
protection

## ENCAPSULATION

### LIMITED

BEFORE FLASHOVER

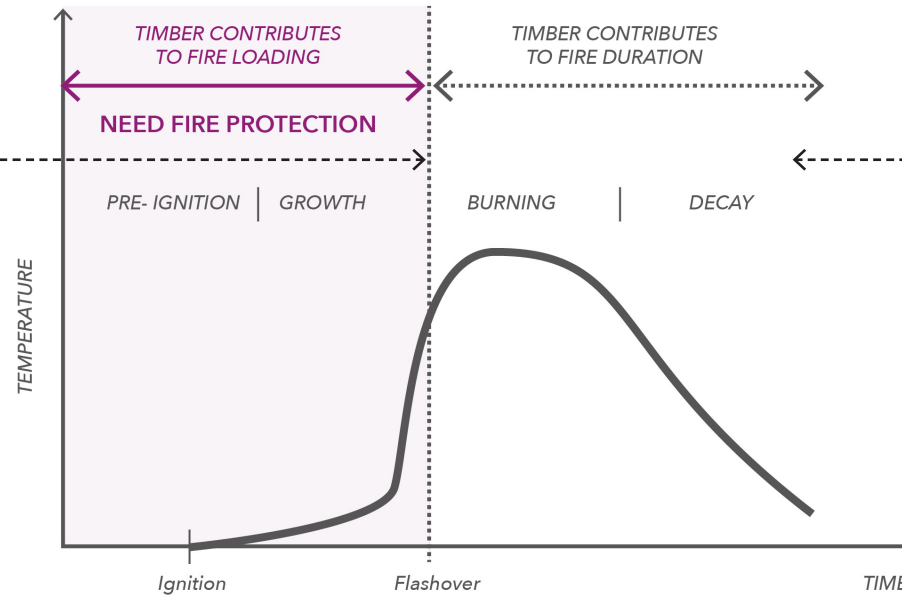
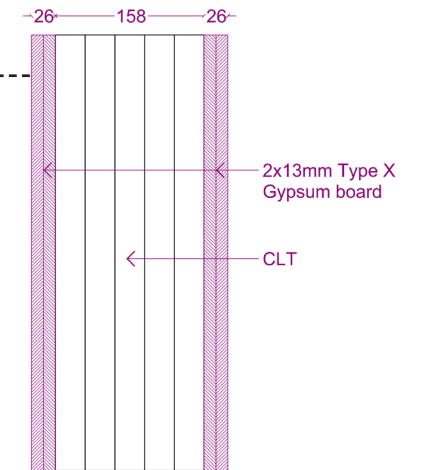
60 min



### COMPLETE

AFTER FLASHOVER

120 min



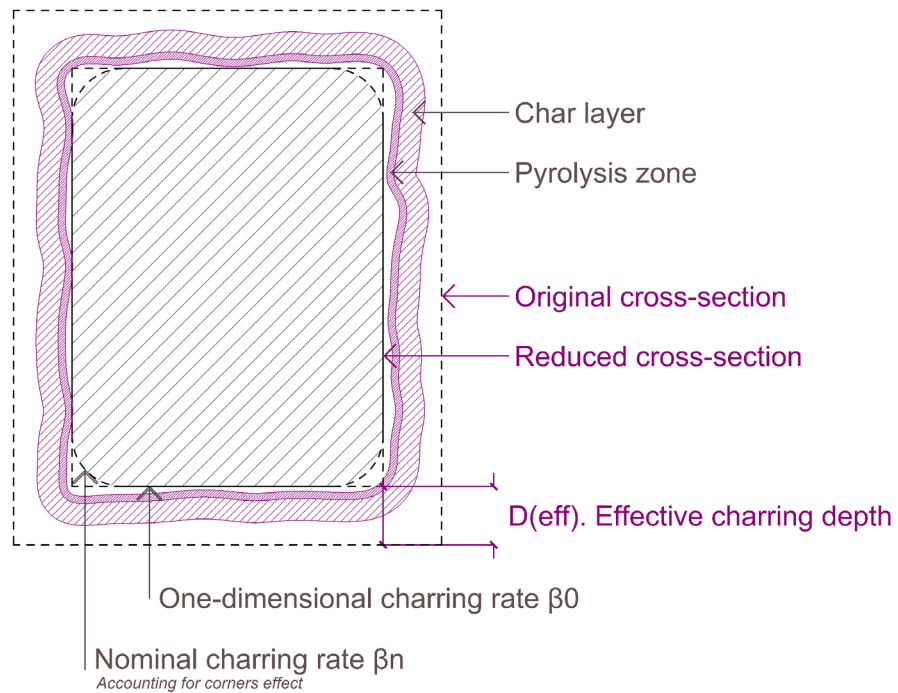
## EXPOSED

Only within fire compartments

Research + approval with fire authorities

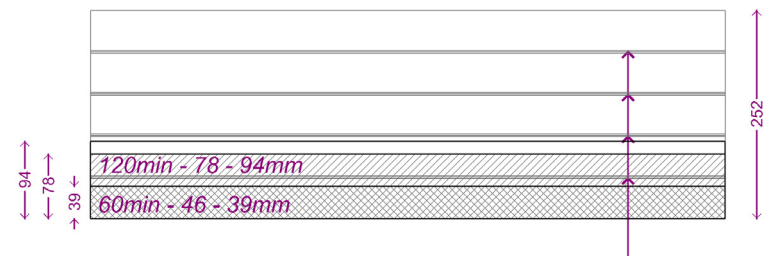
### CHARRING DEPTH

Type of timber  
One or more side exposed



### DE-LAMINATION CLT

Thickness of laminations  
Adhesive type



**Phenol-resorcinol-formaldehyde**  
**NO DELAMINATION**

*0.65mm/min*

**Polyurethane (PUR)**

**DELAMINATION**

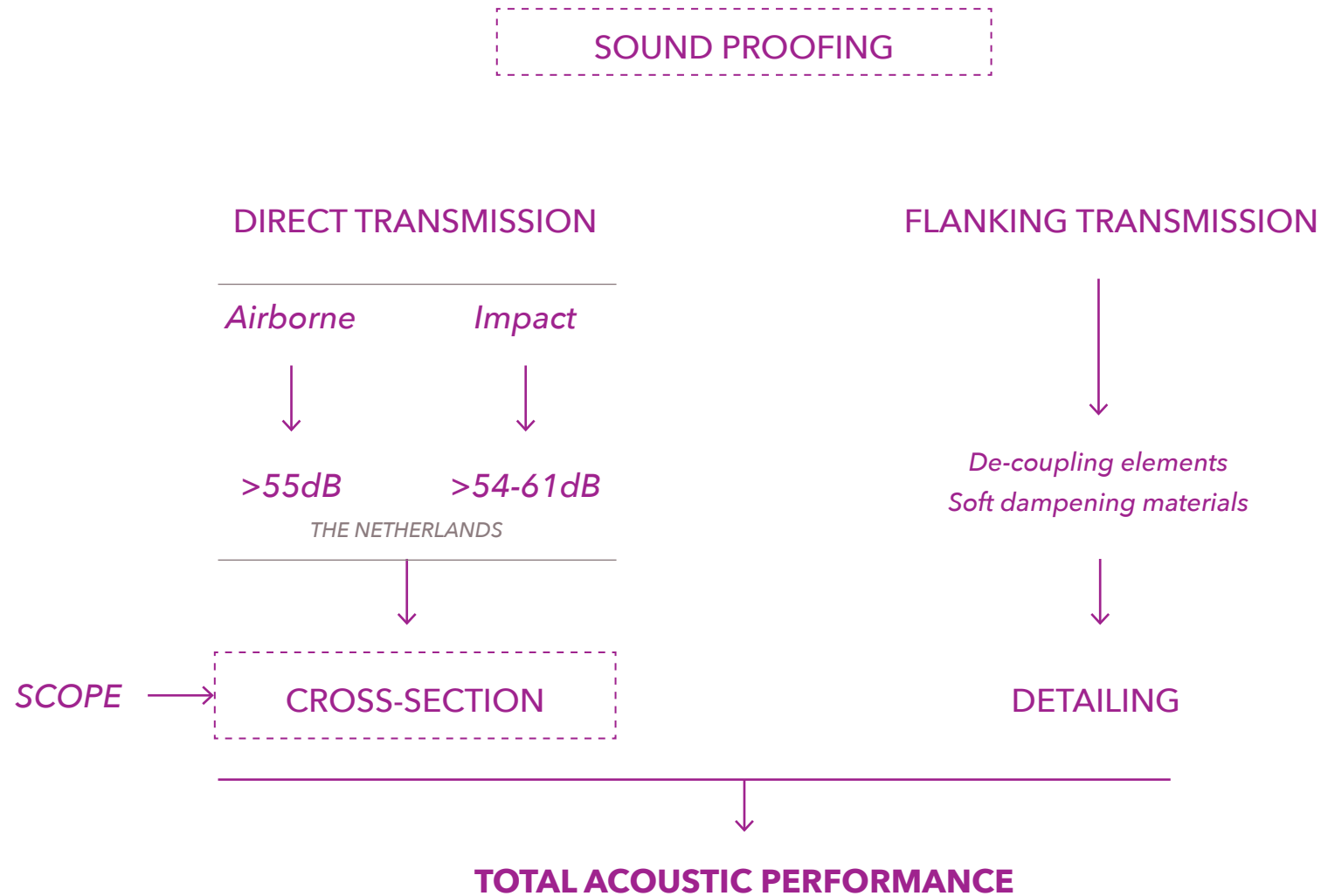
*35mm - 0.65mm/min*

*30mm - 0.69mm/min*

*25mm - 0.73mm/min*

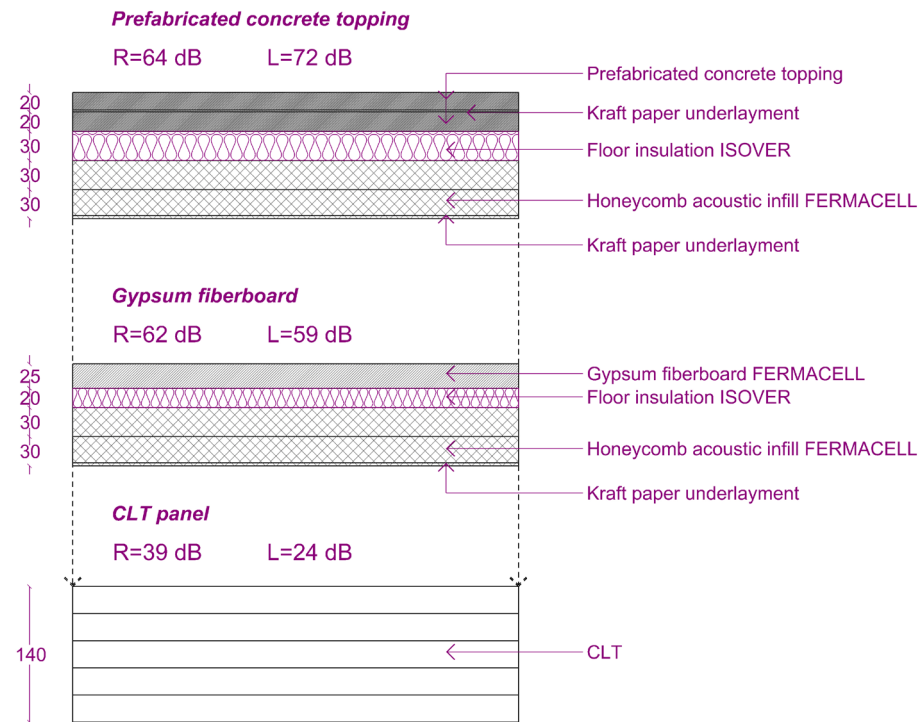
*20mm - 0.78mm/min*



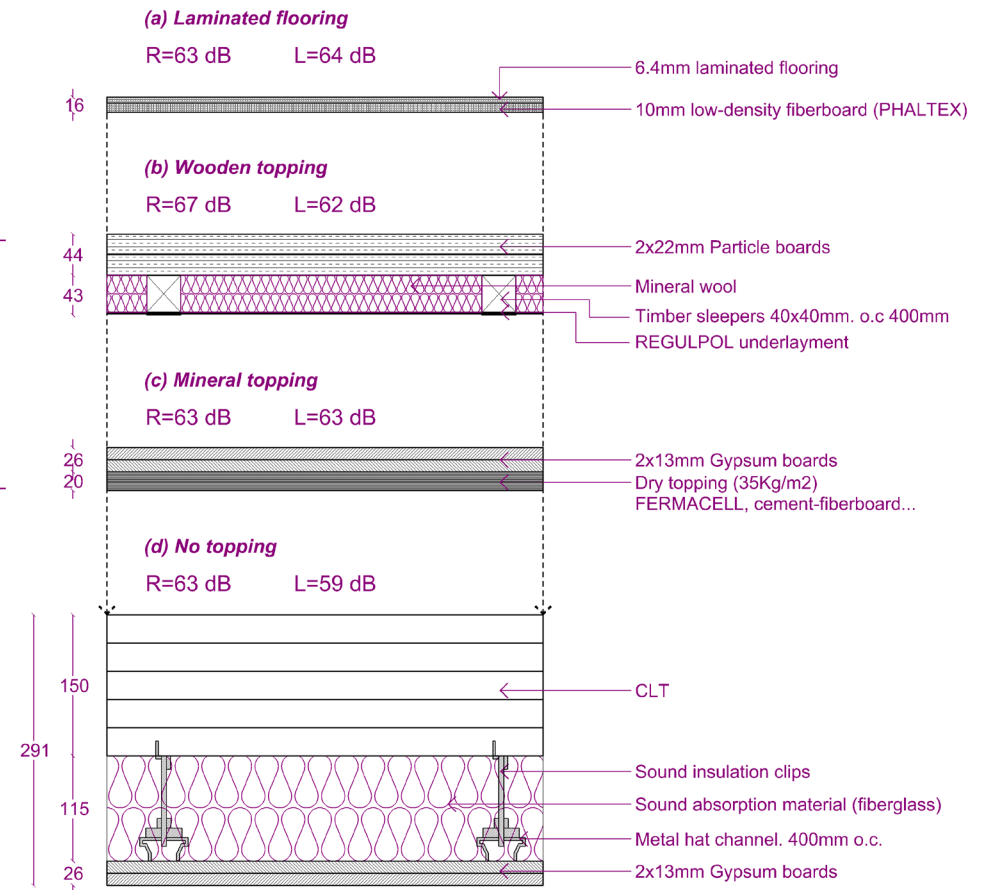


# SOUND PROOFING

## CLT FLOOR



## CLT FLOOR + CEILING



## SOUND PROOFING

### CLT WALL

#### *Double CLT panel*

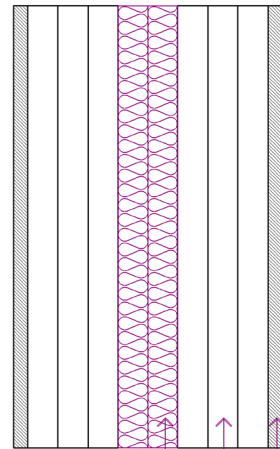
**30mm Mineral wool**

**R=55 dB**

**60mm Mineral wool**

**R=60 dB**

15 95 30 30 95 15



Mineral wool

95-115mm CLT

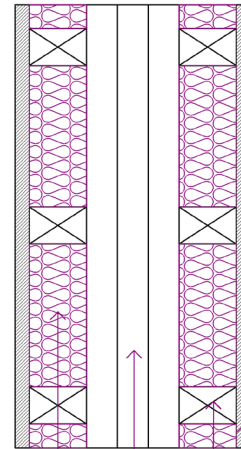
15mm Gypsum board

#### *CLT panel + studs*

**120mm Mineral wool**

**R=58 dB**

15 60 95 60 15



Mineral wool

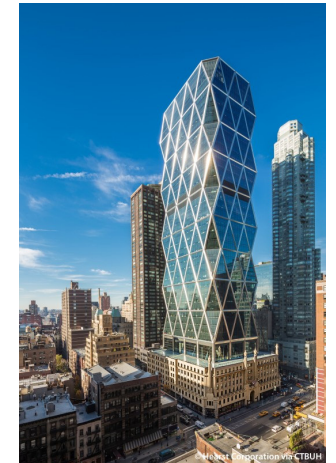
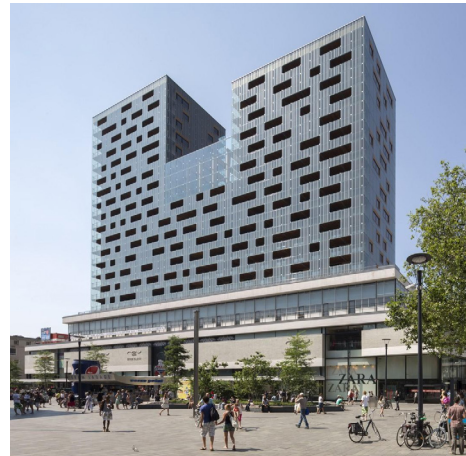
CLT

38x63mm Timber studs 400 o.c.

15mm Gypsum boards

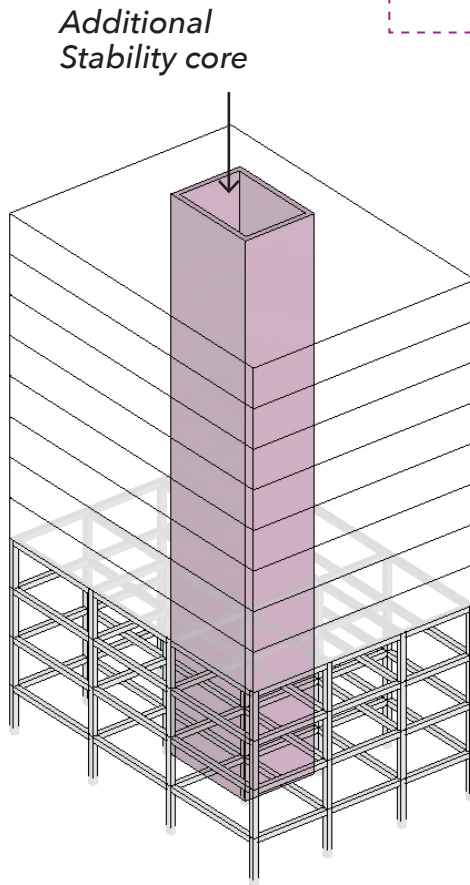
EXTENSION (TE)

**TO WHAT EXTENT IS A TALL EXTENSION (TE)  
TECHNICALLY FEASIBLE?**





## "TALL" EXTENSION



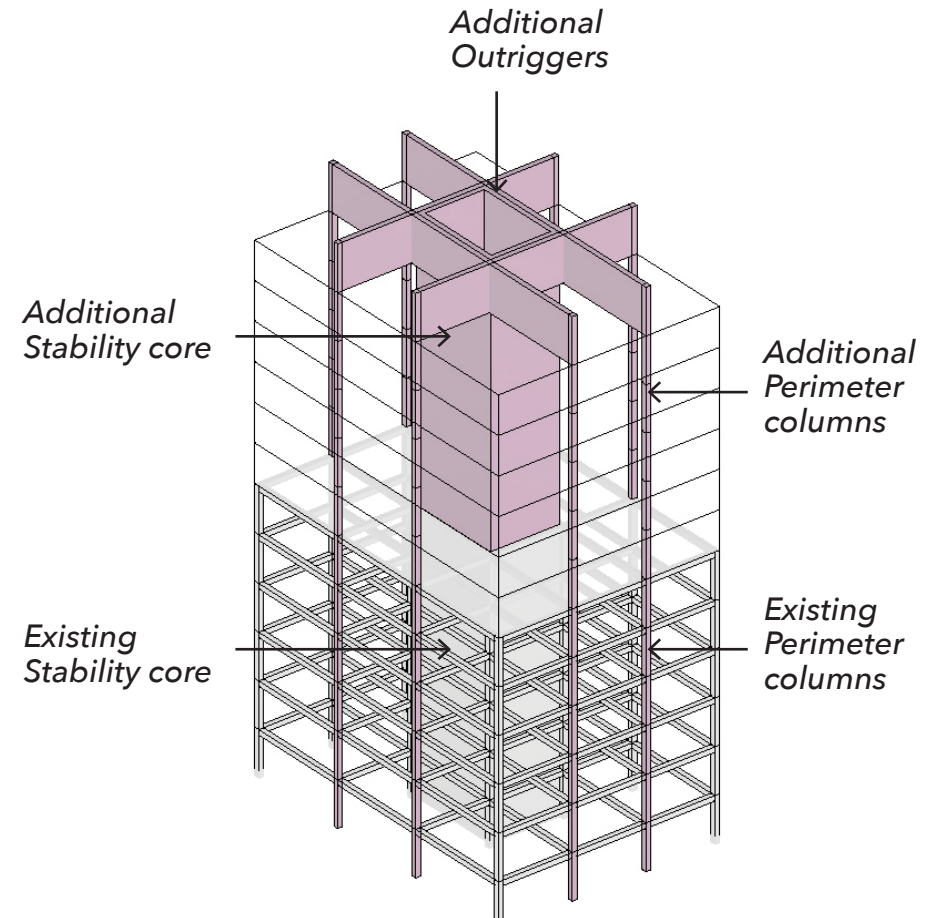
*Lateral stability:*

- Additional stability core
- Composite action with existing

*Vertical loading:*

- Existing structure
- Additional stability core

*Demolition required*  
*Medium/Large-scale*



*Lateral stability*

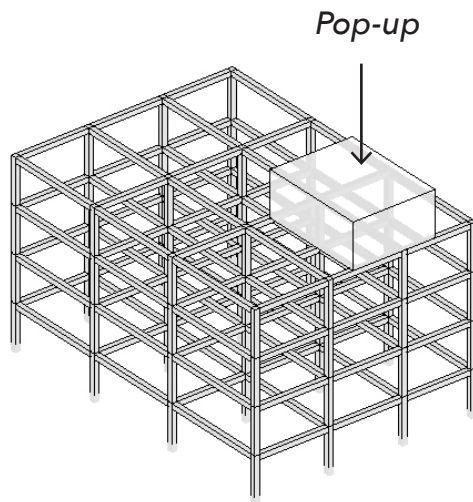
- Composite action with existing

*Vertical loading:*

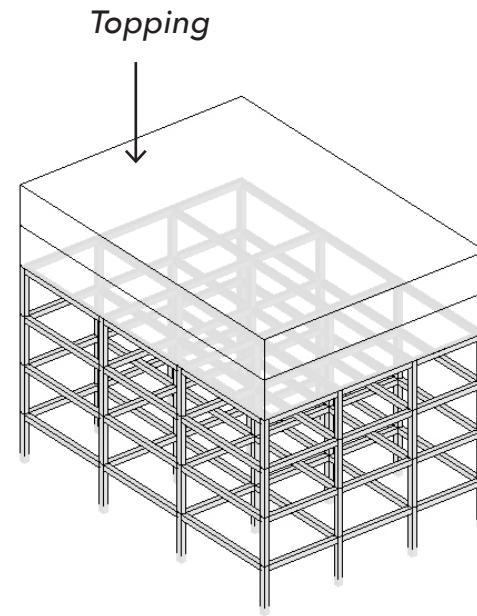
- Existing structure
- Demolition required

*Medium/Large-scale*

## "SMALL" EXTENSION



*Lateral stability:*  
- Existing structure  
*Vertical loading:*  
- Existing structure  
*No demolition*  
*Small scale*



*Lateral stability:*  
- Existing structure  
*Vertical loading:*  
- Existing structure  
*Reduced demolition*  
*Small/medium scale*

## EXTENSIONS

Type of extension	Extra storeys	Use of existing structure	Demolition	Tall building
Pop-up	1-2	+	-	-
Topping	2-4	+	-	-
Stability cores	4-16	+/-	+/-	+/-
Outriggers	2-4	+/-	+/-	+/-
Table structure	2-4	-	-	-

+ Likely; +/- Partially; - Unlikely



## EXTENSIONS

### "TALL" EXTENSION

*> 4 extra storeys*

*New stability system*

*Strengthen existing structure*

*Demolition*

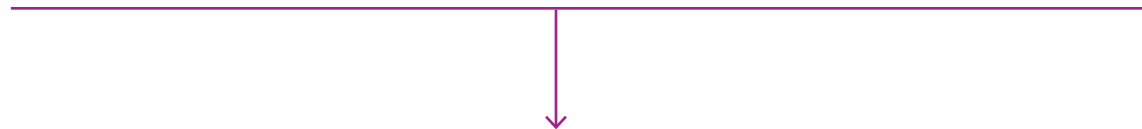
*Complex*

### "SMALL" EXTENSION

*< 4 extra storeys*

*Existing stability system*

*No demolition*



**RESPECT EXISTING STRUCTURAL GRID**

**EVEN DISTRIBUTION OF VERTICAL LOADING**

Avoid extra complexity, transfer structures, differential settlements

**WHAT IS A TALL BUILDING?**

## TALLNESS AND HEIGHT

HEIGHT is objective (number of meters)

TALL is subjective (in relation to references)

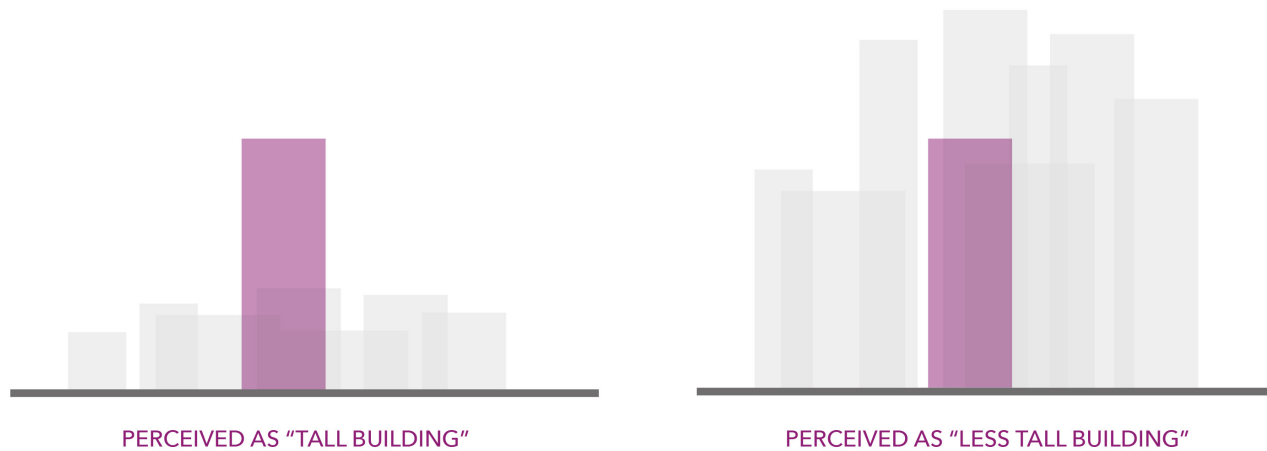
## OBJECTIVE HEIGHT

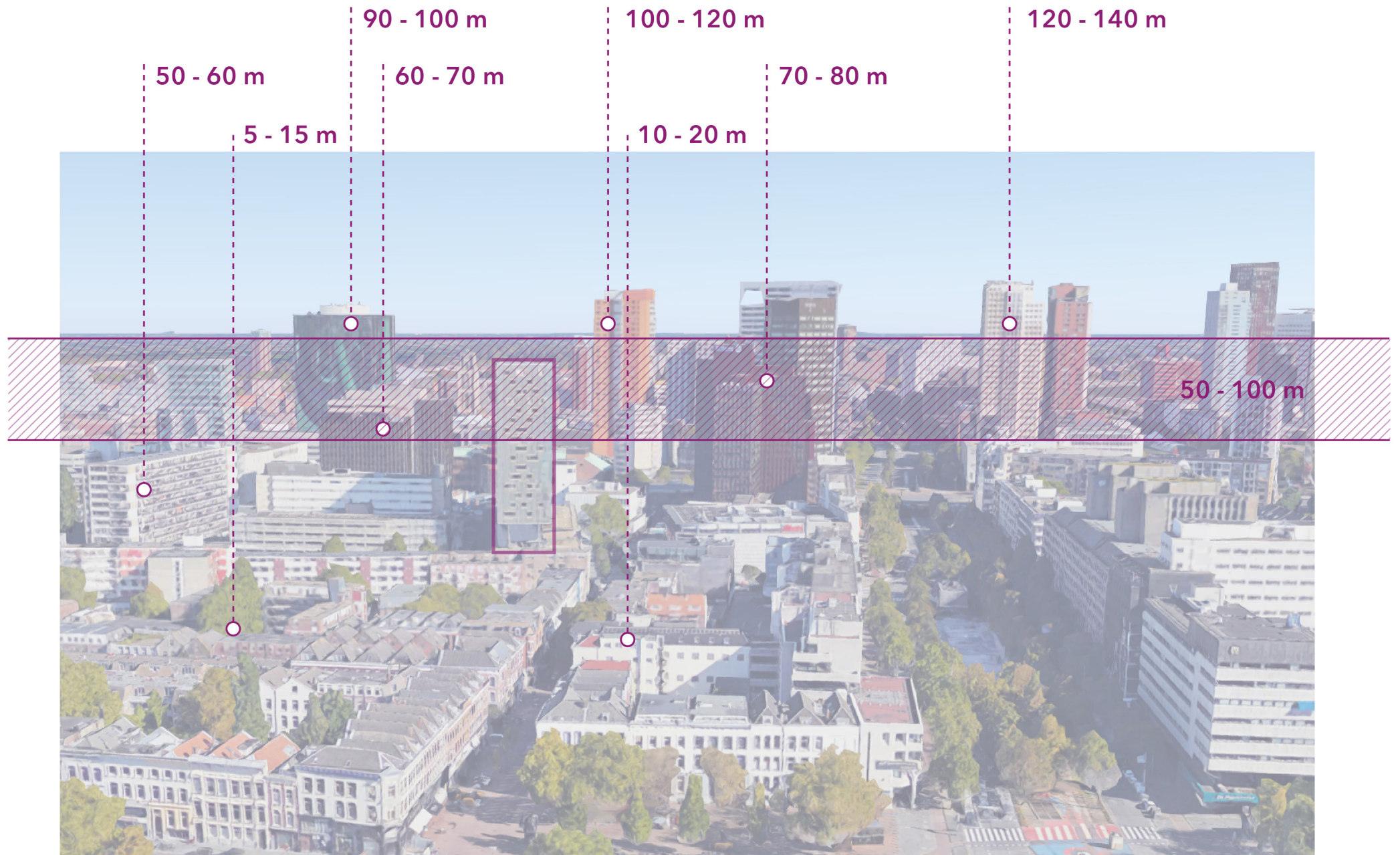
According to CTBUH, tall building is above 50 meters

According to National Covenant Hoogbouw, tall building is above 70 meters

## SUBJECTIVE TALLNESS

According to context (CTBUH)

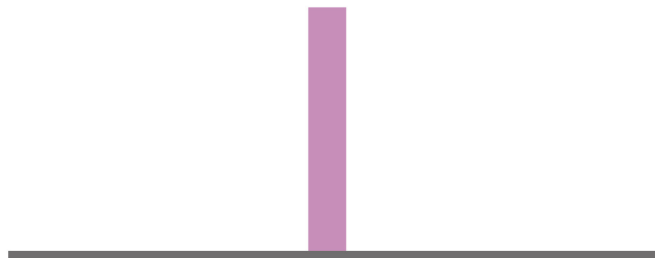








According to proportion (CTBUH)



PERCEIVED AS "TALL BUILDING"



PERCEIVED AS "LESS TALL BUILDING"



According to CTBUH a building can be considered tall if it uses tall-building technologies:

- Lateral resisting system
- Damping for wind-induced accelerations
- Fire design
- Vertical transportation

According to Foster et al. 2013, tall building can be defined as any height that exceeds current precedents

### **Tall Timber (TT)**

The Treet” and UBC Brock Commons



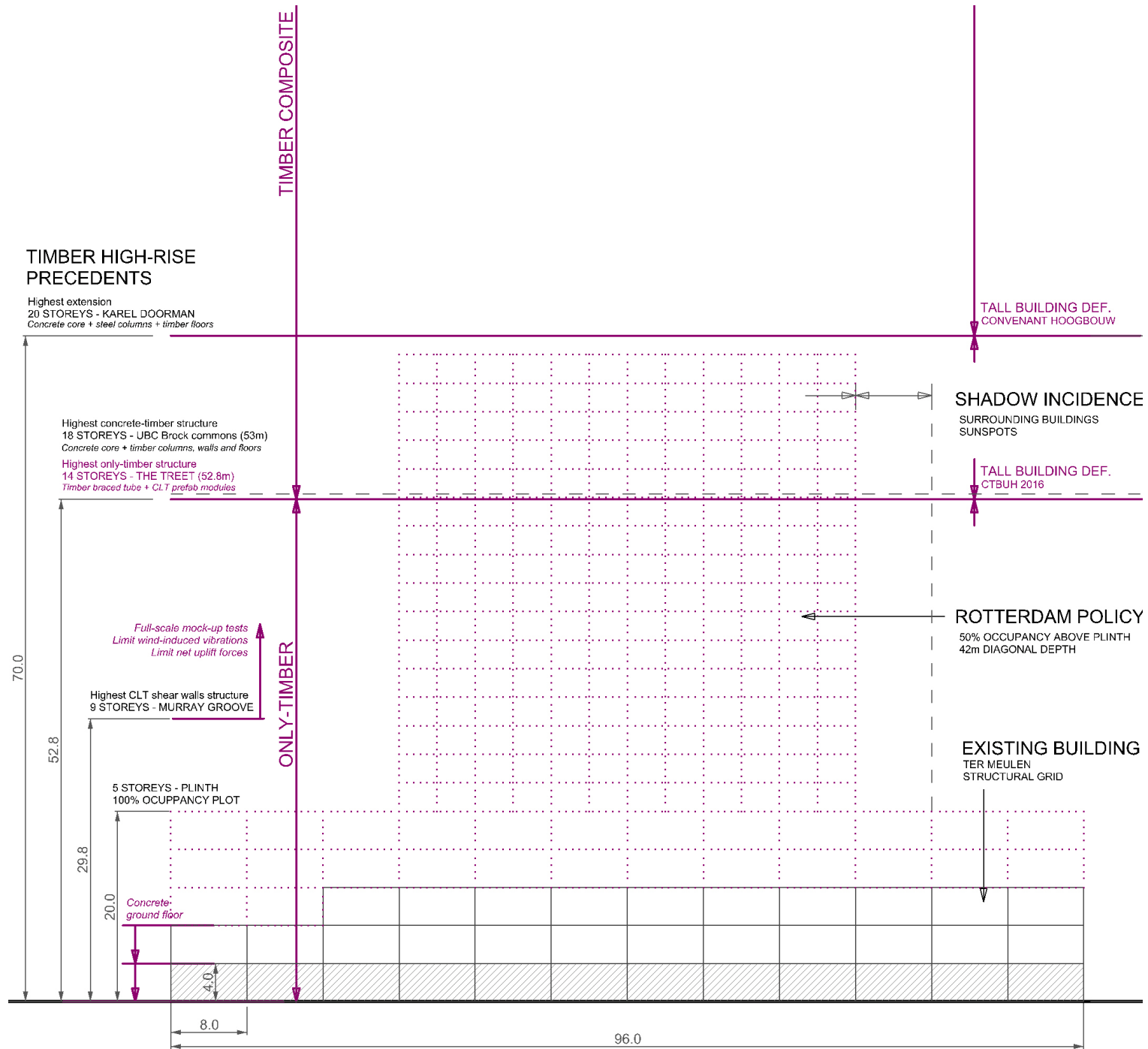
**53 M**

### **Tall Extension (TE)**

Karel Doorman



**70 M**



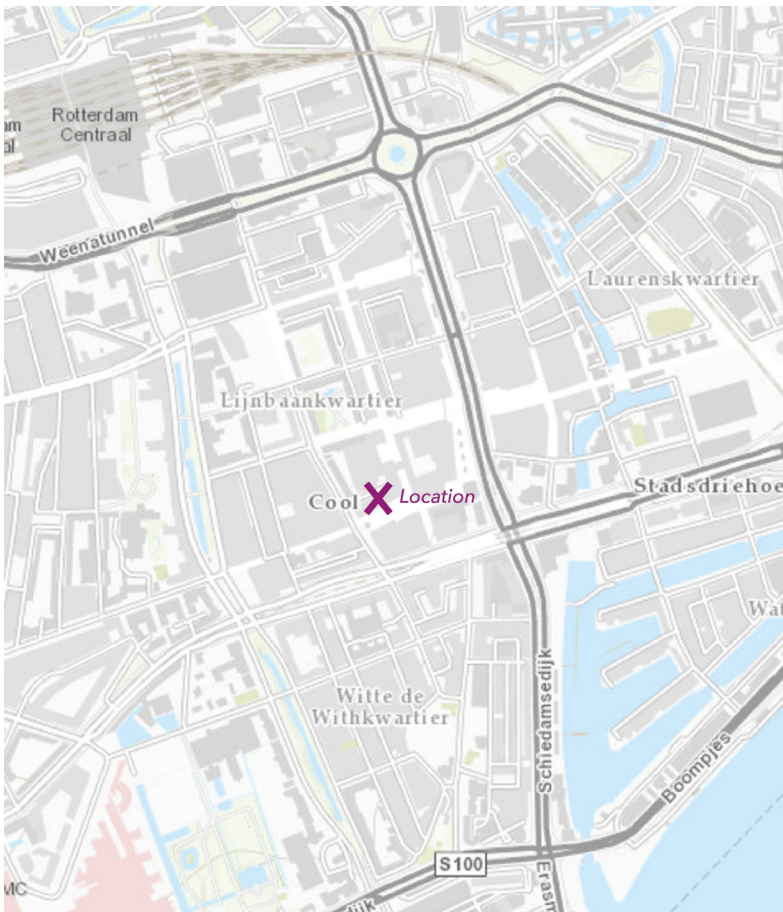
## DESIGN STUDY

**WHAT IS THE CITY POLICY REGARDING TALL  
BUILDINGS IN THE INTENDED LOCATION?**

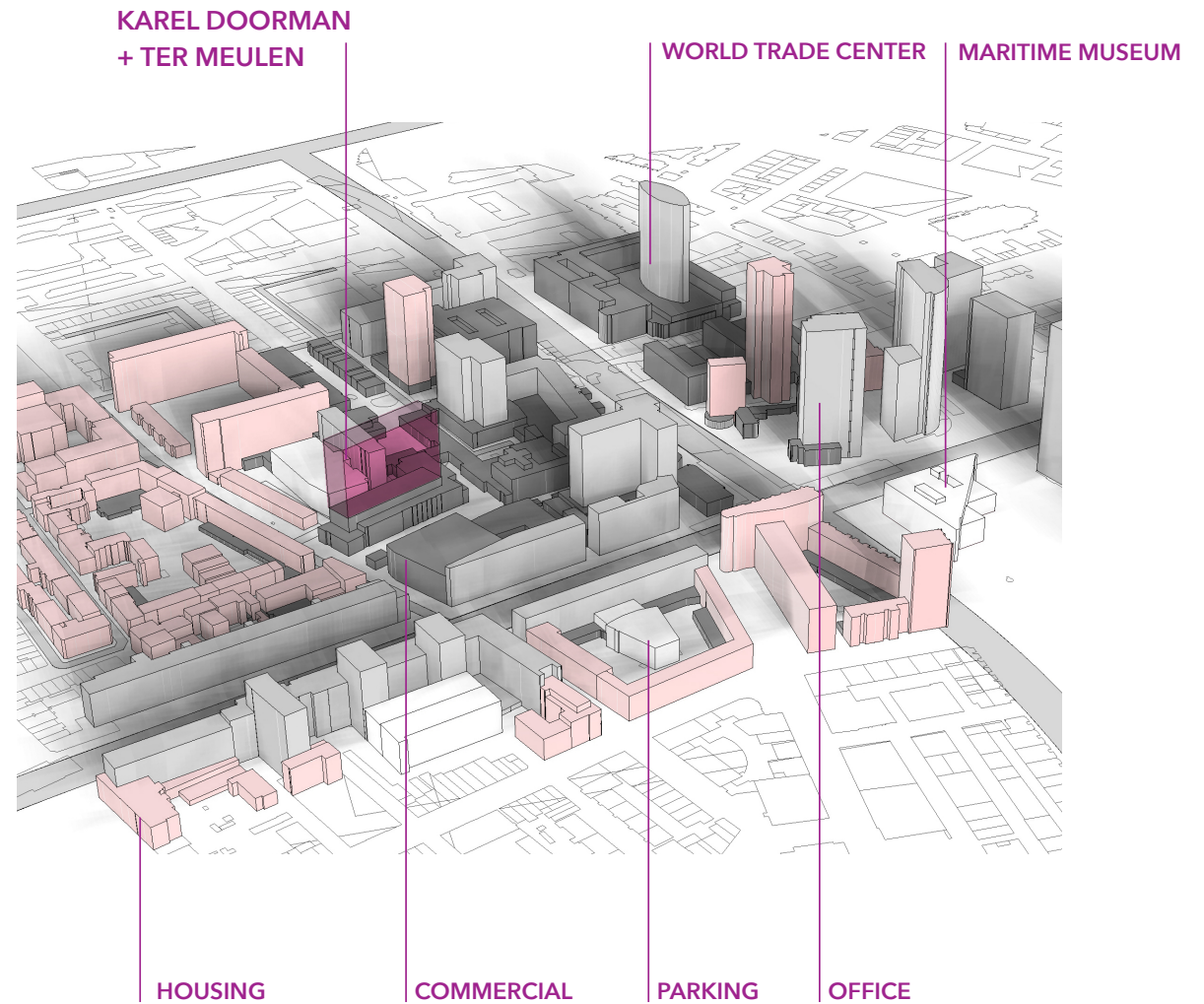
## LOCATION

Important commercial location

Pedestrian active area



COOL DISTRICT - ROTTERDAM CENTER



## HIGH-RISE ZONING



High-rise area

No height limit

Transition zone

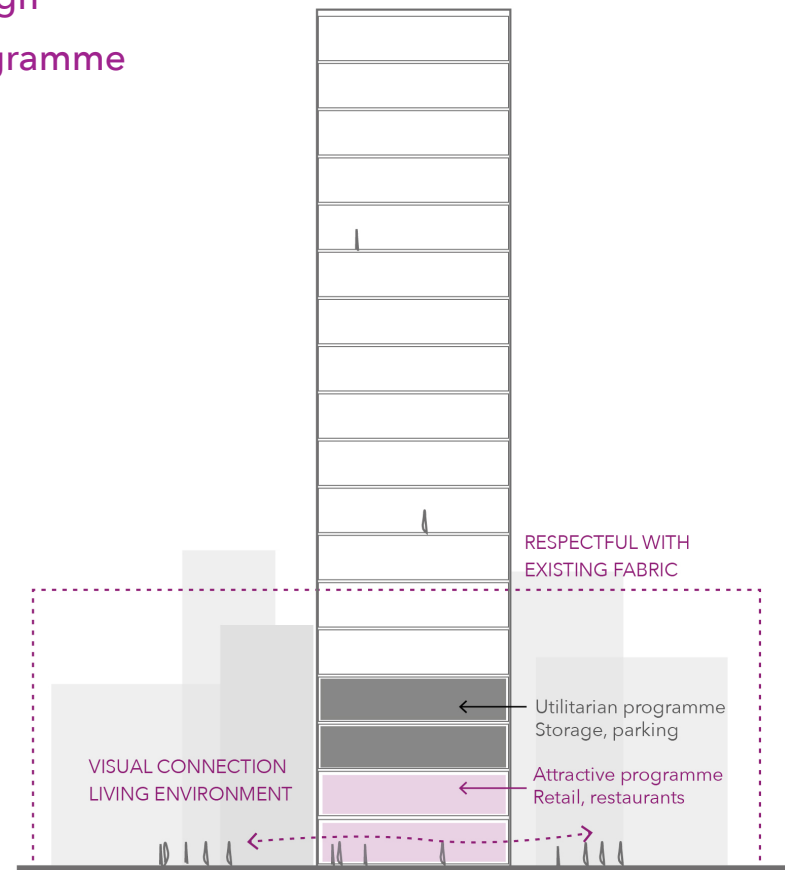
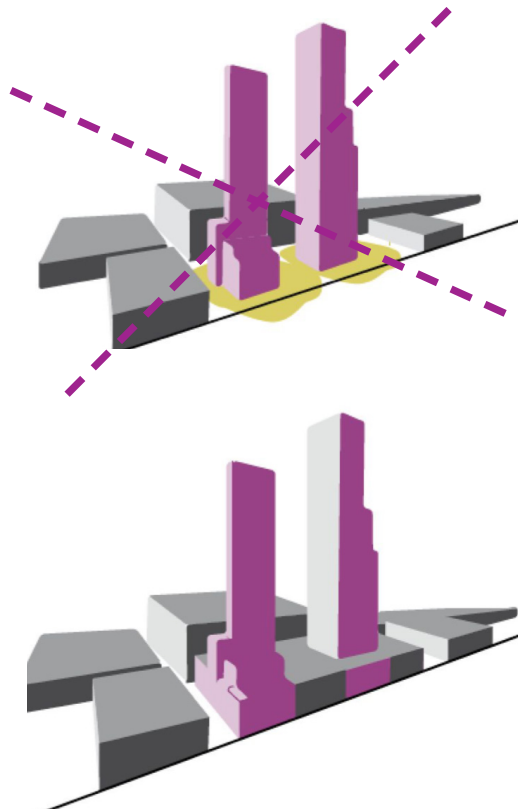
Height limit 70-150m

## URBAN PLINTH

Urban fabric continuity

20-25 meters high

Urban-attractive programme





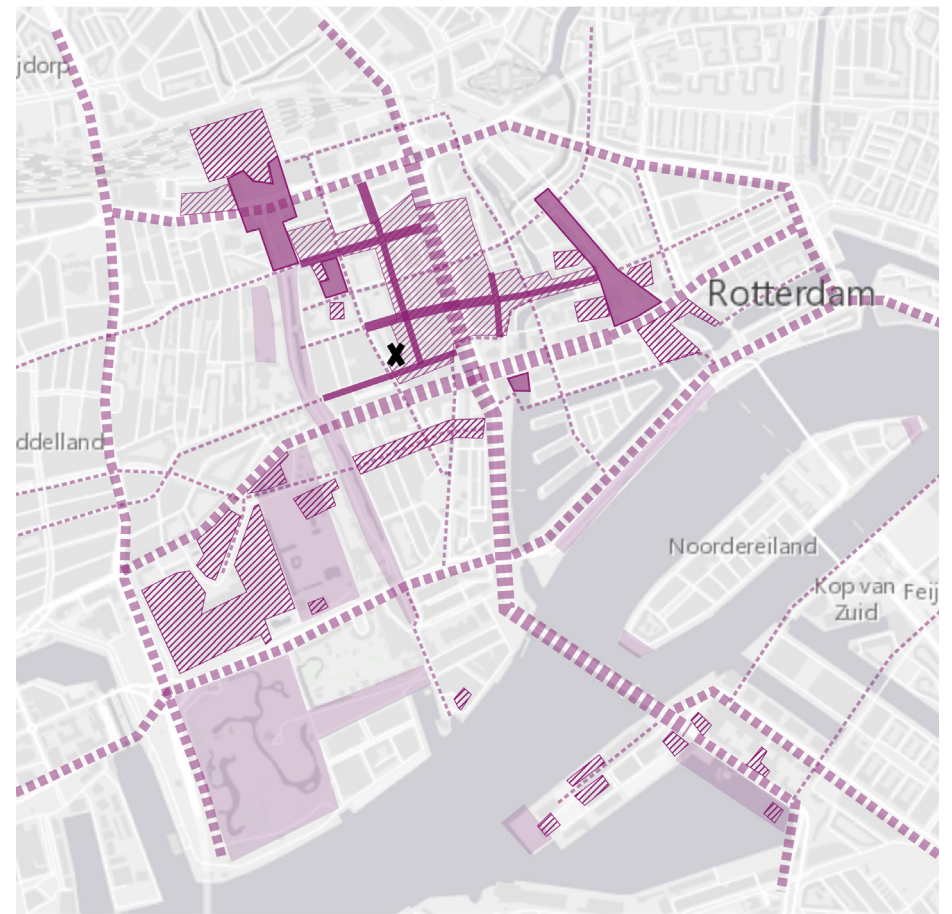
## SHADOW IMPACT

### CLIMATE COMFORT ZONE



- Comfort zone
- Super high-rise areas
- Climate buffer zone

### ACTIVE CITY CENTRE





- Main avenues and boulevards
- Secondary streets
- Squares
- (Semi) public ground floor
- Parks

## SHADOW IMPACT

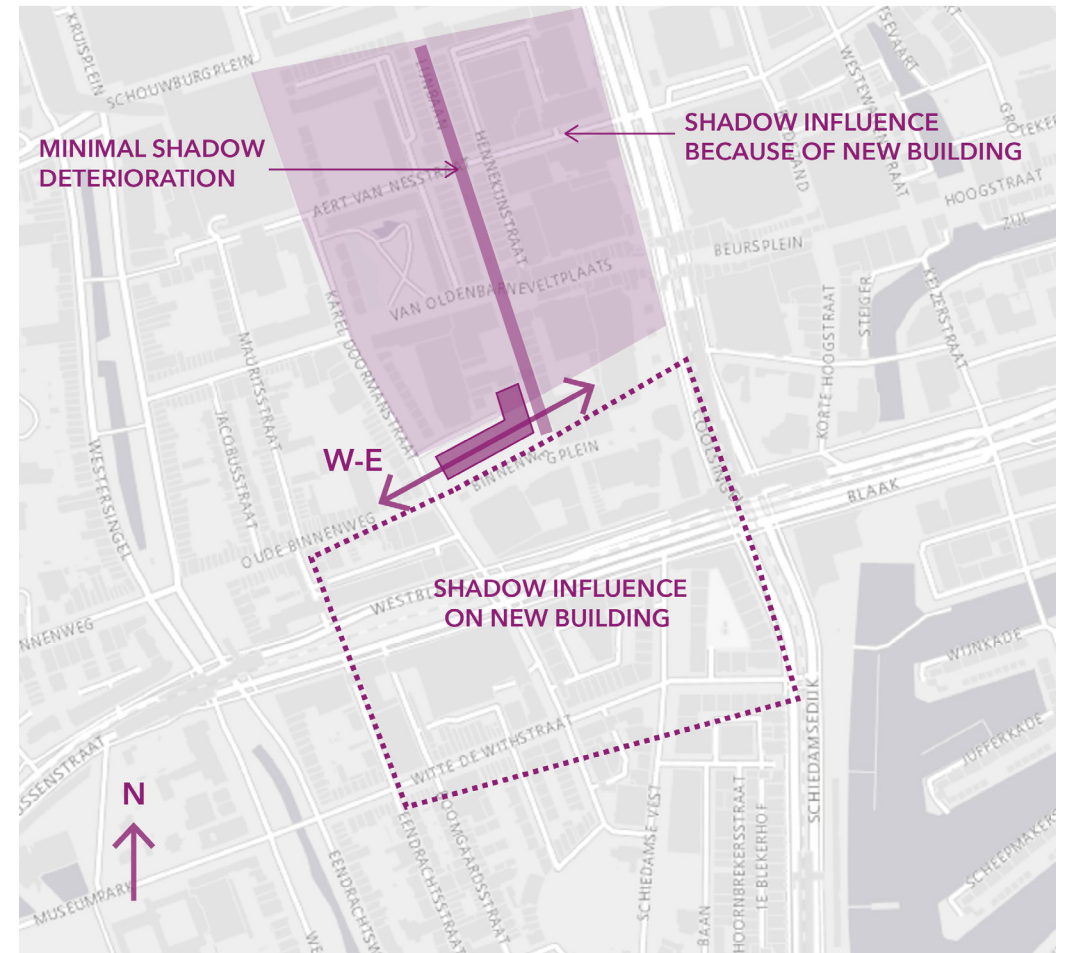
### PROTECTED ZONES



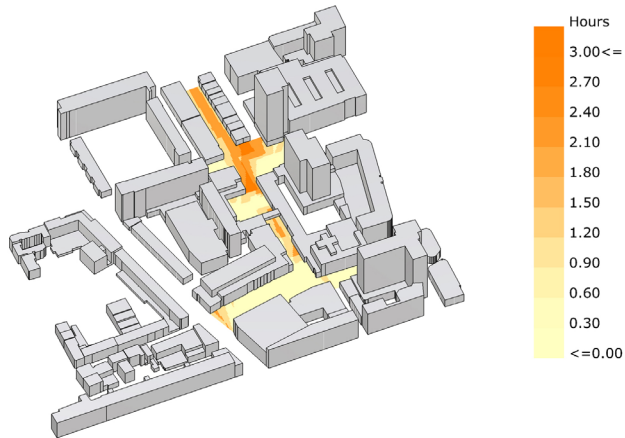
 Sunspots. No deterioration allowed during time of use.

 Sites with special quality (Slight shadow deterioration is allowed)

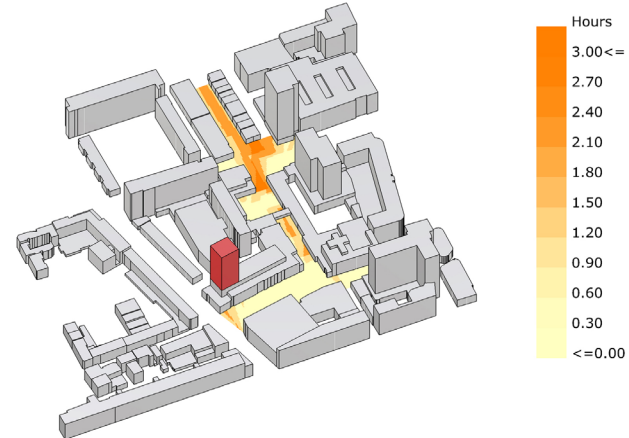
### LOCATION



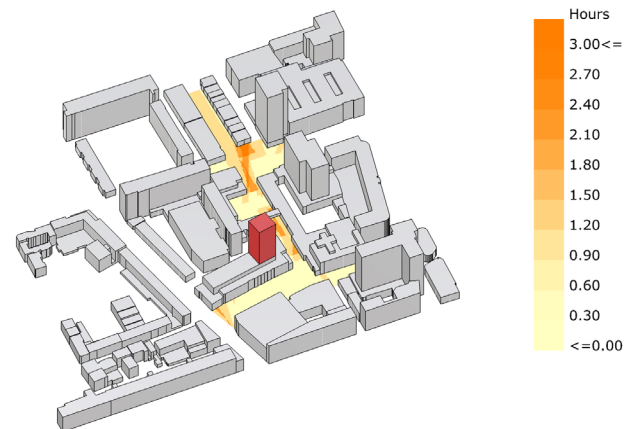
# SHADOW ANALYSIS 21 DEC



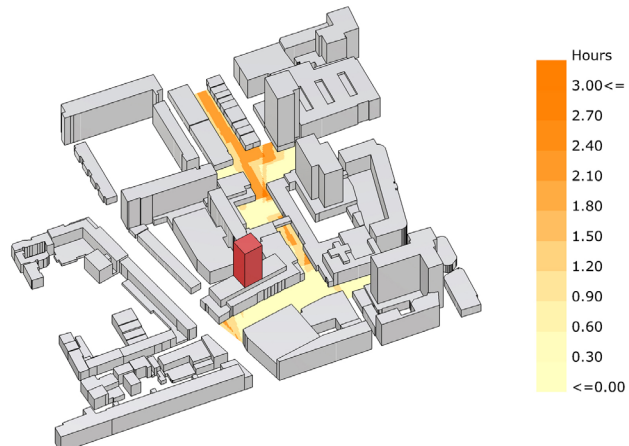
**CONTEXT** >1h sunlight : 34%  
>2h sunlight: 16%



**SCENARIO 2** >1h sunlight : 34%  
>2h sunlight: 16%  
- Most optimal position -



**SCENARIO 1** >1h sunlight : 31%  
>2h sunlight: 5.7%



**SCENARIO 3** >1h sunlight : 32%  
>2h sunlight: 14%



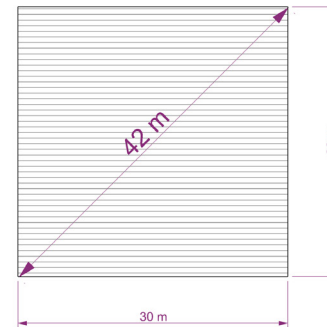
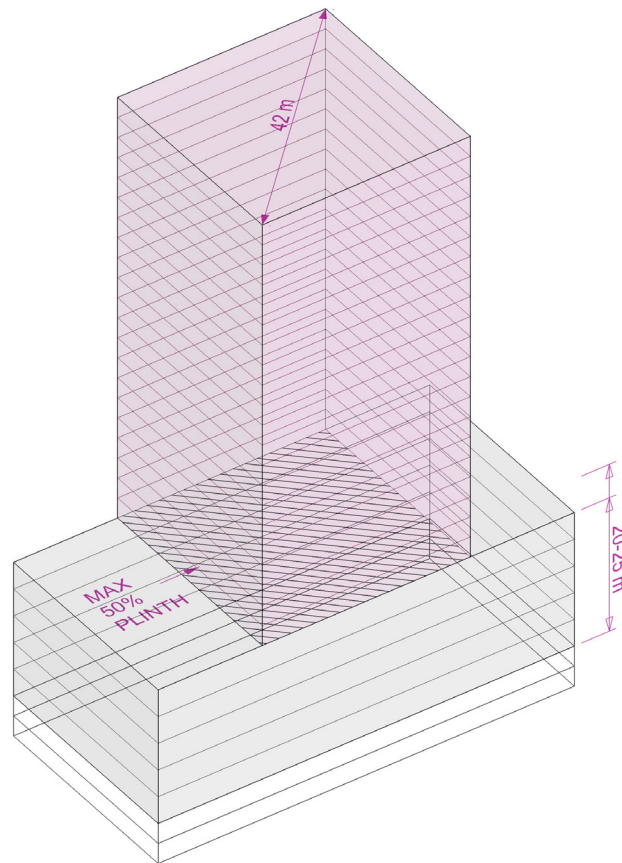
## MAXIMUM VOLUME

50% volume above plinth

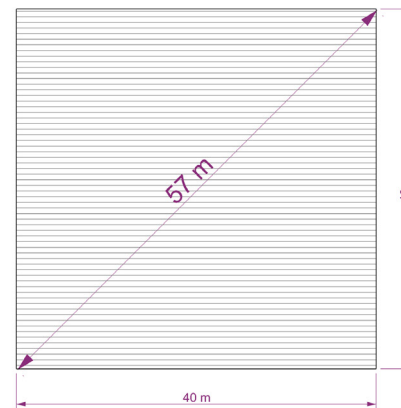
Area: 900m<sup>2</sup>

Depth: 30m

Diagonal 42m



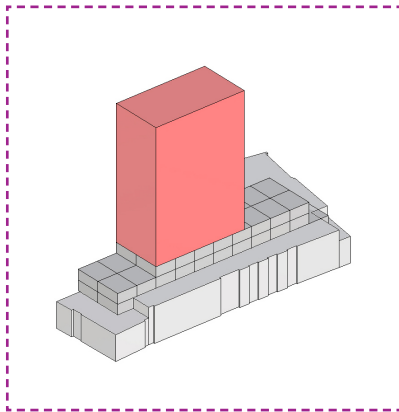
RESIDENTIAL USE  
GFA 900m<sup>2</sup>



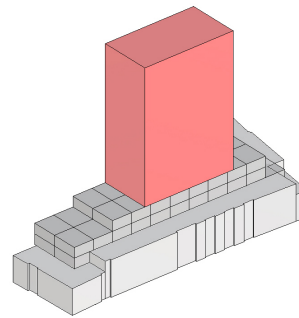
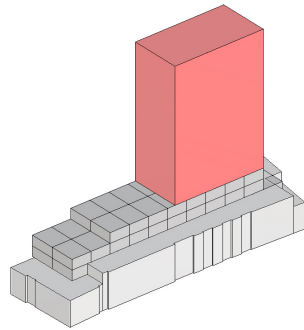
OFFICE USE  
GFA 1600m<sup>2</sup>

## SCHEMATIC SHAPES REGARDING POLICY MEASURES

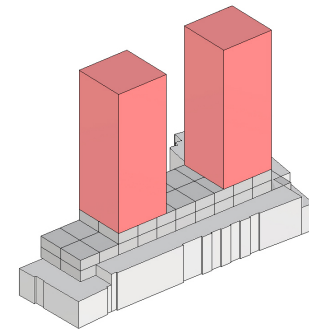
DISK



LESS SHADOW IMPACT



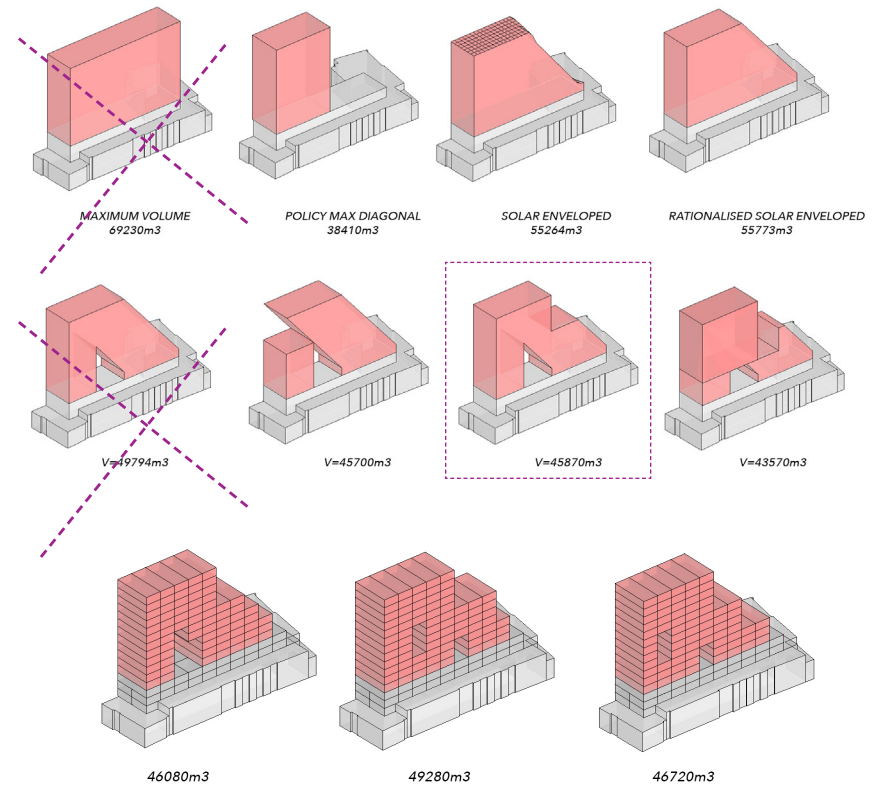
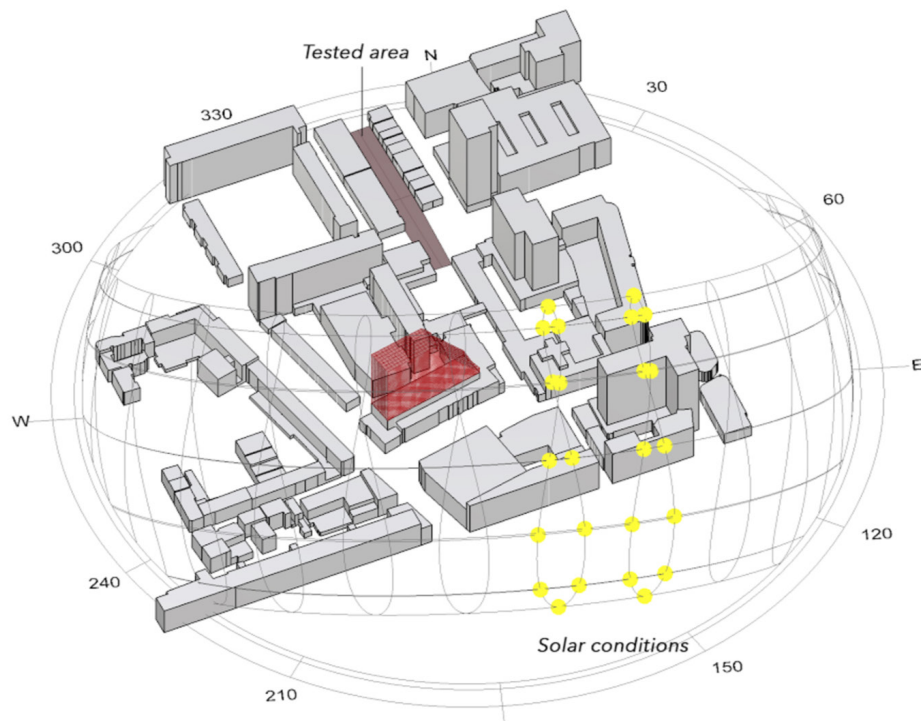
2 ISOLATED TOWERS



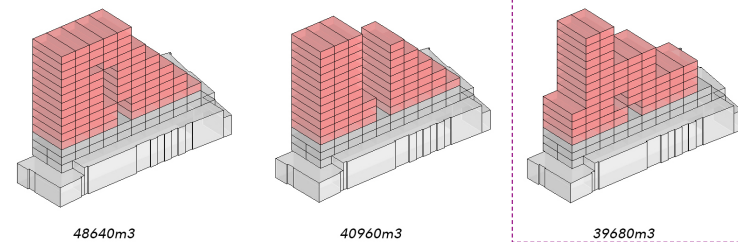
# SOLAR ENVELOPE

11-12 p.m. JAN - DEC

Maximum sunlight in Lijnbaan street



MIN SHADOW IMPACT (Rotterdam policy)  
 MAX VOLUME (Economics)  
 MIN 42M DIAGONAL (Rotterdam policy)  
 1 SOLID MASS (Architectural)



# SOLAR ENVELOPE

ALL YEAR

More than 50m height average

At least 25% boundaries higher than 70 meters

Maximum sunlight on Lijnbaan street



Sunlight = 29%  
Average height = 46  
Maximum height = 68  
N° Towers = 4



Sunlight = 29%  
Average height = 50  
Maximum height = 74  
N° Towers = 3



Sunlight = 30%  
Average height = 41  
Maximum height = 72  
N° Towers = 9



Sunlight = 35%  
Average height = 47  
Maximum height = 70  
N° Towers = 3



Sunlight = 34%  
Average height = 51  
Maximum height = 68  
N° Towers = 2



Sunlight = 36%  
Average height = 44  
Maximum height = 74  
N° Towers = 4



Sunlight = 24%  
Average height = 53  
Maximum height = 72  
N° Towers = 3



Sunlight = 18%  
Average height = 49  
Maximum height = 78  
N° Towers = 4



Sunlight = 48%  
Average height = 40  
Maximum height = 68  
N° Towers = 5



Sunlight = 14%  
Average height = 54  
Maximum height = 78  
N° Towers = 4



Sunlight = 15%  
Average height = 54  
Maximum height = 78  
N° Towers = 4



Sunlight = 13%  
Average height = 55  
Maximum height = 76  
N° Towers = 7



Sunlight = 12%  
Average height = 57  
Maximum height = 80  
N° Towers = 4



Sunlight = 14%  
Average height = 55  
Maximum height = 80  
N° Towers = 4



Sunlight = 28%  
Average height = 46  
Maximum height = 74  
N° Towers = 6



Sunlight = 22%  
Average height = 49  
Maximum height = 74  
N° Towers = 4



Sunlight = 29%  
Average height = 46  
Maximum height = 74  
N° Towers = 4



Sunlight = 11%  
Average height = 52  
Maximum height = 74  
N° Towers = 5



Sunlight = 19%  
Average height = 50  
Maximum height = 74  
N° Towers = 5



Sunlight = 21%  
Average height = 48  
Maximum height = 74  
N° Towers = 5



Sunlight = 30%  
Average height = 44  
Maximum height = 74  
N° Towers = 5



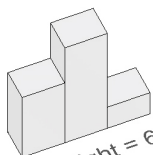
Sunlight = 15%  
Average height = 52  
Maximum height = 80  
N° Towers = 6



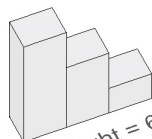
Sunlight = 18%  
Average height = 51  
Maximum height = 74  
N° Towers = 6



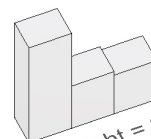
Sunlight = 22%  
Average height = 48  
Maximum height = 74  
N° Towers = 6



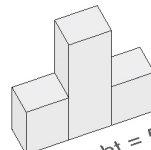
Sunlight = 69%  
Average height = 50  
Maximum height = 80



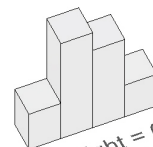
Sunlight = 67%  
Average height = 50  
Maximum height = 80



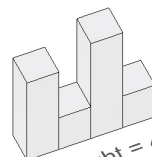
Sunlight = 60%  
Average height = 50  
Maximum height = 80



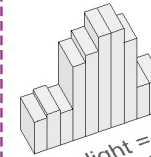
Sunlight = 57%  
Average height = 50  
Maximum height = 80



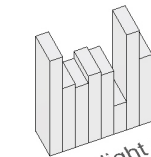
Sunlight = 61%  
Average height = 50  
Maximum height = 80



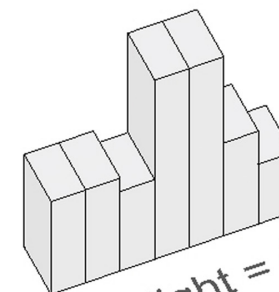
Sunlight = 48%  
Average height = 50  
Maximum height = 80



Sunlight = 58%  
Average height = 50  
Maximum height = 80



Sunlight = 52%  
Average height = 50  
Maximum height = 80



Sunlight = 64%  
Average height = 50  
Maximum height = 80

**TO WHAT EXTENT CAN ENGINEERED TIMBER  
INSPIRE THE ARCHITECTURE OF A TTE?**



## TIMBER CONCEPT

WOOD AND TIMBER ARE NOT THE SAME THING

### WOOD



*Tree in nature*



*Dried logs*

Natural  
Calm  
Soft

### TIMBER



*Blocked-glue laminated elements*



*Erection of Cross-Laminated Timber*

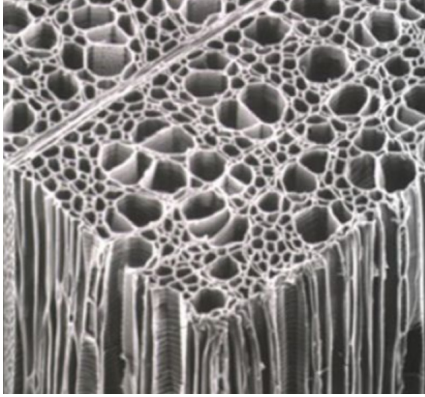
Precision  
Prefabrication  
Assembly

A QUEST FOR MORE TECHNICAL PRECISION

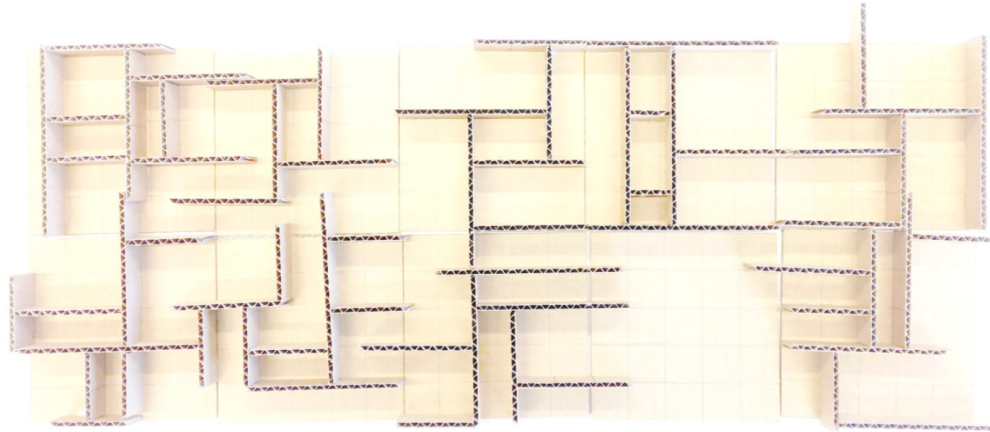


## TIMBER CONCEPT

Wood micro-structure



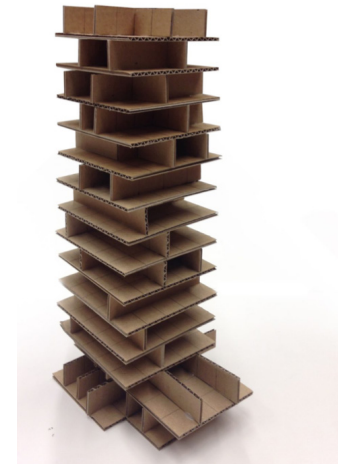
CLT macro-structure



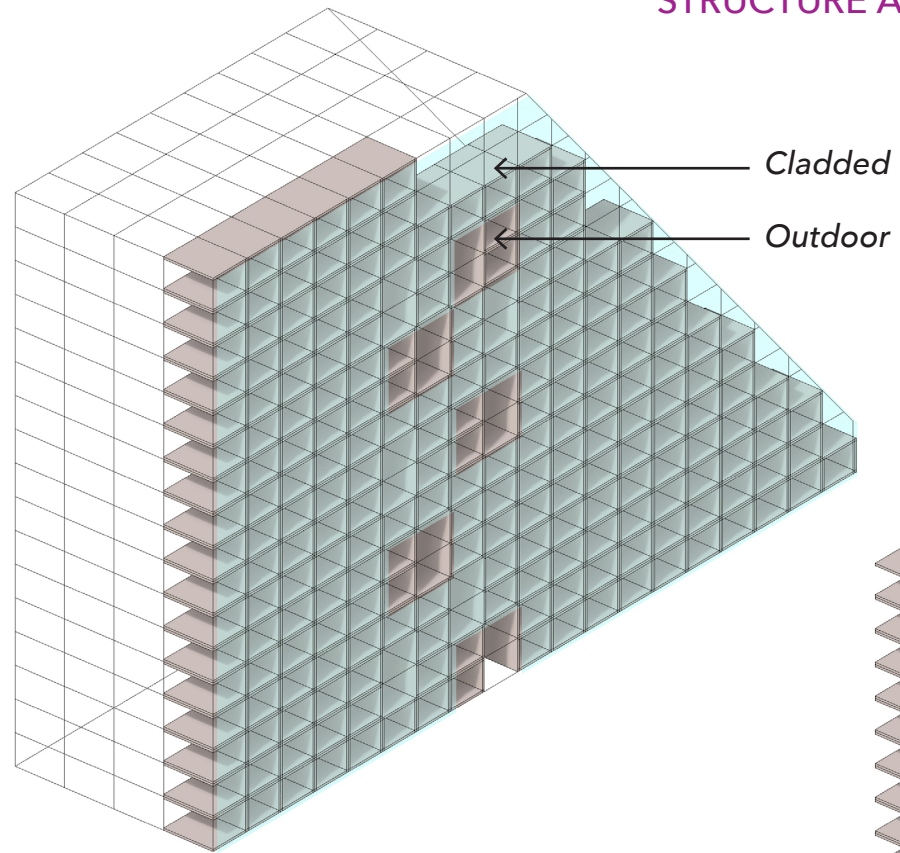
ORGANIC CELLULAR  
PATTERNS



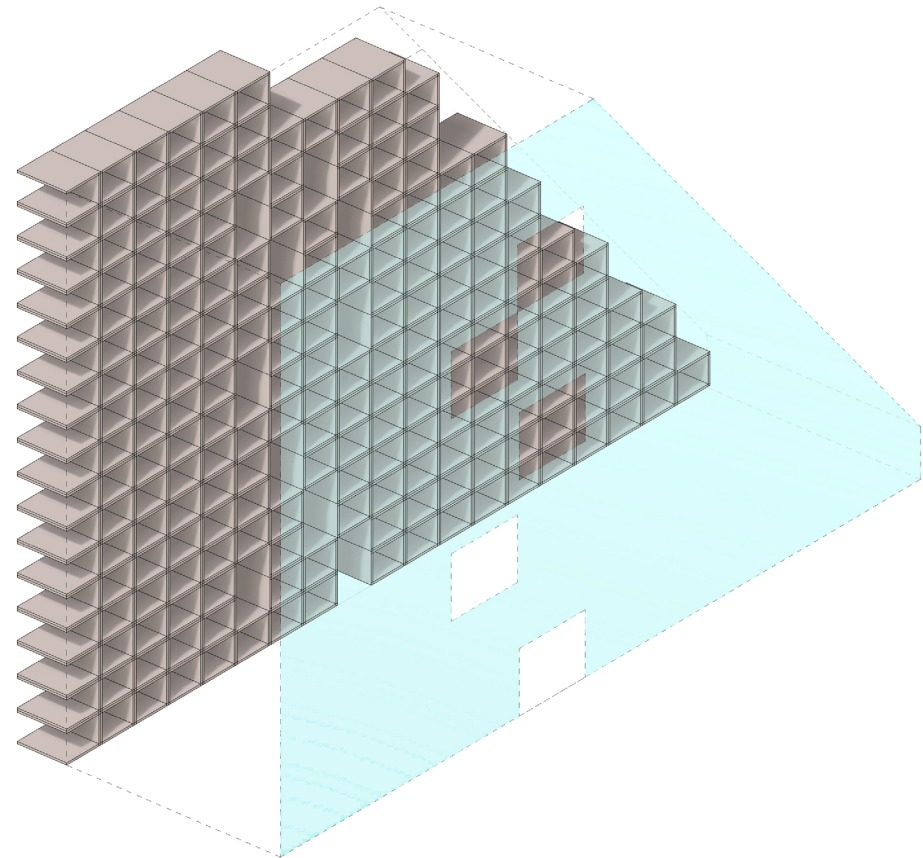
STRAIGHT CELLULAR  
PATTERNS



## STRUCTURE AS ARCHITECTURE



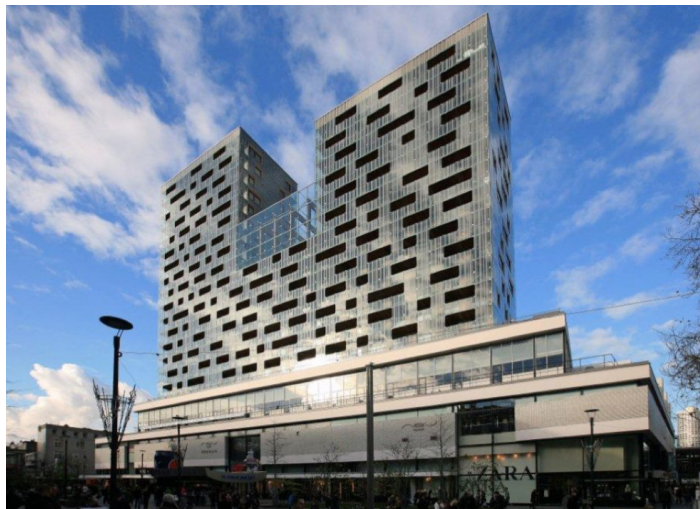
*The expression of the structure is the main leitmotiv for the architecture of the building.*



**WHAT ARE THE CURRENT MARKET AND  
USER DEMANDS FOR A TALL BUILDING IN  
ROTTERDAM?**

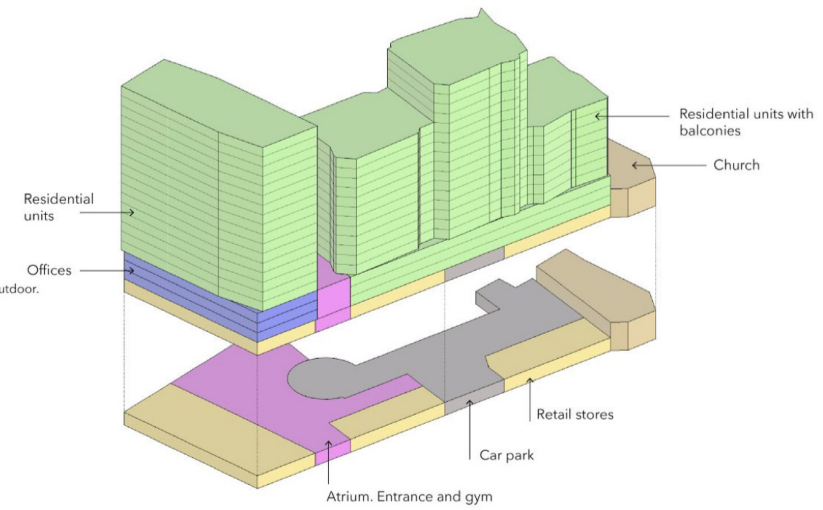
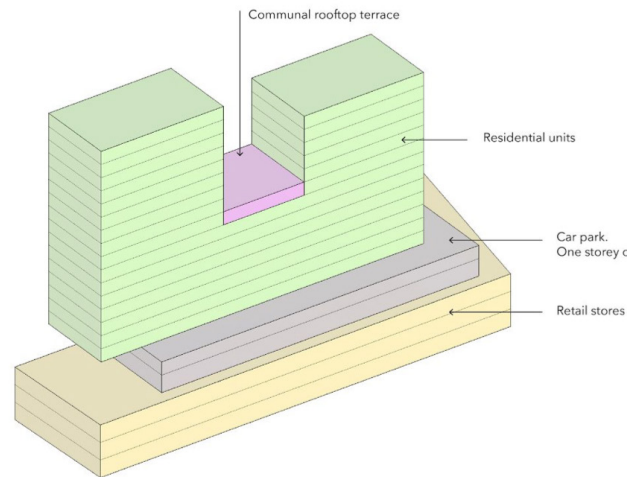
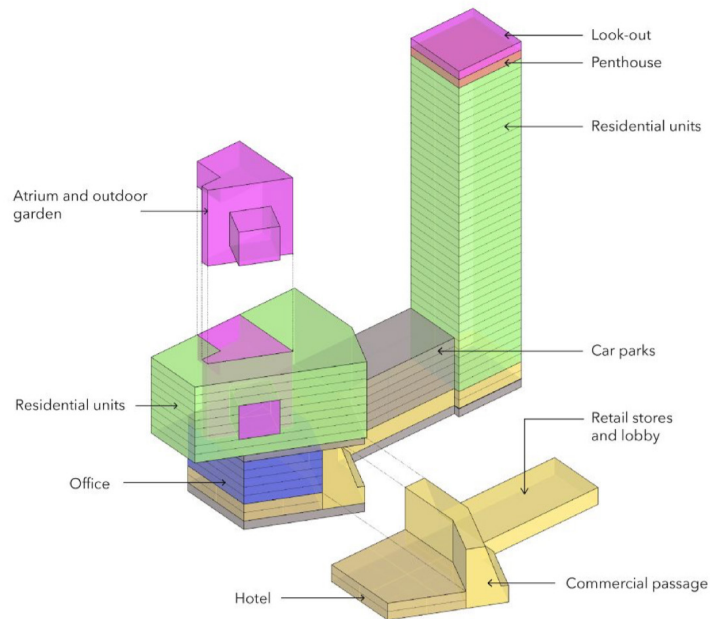
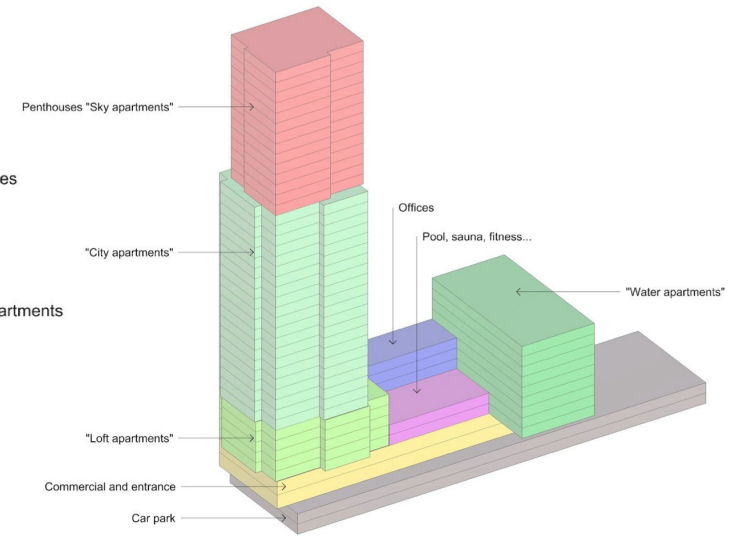
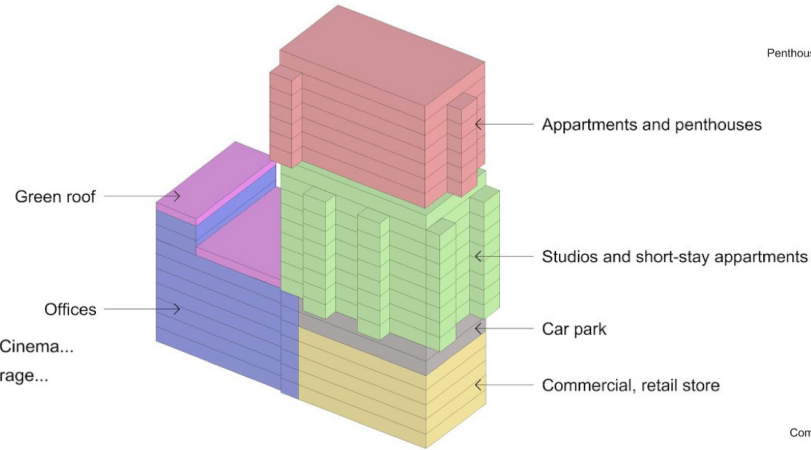
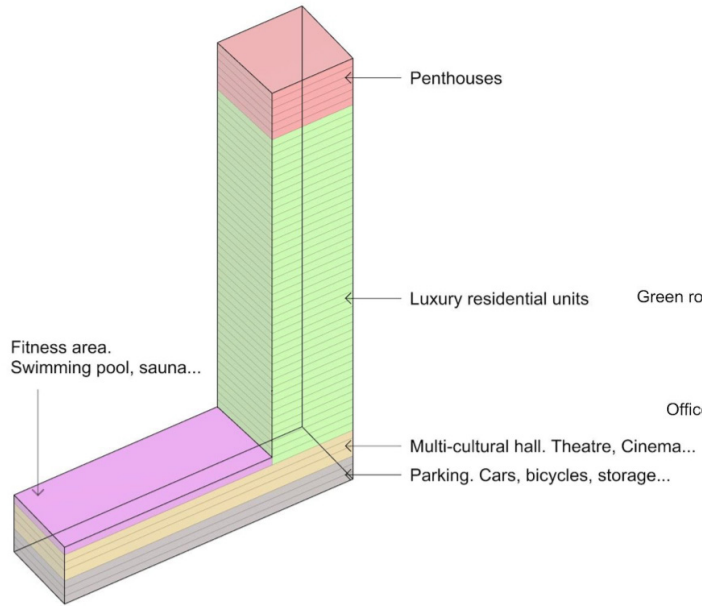


## RECENT RESIDENTIAL HIGH-RISE





# PROGRAMME



## PROGRAMME

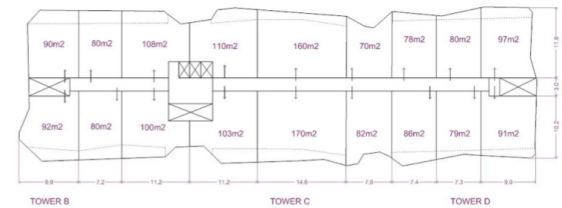
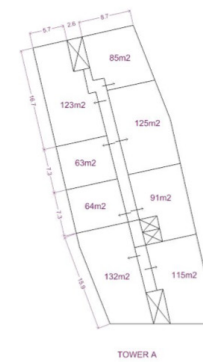
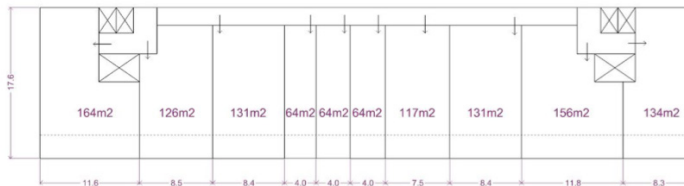
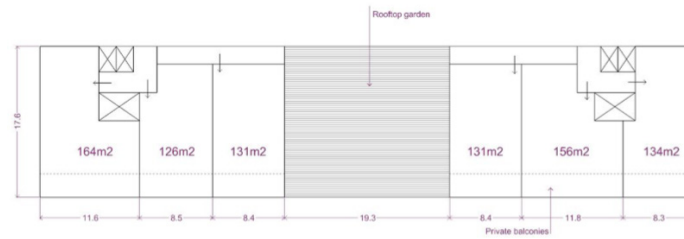
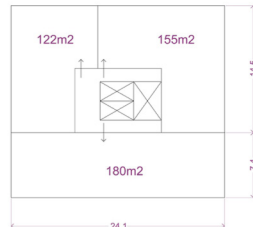
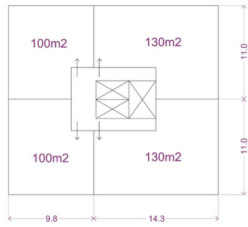
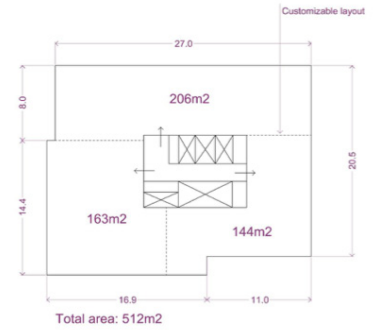
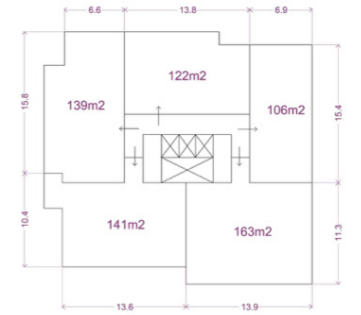
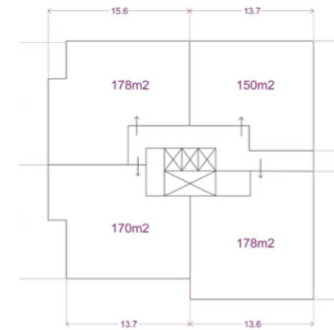
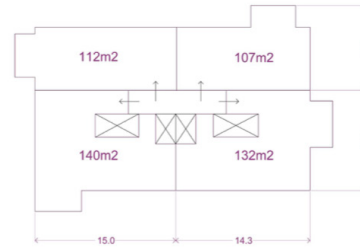
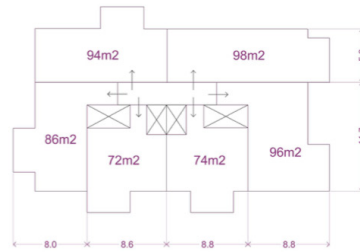
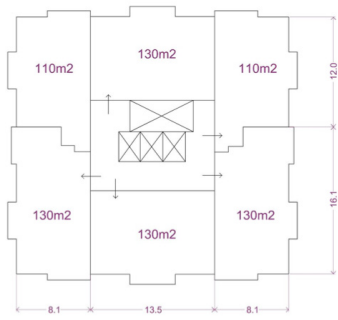
Increase exclusivity and luxury towards the top

Communal spaces. Indoor or outdoor

Commercial and multi-use plinth

Car-park above commercial plinth

# FLOOR LAYOUT





## FLOOR LAYOUT

High variety of housing units

80-170m<sup>2</sup>

Increase size units towards the top

Efficient vertical transportation

## URBAN

Located in high-rise zone  
Minimum shadow impact

## PLINTH

Respect fabric continuity  
*(occupy full plot)*  
20-25 meters high  
Urban-attractive programme  
*(Retail or cultural programme)*

## VOLUME

50% max volume above plinth  
Area: 900m<sup>2</sup>  
Depth: 30m  
Diagonal 42m

## ARCHITECTURAL

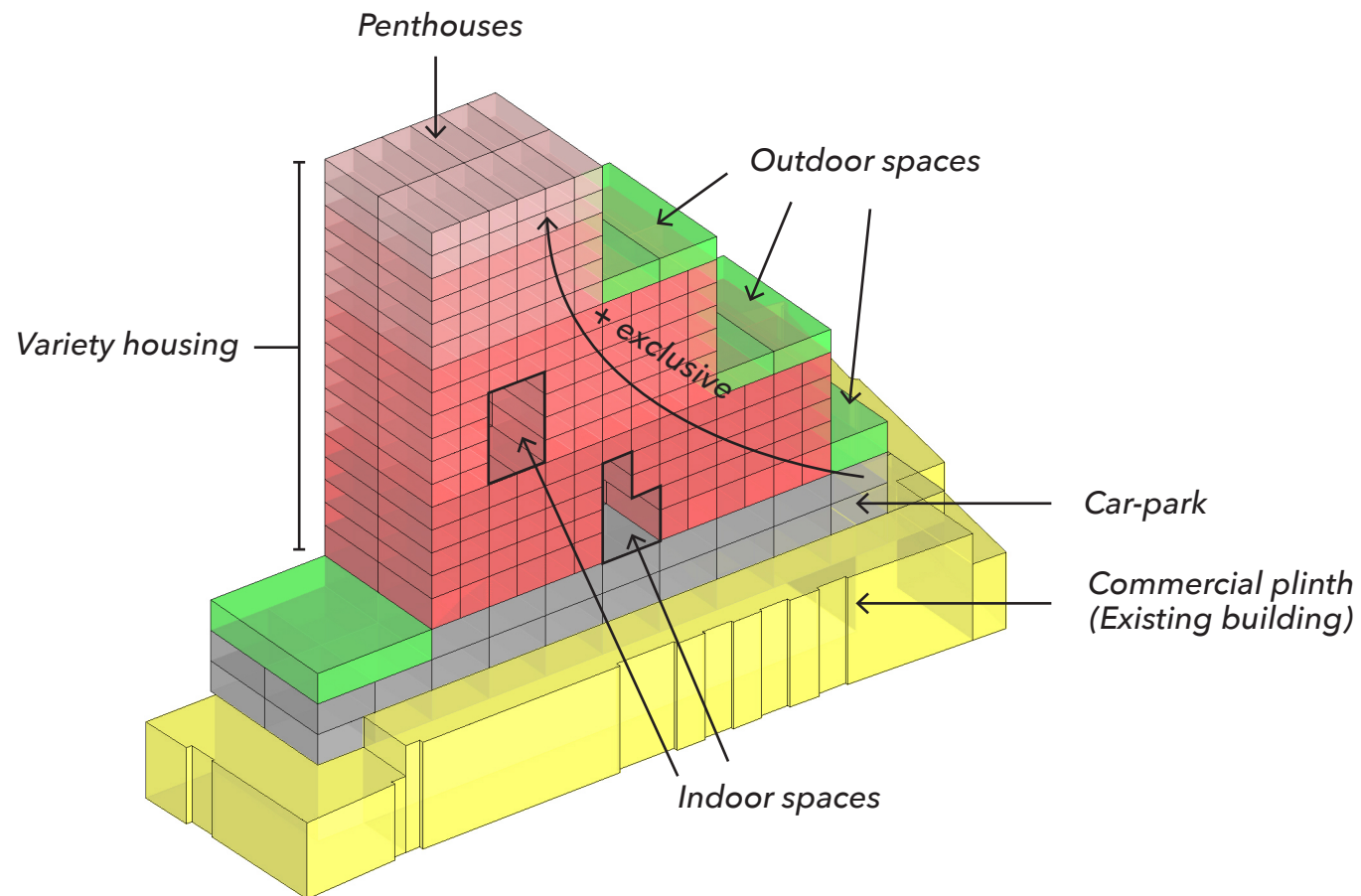
Green rooftops

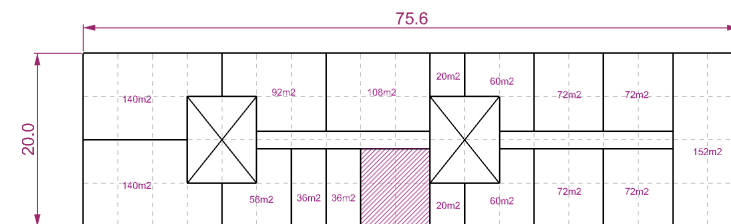
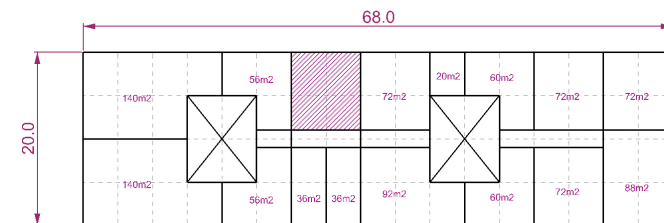
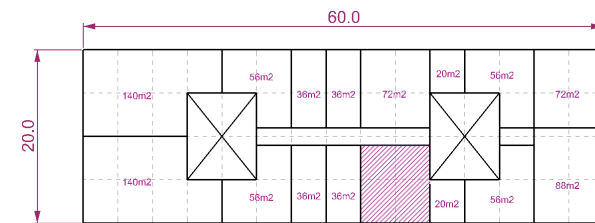
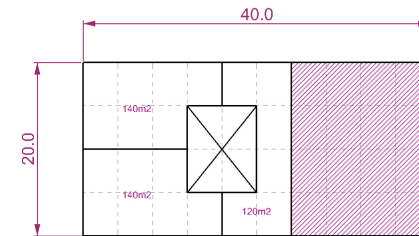
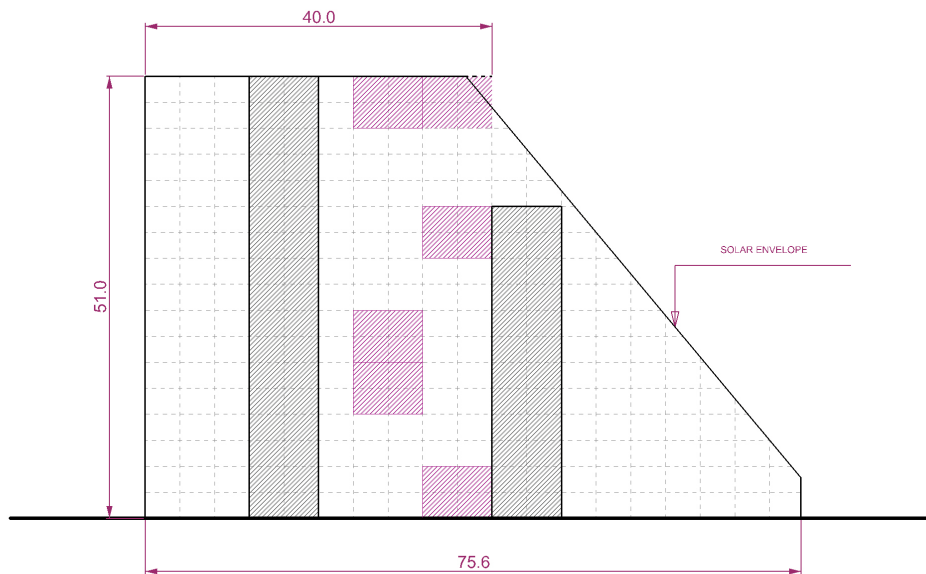
## PROGRAMME

Increase exclusivity and luxury towards the top  
Communal spaces. Indoor or outdoor  
Commercial and multi-use plinth  
Car-park above commercial plinth

## FLOOR LAYOUT

High variety of housing units  
80-170m<sup>2</sup>  
Increase size units towards the top  
Efficient vertical transportation





## STRUCTURE STUDY



## STRUCTURAL DESIGN

TIMBER  
MODIFICATION  
FACTORS

Humidity  
Load duration

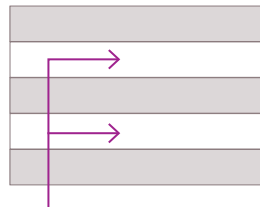
$k_{mod}$   
 $k_{def}$

STRENGTH

CREEP

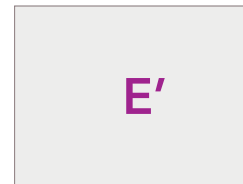
CLT  
CROSS-SECTION

ORIGINAL



No contribution  
 $E_{90} = 1/30 E_0$

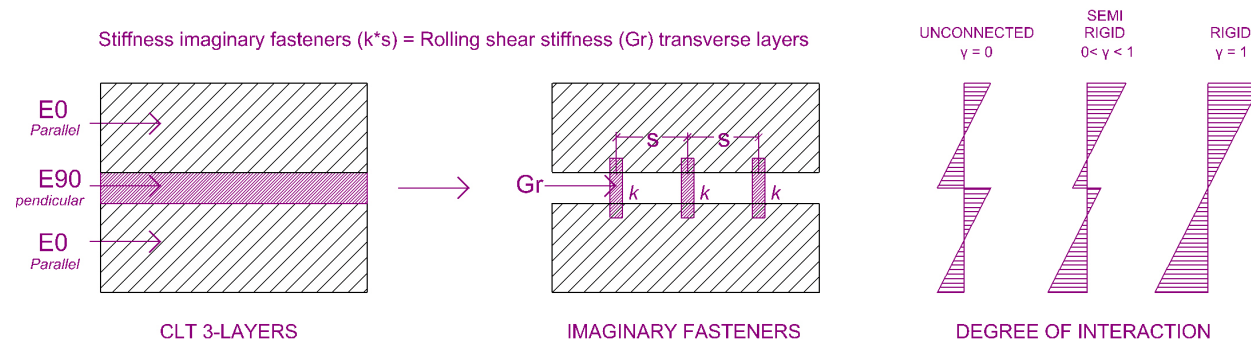
HOMOGENISATION



EQUIVALENT  
B. STIFFNESS

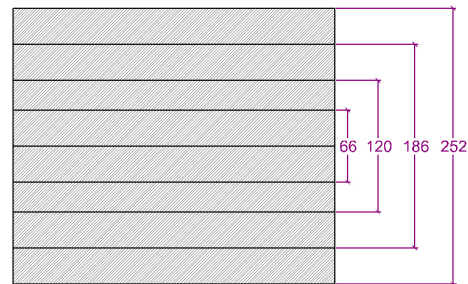
## INTERACTION LAYERS

80% DECREASE IN BENDING STIFFNESS



## COMPOSITE THEORY

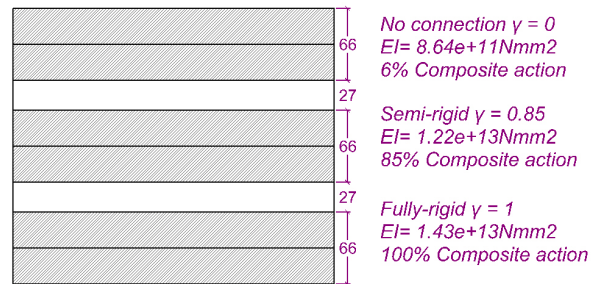
(Blass 2005)



$K_1 = 0.73$   
 Bending stiffness=  $1.14e+13 \text{ Nmm}^2$   
 79% Composite action

## GAMMA METHOD

(Eurocode 5)



$\gamma = 0.96$   
 Bending stiffness=  $1.40e+13 \text{ Nmm}^2$   
 97% Composite action



## FLOOR SYSTEM

## 190mm CLT - 8 meters SPAN

## NORMAL STRESSES

## SHEAR STRESSES

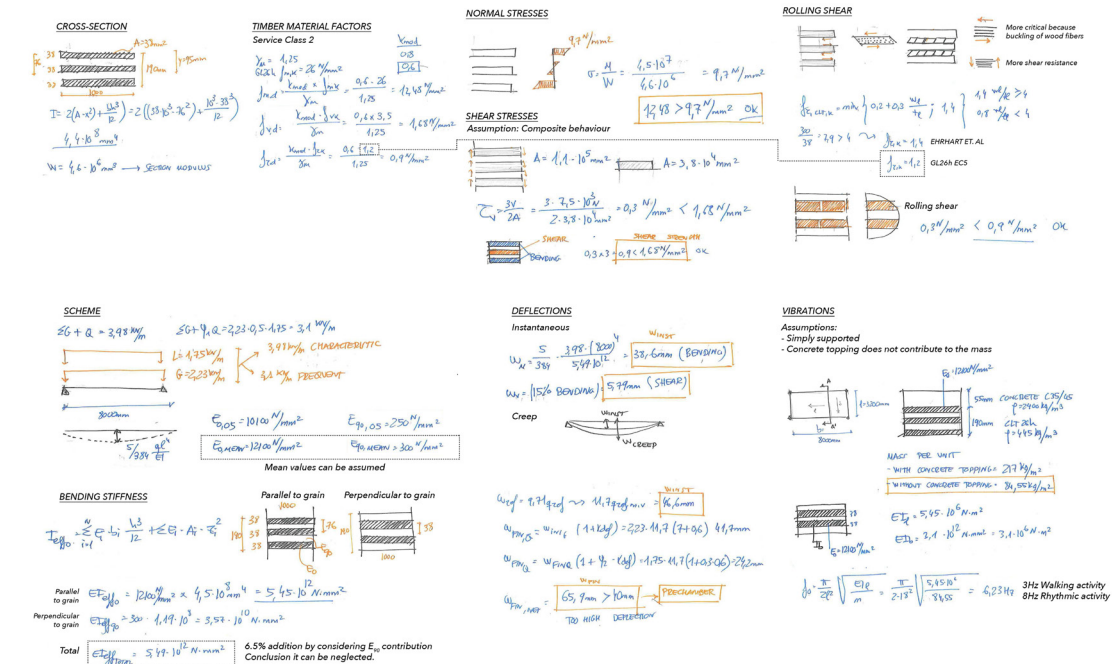
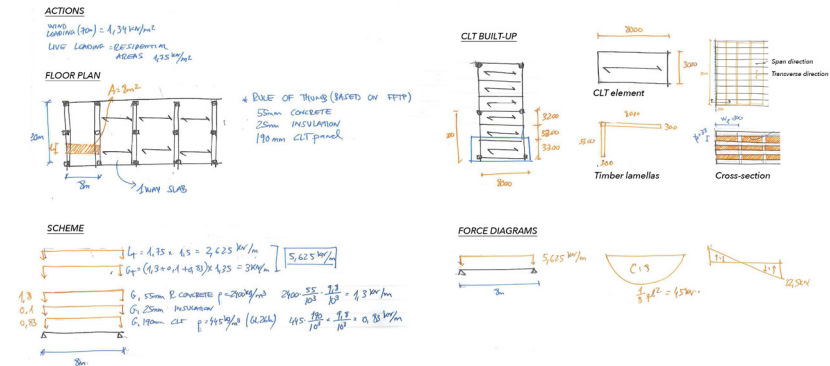
**NOT OK**

## ROLLING SHEAR

## DEFLECTION

## VIBRATION

# FIRE



## FLOOR SYSTEM



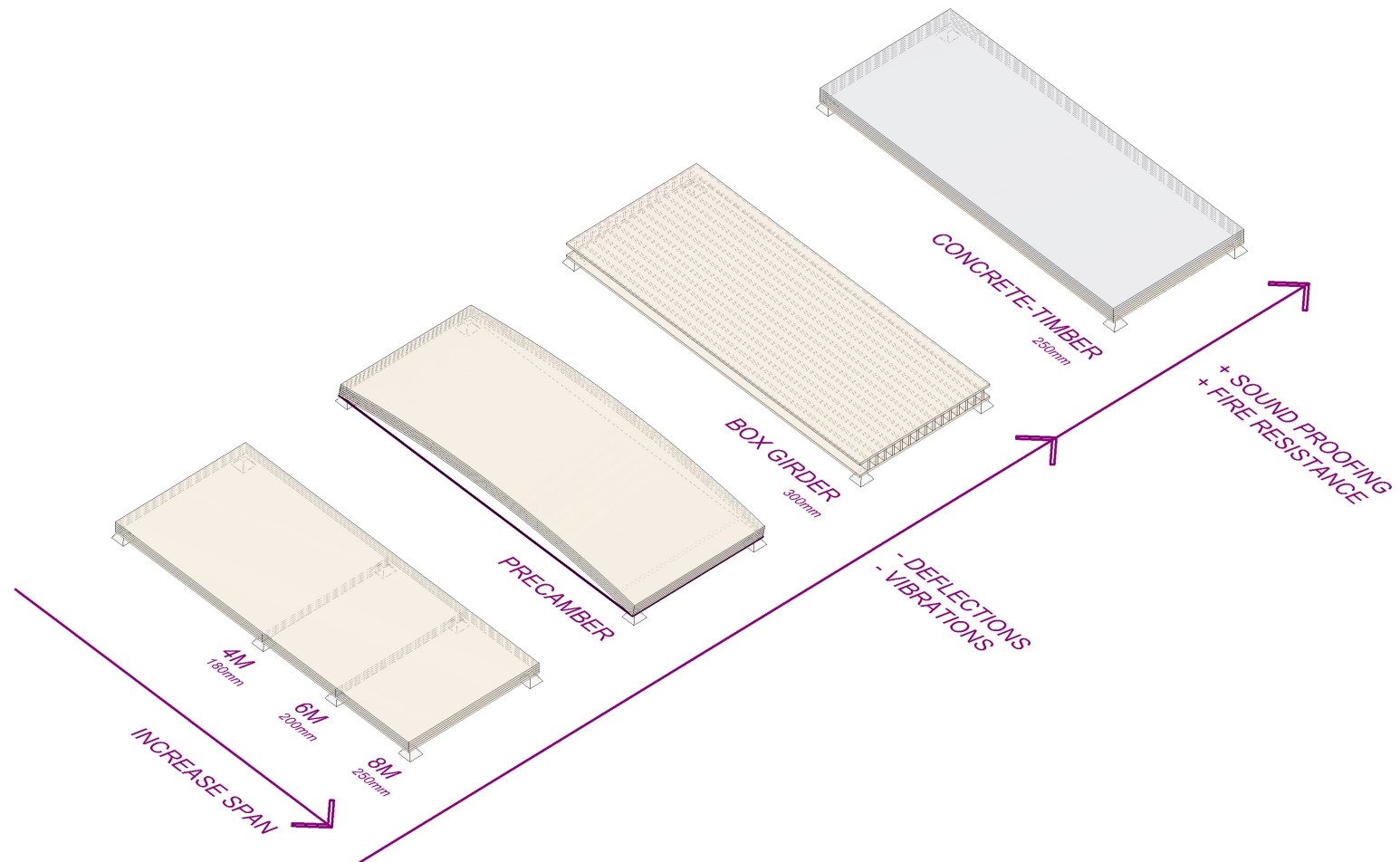
REDUCE SPAN



INCREASE FLOOR DEPTH

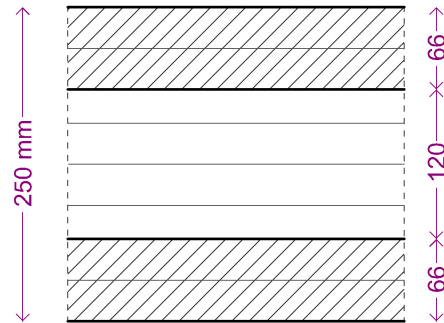


STIFFER MATERIAL



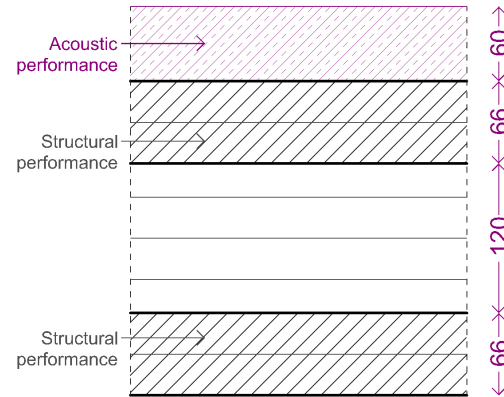
## FLOOR SYSTEM

8m span



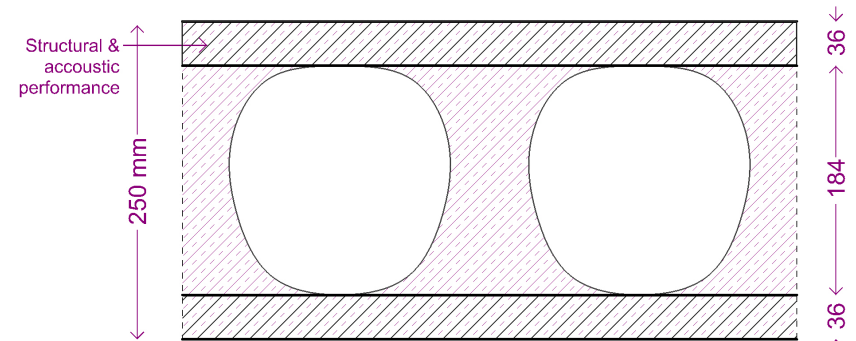
250mm CLT

21.66mm



250mm CLT + Concrete

31.02mm **X**



Concrete hollow core

19.04mm

REDUCE SPAN

ADDITIONAL  
LAYERS

DEFLECTION

VIBRATION

FIRE

SOUND



## FLOOR SYSTEM

### CONCRETE + TIMBER COMPOSITE

50 - 100 mm concrete topping + shear connectors

50% REDUCTION DEFLECTIONS

CONTRIBUTION TO FIRE AND SOUND PERFORMANCE



STRUCTURAL  
CONTRIBUTION

50 mm

CONCRETE TOPPING  
IN TIMBER FLOORS

*Acoustic + fire resistance*

COMPRESSION  
Concrete

CONCRETE  
NOT EFFICIENT

TENSION  
Steel rebars/Timber

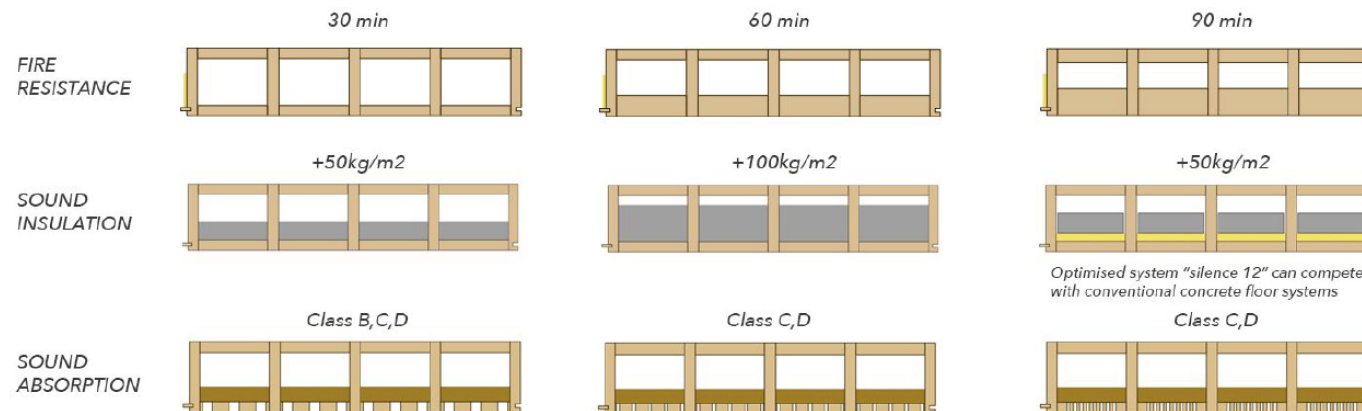
TENSION CRACKS

*Corrosion + moisture in steel rebars*

# FLOOR SYSTEM

## LIGNATUR

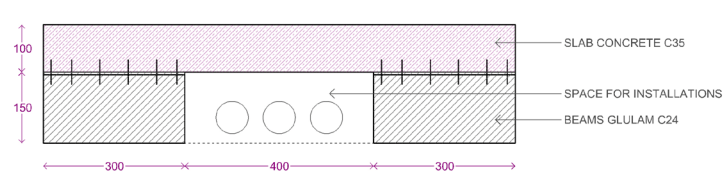
	<u>CLT</u>	<u>LIGNATUR</u>
STRUCTURE	✓	✓
SOUND	✗	✗
FIRE	✗	✗✗
CUSTOMISATION	✗	✓



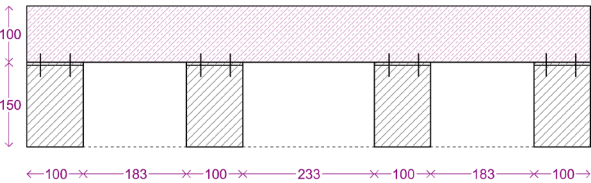
FLOOR SYSTEM

150MM GLULAM + 100 CONCRETE

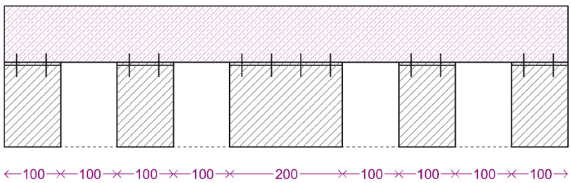
SOUND ✓  
FIRE ✓  
STRUCTURE ✗



24 mm



40.4 mm



29 mm

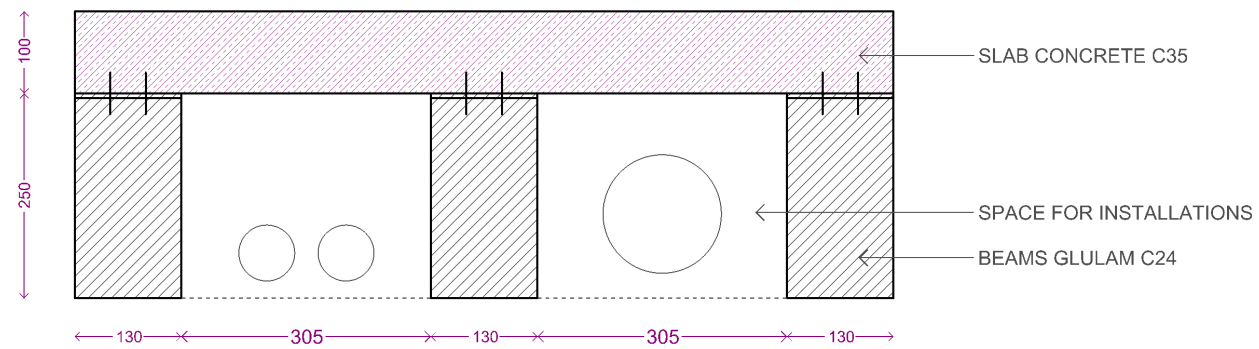
## FLOOR SYSTEM

250MM GLULAM + 100 CONCRETE

SOUND ✓

FIRE ✓

STRUCTURE ✓



## FLOOR SYSTEM

100MM CLT - 3M SPAN

250MM CLT - 6M SPAN

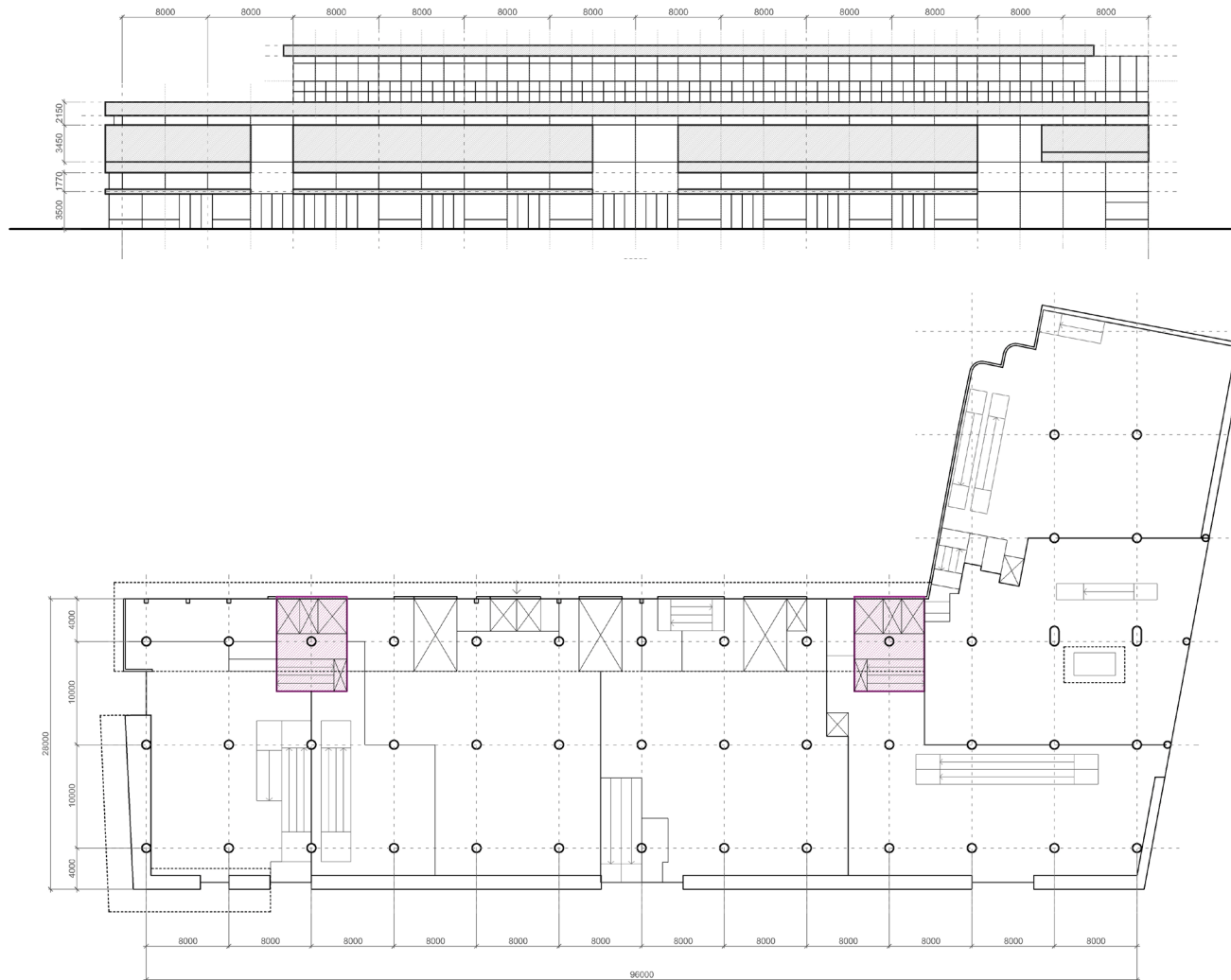
COMPOSITE GLULAM + TIMBER - 8M SPAN

ADDITIONAL LAYERS FOR SOUND & FIRE



**TO WHAT EXTENT THE EXISTING BUILDING  
AFFECTS THE STRUCTURE OF THE TTE?**

## EXISTING BUILDING



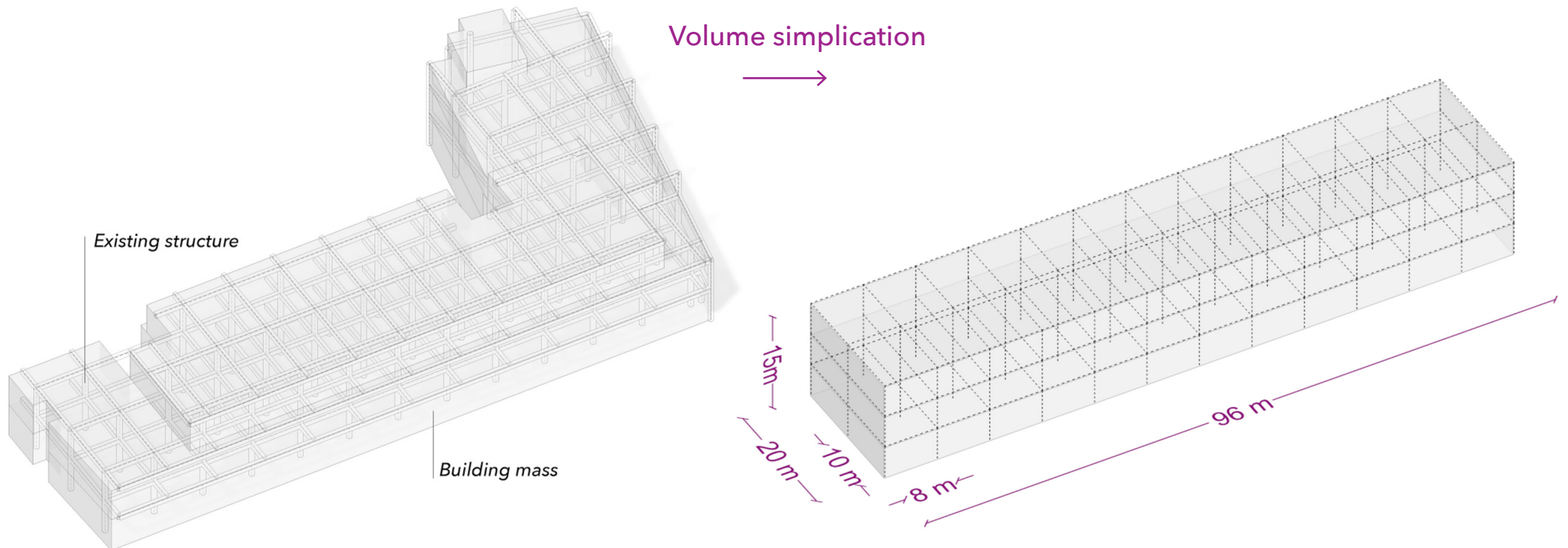
## TER MEULEN

Heritage for city of Rotterdam

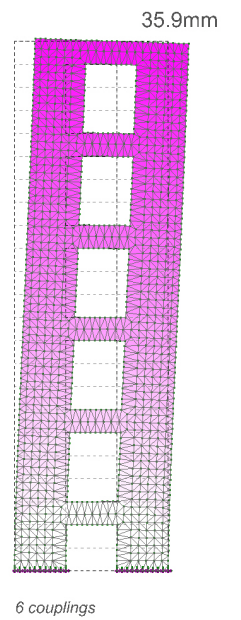
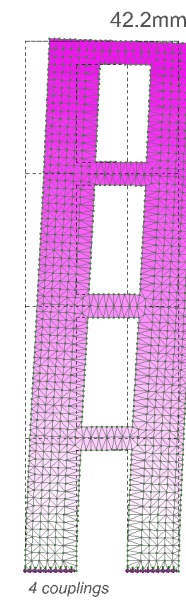
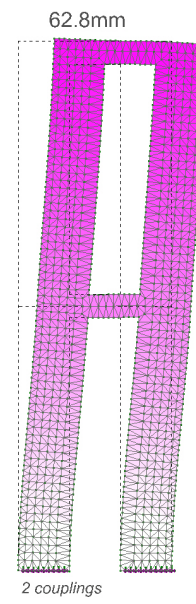
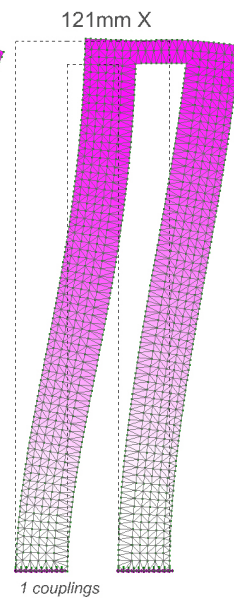
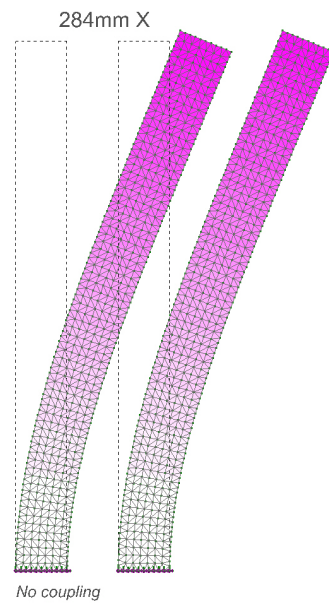
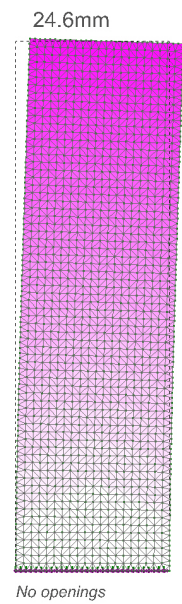
Important commercial location

Boundaries 96x20m

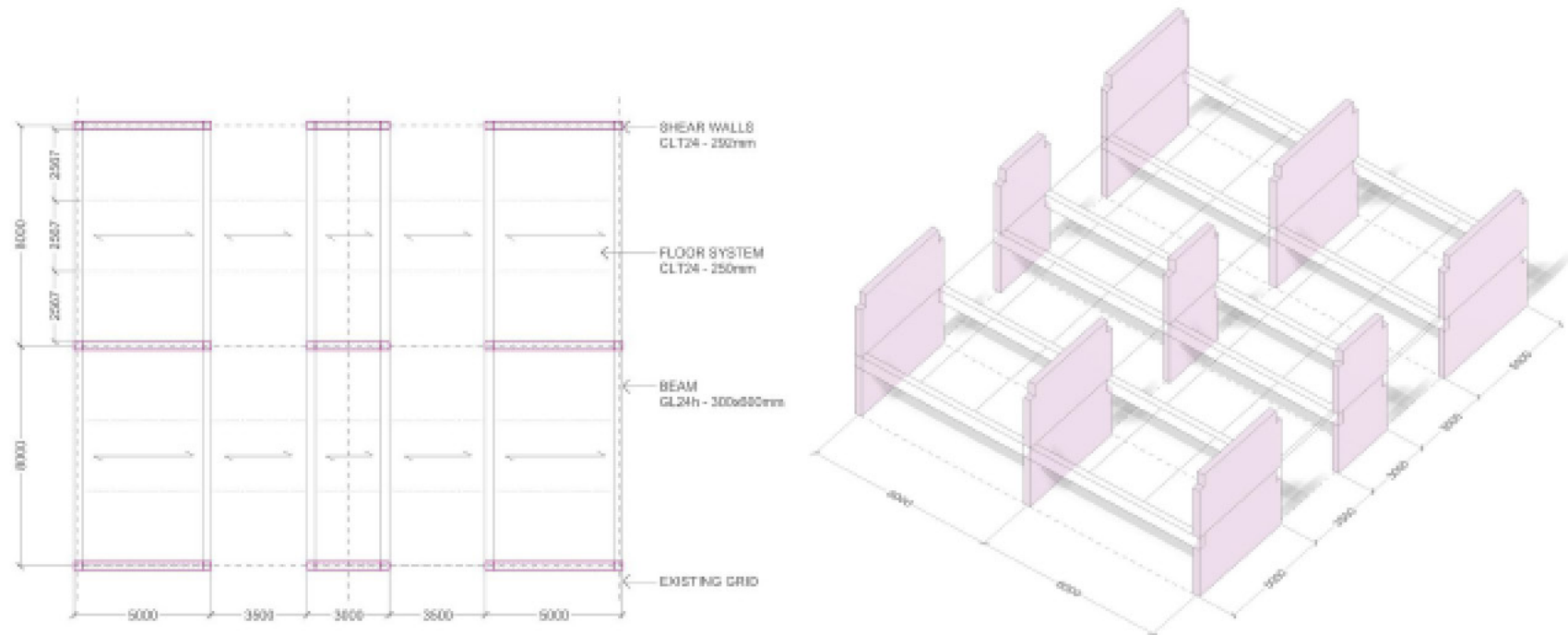
Existing grid 8x10m



## LATERAL SYSTEM



## 26 shear walls - 5 meters + 13 shear walls - 3 meters



Floor plan and 3D scheme with uncoupled shear walls (Own elaboration)

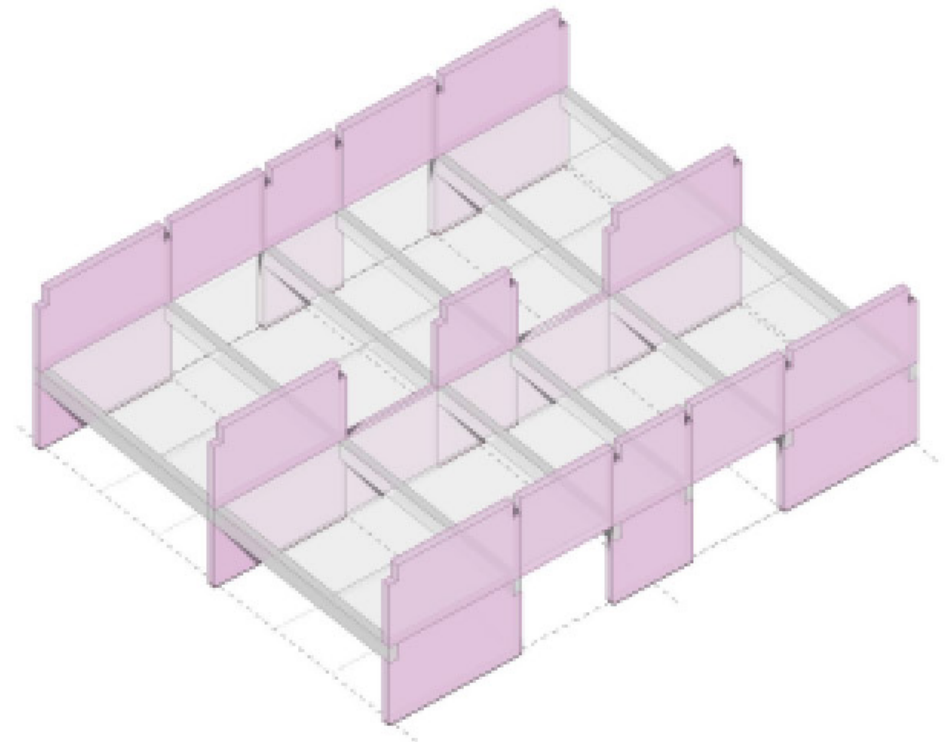
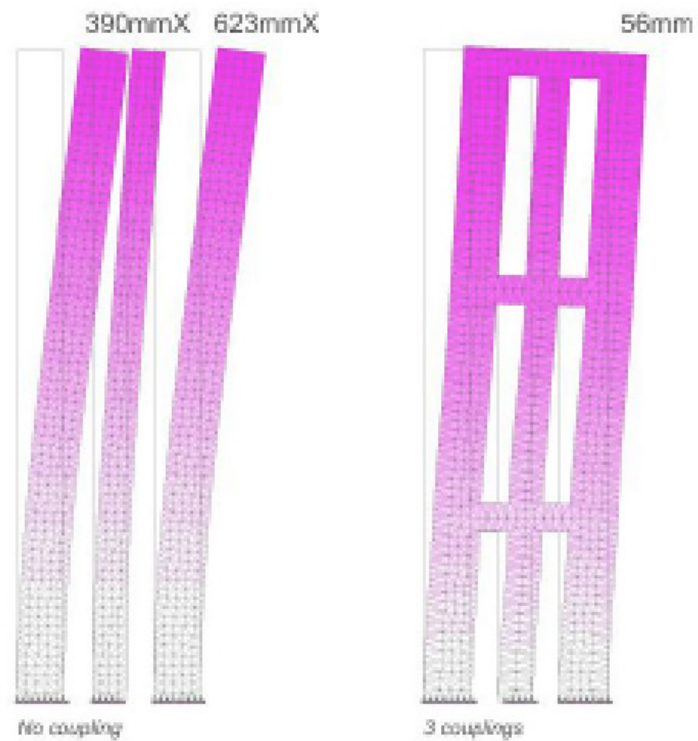
- Wind load distribution of each separate shear wall is proportional to the bending stiffness

Shear wall	Second moment of area Nmm <sup>2</sup>	Wind loading	Deflections (mm)
5 meters	2.06e+12	45%	623mm X
3.5 meters	4.5e+11	10%	390 X

Need at least three couplings (storey height) along height of the building.

## Couplings

### *Alternate couplings in different storeys*

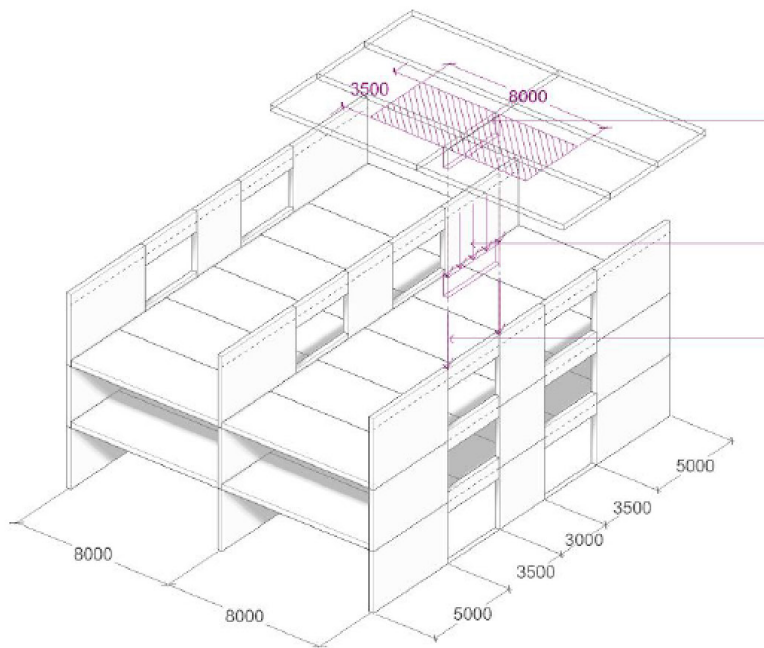


Storey height couplings (Own elaboration)



## LINTEL DESIGN

3.5m span

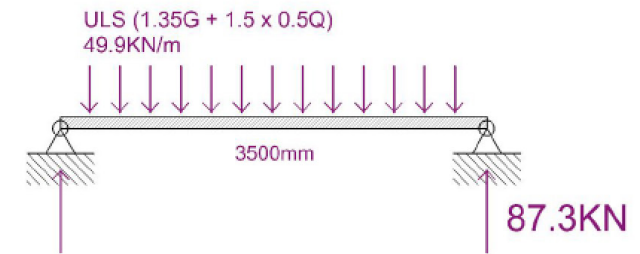


$G = 3.65 \text{ kN/m}^2$   
 $L = 1.75 \text{ kN/m}^2$

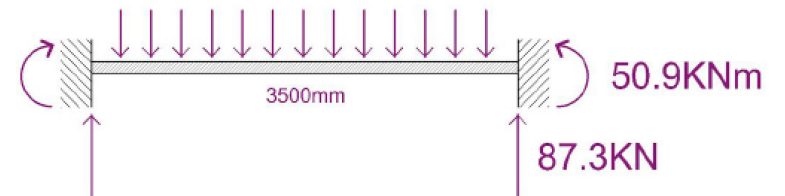
$G = 29.2 \text{ kN/m}$   
 $L = 14 \text{ kN/m}$

$G = 51.1 \text{ kN}$   
 $L = 24.5 \text{ kN}$

SUPPORTED  
Beam element



CLAMPED  
Same element  
with wall panel





## LINTEL DESIGN

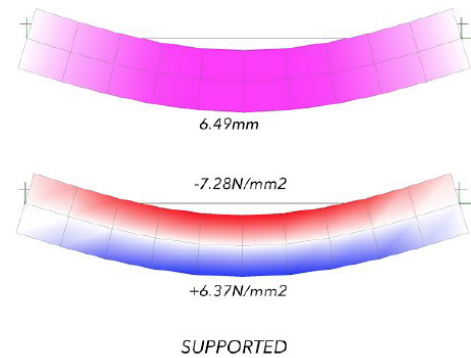
3.5m span

### 500-600mm DEPTH

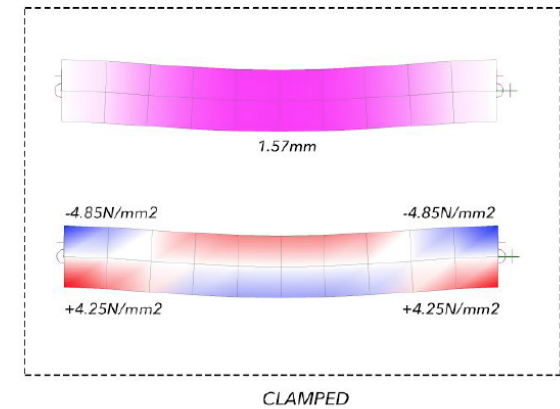
Fully rigid connections are very complex and costly

Simply supported easier option

Intermediate "partly rigid" stiffness = 0.5



Less deflections and stresses  
However, full connections are usually very difficult and costly to achieve in practice, especially in timber

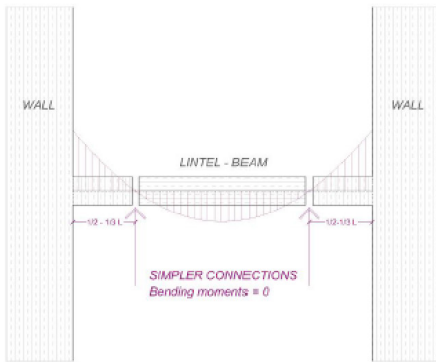


Depth of lintel beam	Bending stiffness (reducing 80% composite action)	Final deflections (including creep) SIMPLE SUPPORTED	Final deflections (including creep) CLAMPED
1000mm	1.452e+14Nmm <sup>2</sup>	0.87mm OK	0.174mm OK
500mm	1.815e+13Nmm <sup>2</sup>	6.492mm OK	1.30mm OK
400mm	9.3+13Nmm <sup>2</sup>	13.6mm	2.72mm OK

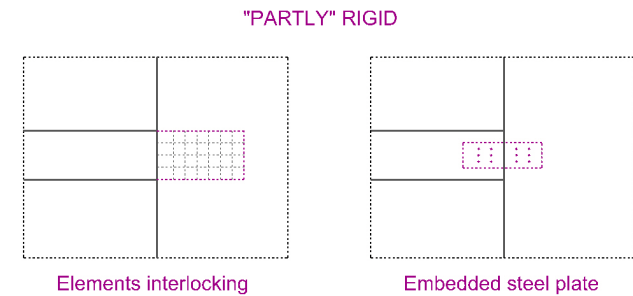
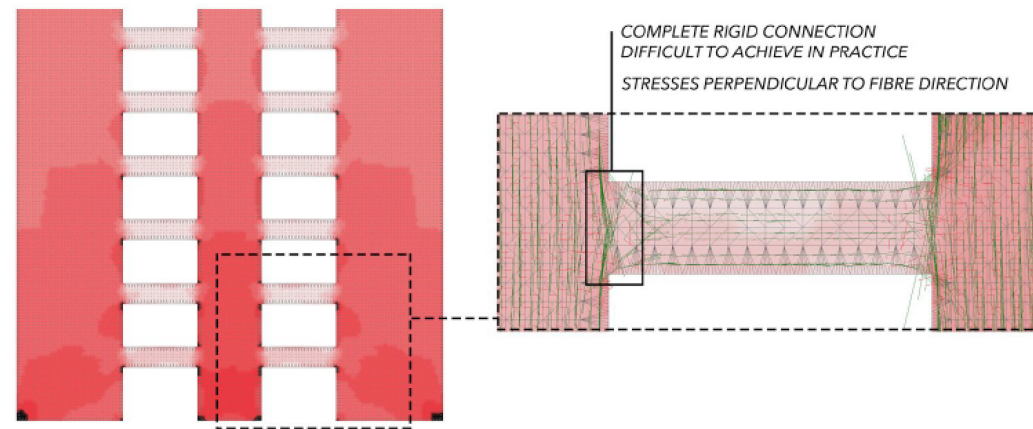
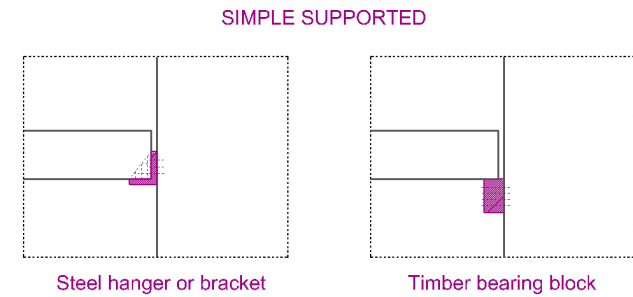
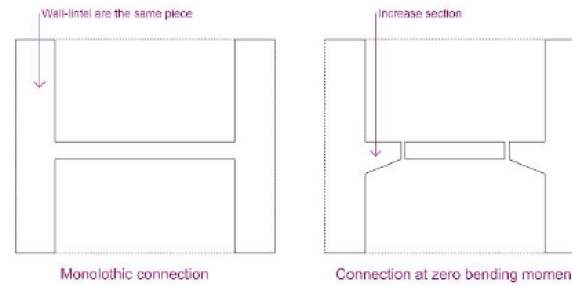
Summary of hand calculations. Deflection limit  $L/400 = 8.75\text{mm}$

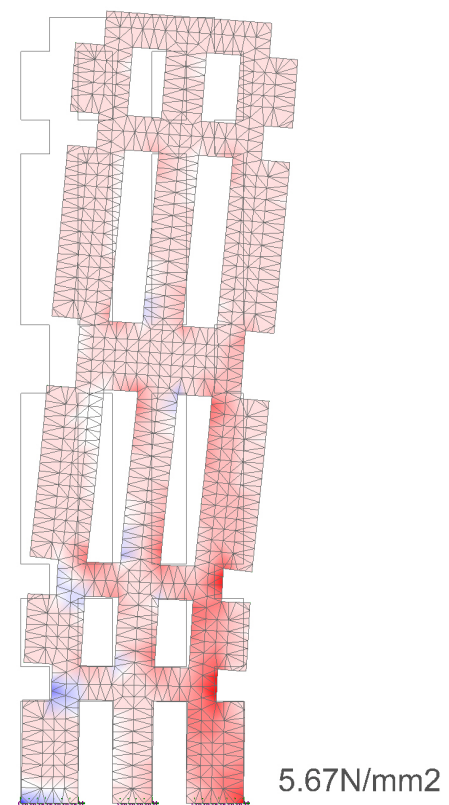
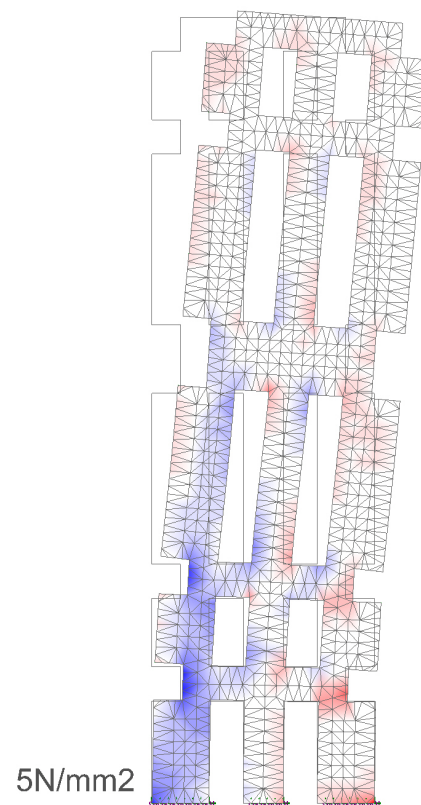
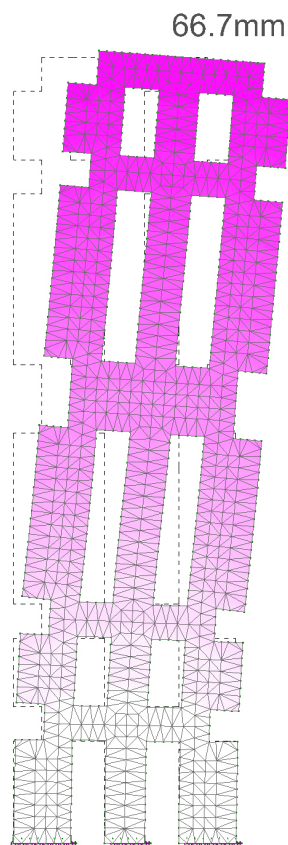
# LINTEL DESIGN

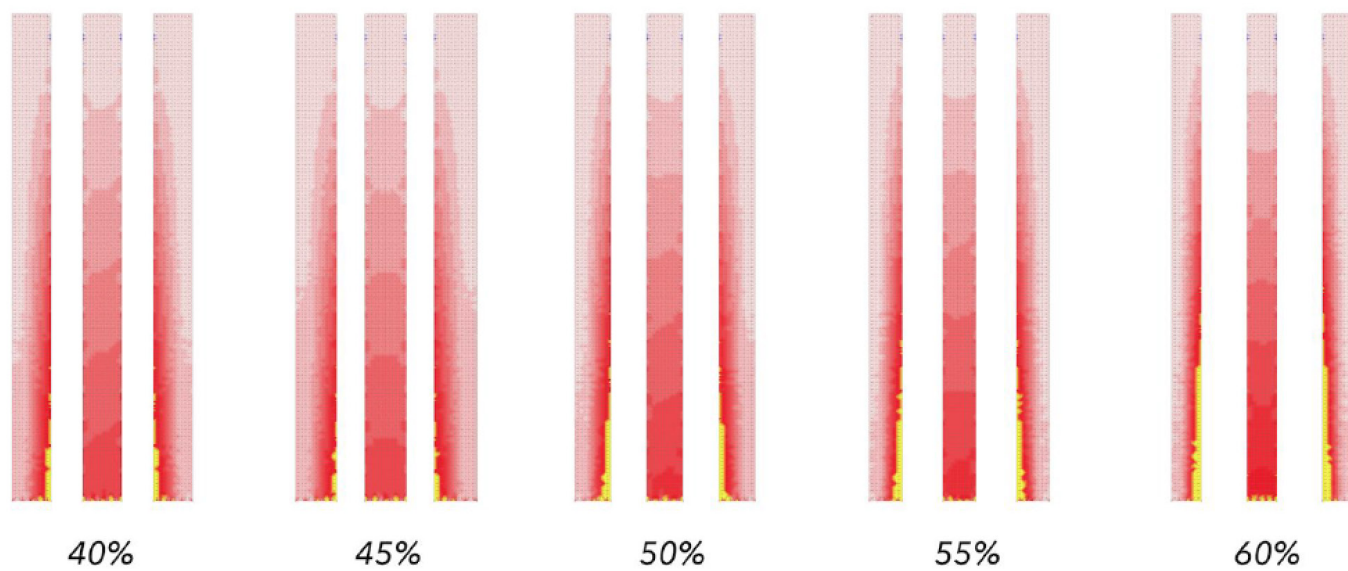
## CONNECTION OPTIONS



Monolithic lintel (left) and connection at  $L/2-3$  for the span for simpler connection







THE END