



# Dynamic Timber

*A Seismic Analysis Workflow for Tall Timber Structures  
with Variable Parameters*

# Presentation Contents

Background

# Presentation Contents

Background

Research Goals

# Presentation Contents

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Project Scope

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Research Goals

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The Workflow

# Presentation Contents

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Research Goals

Project Scope

The Workflow

Case Study

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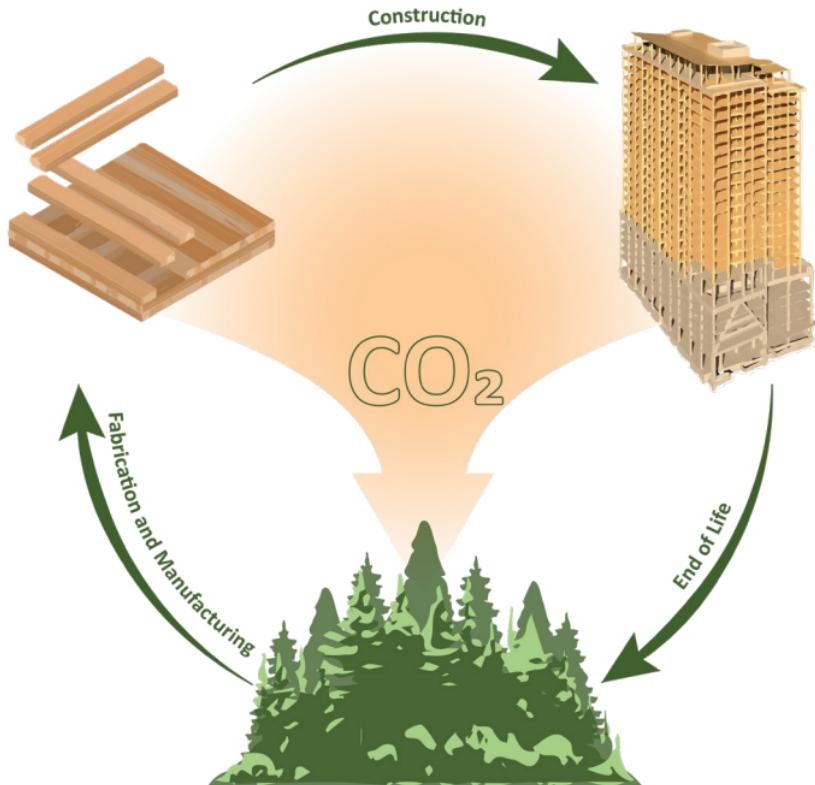
Conclusion

# Background

# Why Timber?

## Carbon Neutral

- Carbon sequestration
- $1\text{m}^3$  of wood = 1.1 tons of  $\text{CO}_2$



# Why Timber?

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## Occupant Health

- Natural materials make for healthier spaces



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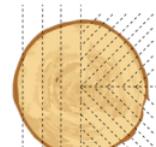
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## New Engineered Products



Planing or Quartering



Solid Sawn Lumber



Cross-Laminated Timber



Glue-Laminated Timber



Slicing



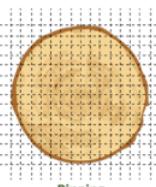
Timber Veneer



Mass Plywood Panels



Laminated Veneer Lumber



Ripping



Timber Strands



Laminated Strand Lumber



Parallel Strand Lumber

# Why Timber?

## Carbon Neutral

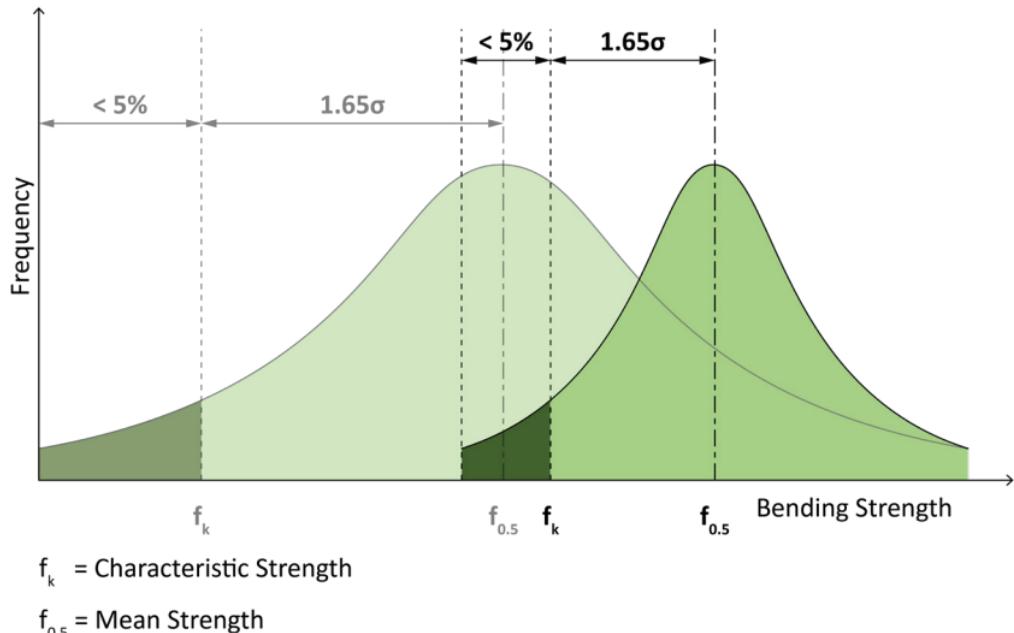
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- Elements are more reliable



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## New Engineered Products

- Elements are more reliable
- New structure types possible

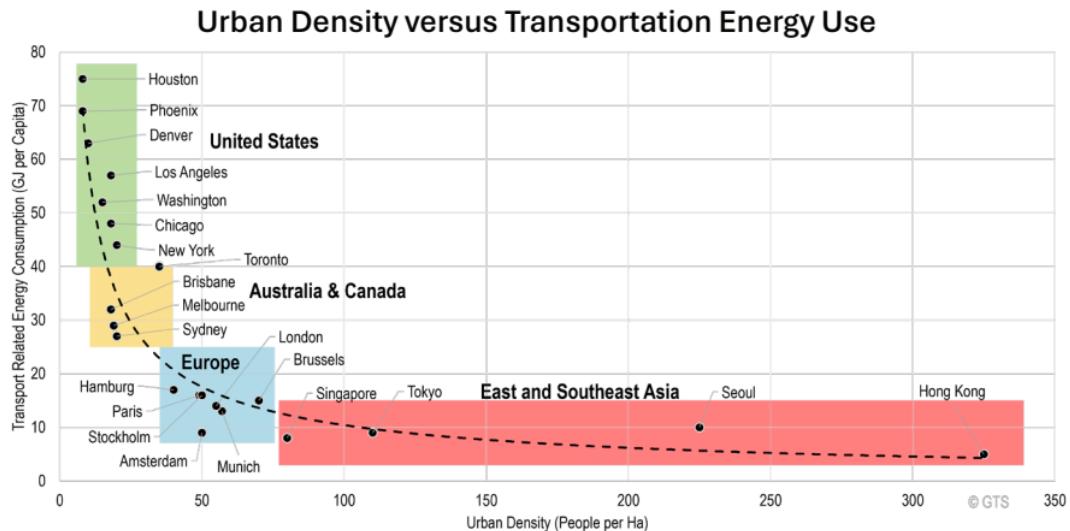


Mjøstårnet - Brumunddal, Norway - Voll Arkitekter

# Why Tall Timber?

## Increased Density

- Higher density cities use less transportation energy



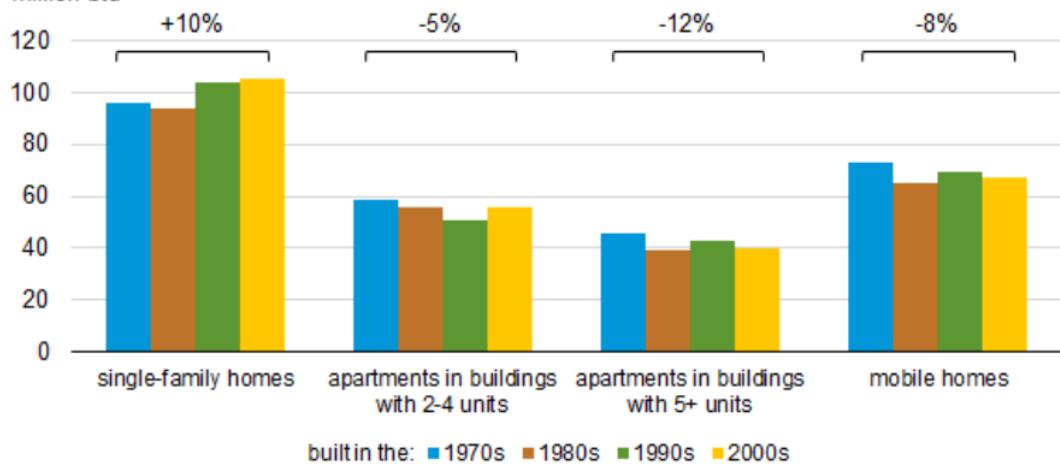
Source: *The Geography of Transportation Systems*

# Why Tall Timber?

## Increased Density

- Higher density cities use less transportation energy
- High density living has lower emissions per floor area

Site energy use per household in 2009 by year of construction  
million btu

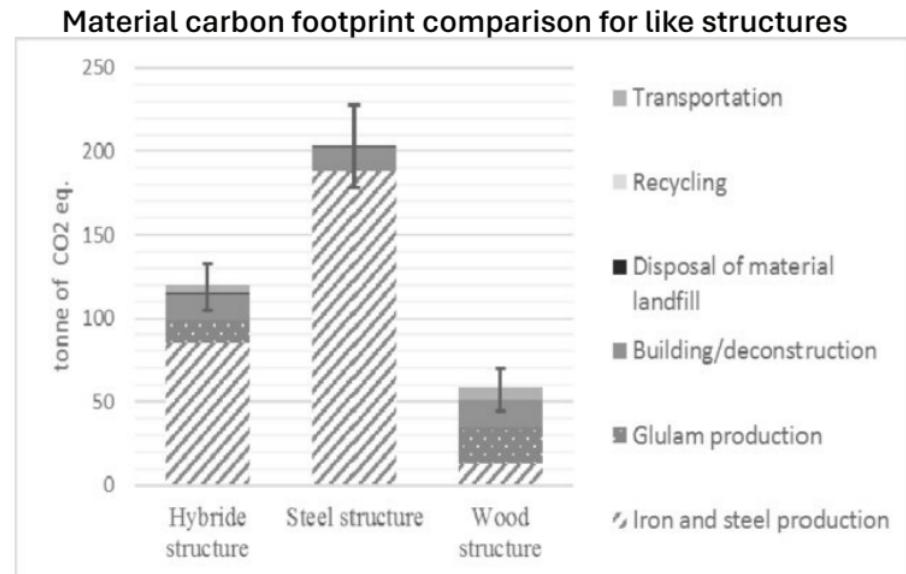


Source: US Energy Information Administration

# Why Tall Timber?

## Increased Density

- Higher density cities use less transportation energy
- High density living has lower emissions per floor area
- Need low carbon solutions for such structures



Source: Laurent et al. 2019

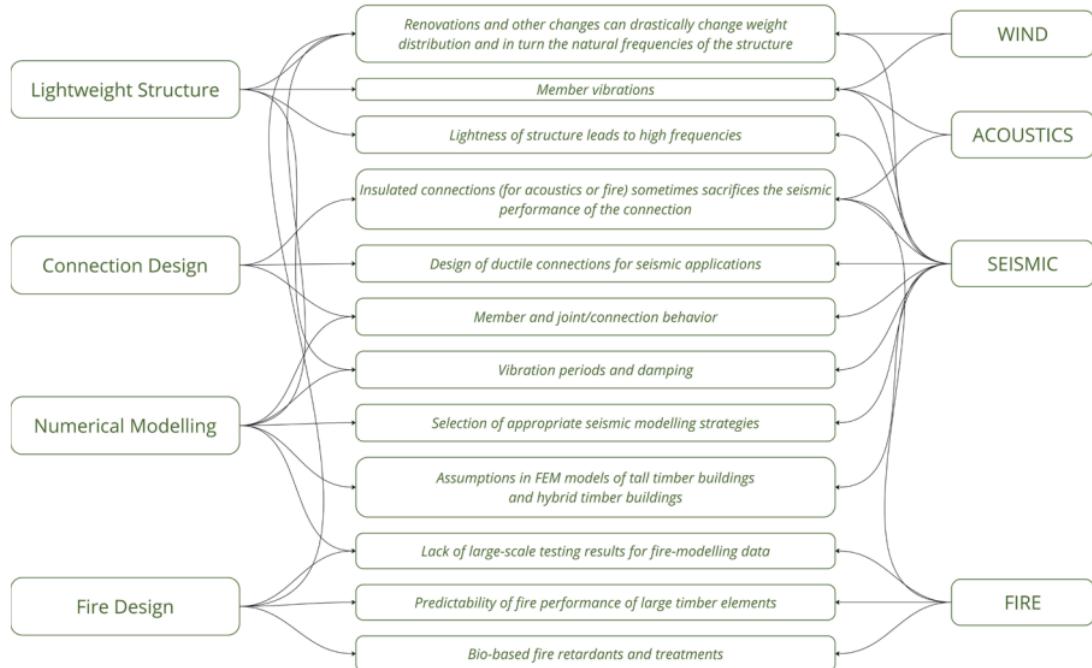
# Why Tall Timber?

## Increased Density

- Higher density cities use less transportation energy
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## Current Challenges

- Lightweight structure
- Connection design
- Computational modelling
- Fire design



# Why Seismic Analysis?

*(of tall timber structures)*

## Material Challenges

- Anisotropic material
  - behaves differently in each direction



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*(of tall timber structures)*

## Material Challenges

- Anisotropic material
  - behaves differently in each direction
- Natural material
  - subject to deterioration



# Why Seismic Analysis?

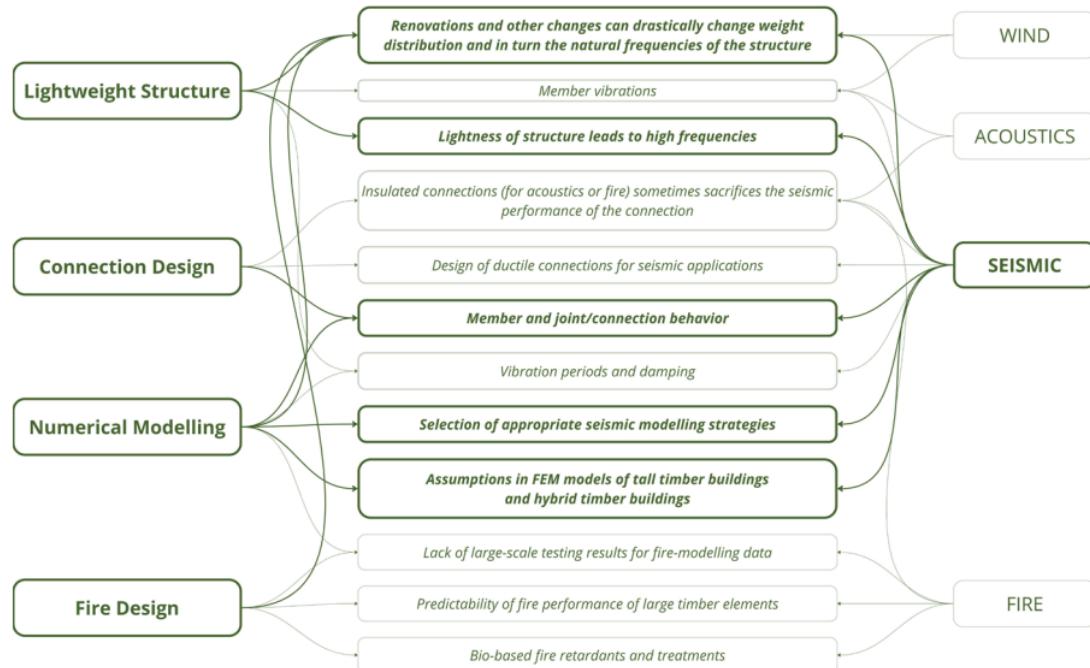
*(of tall timber structures)*

## Material Challenges

- Anisotropic material
  - behaves differently in each direction
- Natural material
  - subject to deterioration

## Tall Structure Challenges

- Relatively lightweight
- Properties can change through the structure's lifetime



# Why Seismic Analysis Workflow?

*(of tall timber structures)*

Current Modelling Strategies

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*(of tall timber structures)*

## Current Modelling Strategies

- Complex to implement
- More accurate models are time-consuming for practice



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- Disconnected from the main design workflow



# Why Seismic Analysis Workflow?

*(of tall timber structures)*

## Current Modelling Strategies

- Complex to implement
  - More accurate models are time-consuming for practice
- Disconnected from the main design workflow
- Does not consider changing parameters overtime



# Research Goals

# Problem Statement

Current **practice-oriented seismic analysis** methods for *tall timber structures* are **isolated** from the main design process, **complex to implement**, and do not consider **variability of parameters** over the structure's lifetime.

# Objective

*How will this problem be addressed by the research project?*

Develop a **practice-oriented seismic analysis** workflow for *tall timber structures* which is **integrated** into the main design process, is **quick** and **adaptable** to implement, and includes **lifetime analysis** options.

# Questions

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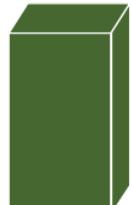
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**What is the accuracy, applicability,** What is the impact of the lifetime analysis?  
**and advantage of this workflow?** How does the workflow impact  
the decision making process for the engineer?

# Project Scope

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## *Form and Function*



Rectilinear



8+ stories



Residential

# Project Scope

## *Form and Function*



Rectilinear

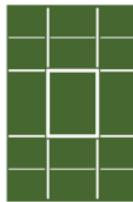


8+ stories

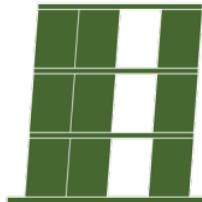


Residential

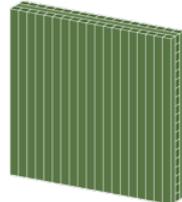
## *Structural System*



Central Core



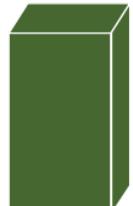
Shear Walls



CLT Walls

# Project Scope

## *Form and Function*



Rectilinear



8+ stories



Residential

## *Building Standard*



Europe Region

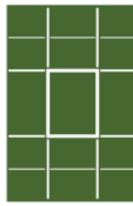


EN 1995

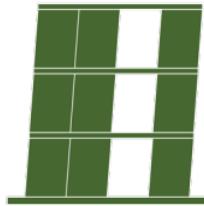


EN 1998

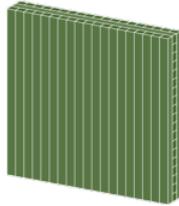
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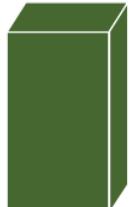
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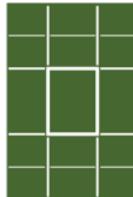


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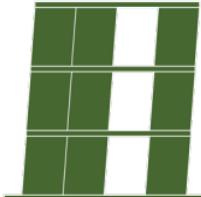


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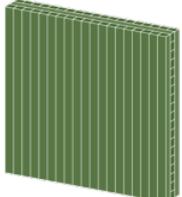
## *Structural System*



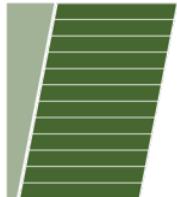
Central Core



Shear Walls



CLT Walls



Static Linear Analysis

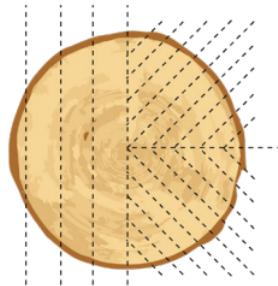


Post Event



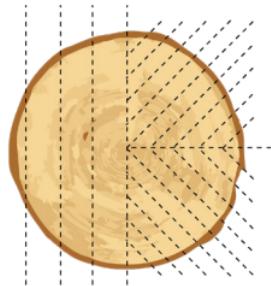
Over Time

# Cross Laminated Timber (CLT)



Planing or Quartering

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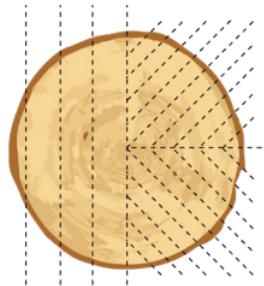


Planing or Quartering



Solid Sawn Lumber

# Cross Laminated Timber (CLT)



Planing or Quartering

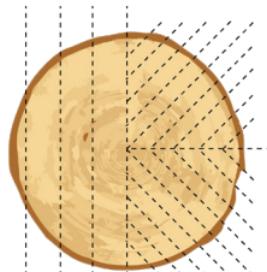


Solid Sawn Lumber



Cross-Laminated Timber

# Cross Laminated Timber (CLT)



Planing or Quartering



Solid Sawn Lumber



Cross-Laminated Timber



# CLT Wall Construction

## CLT Panels

- Single- or



# CLT Wall Construction

## CLT Panels

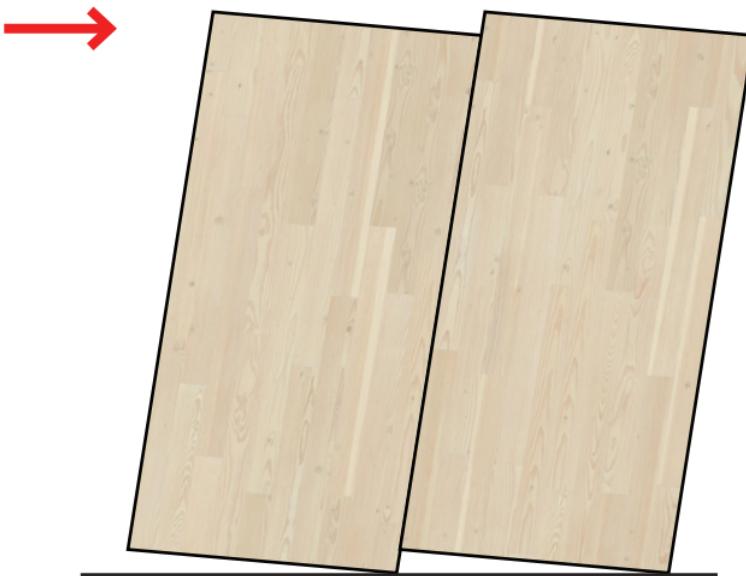
- Single- or multi-panel



# CLT Wall Construction

## CLT Panels

- Single- or multi-panel
- Work together to resist lateral loads

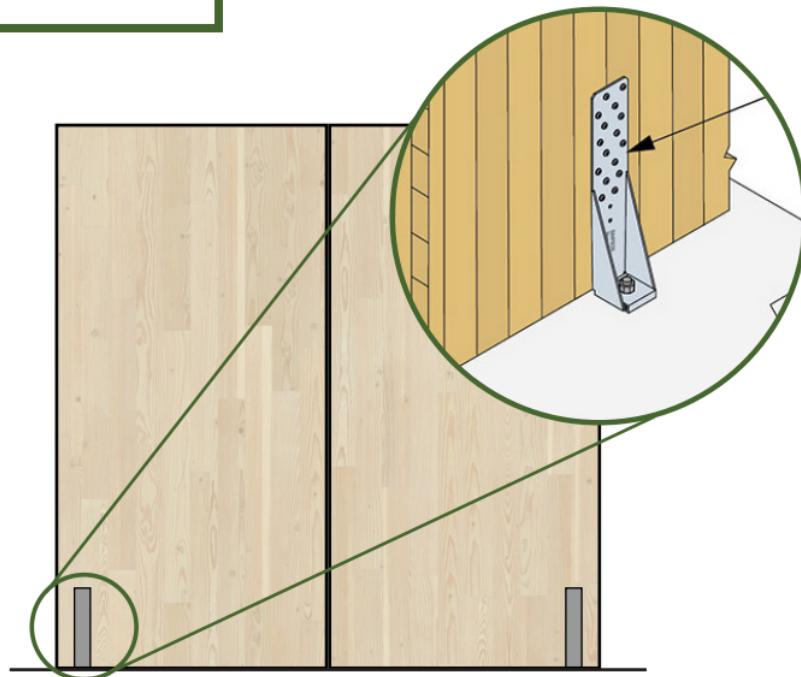


# CLT Wall Construction

## CLT Panels

- Single- or multi-panel
  - Work together to resist lateral loads

## Hold downs



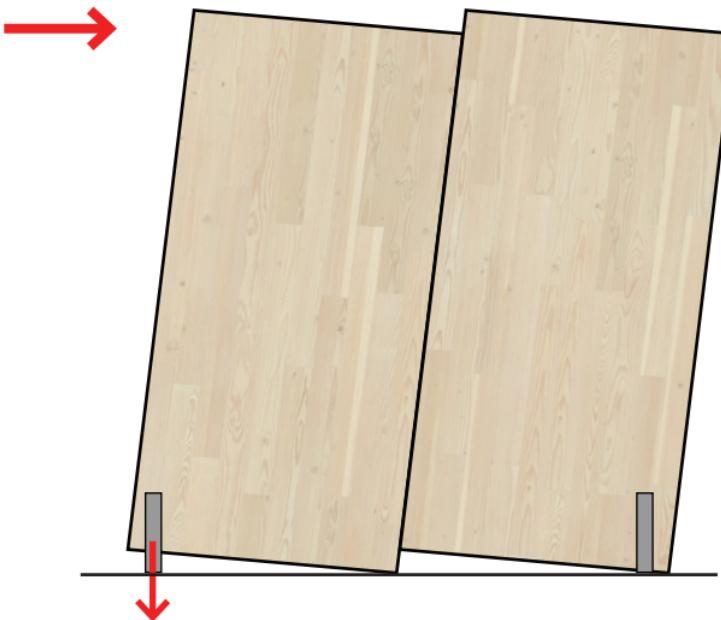
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## CLT Panels

- Single- or multi-panel
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## Hold downs

- Resist tension from overturning



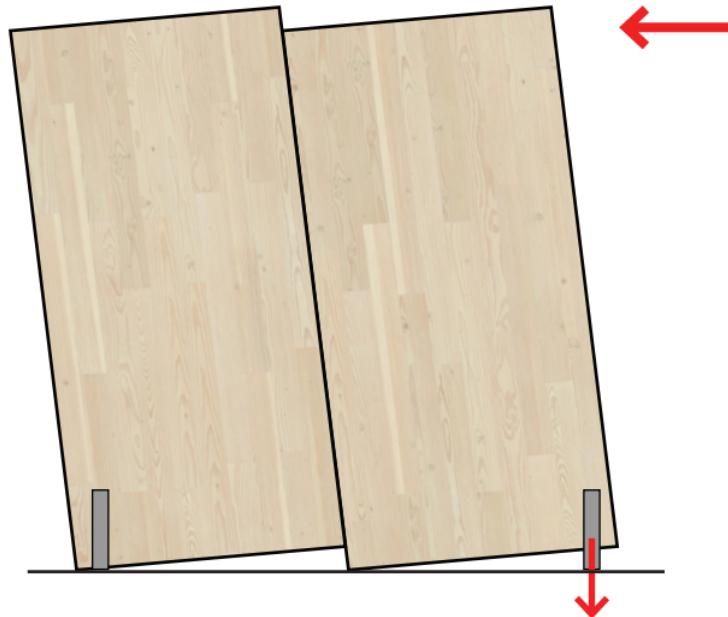
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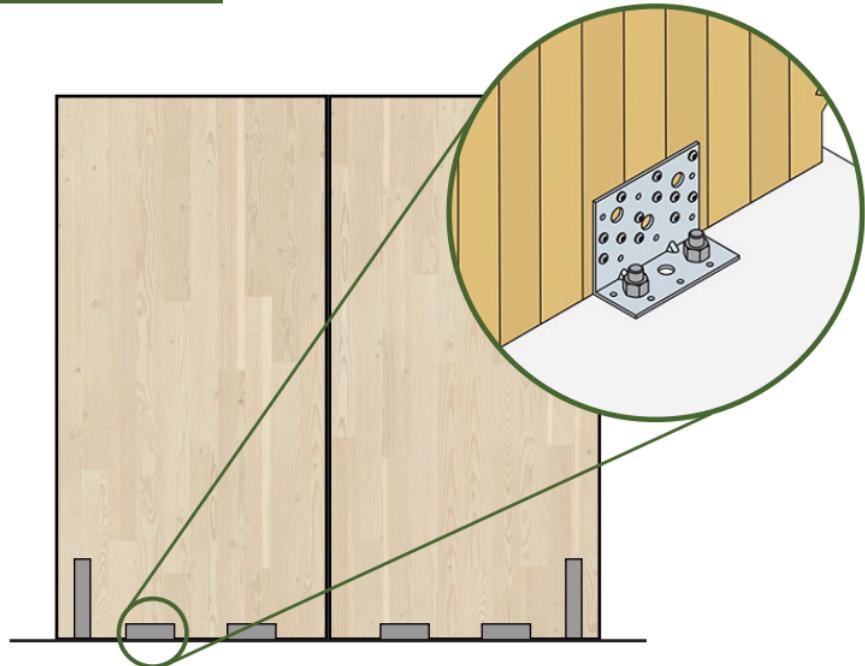
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## Angle brackets



# CLT Wall Construction

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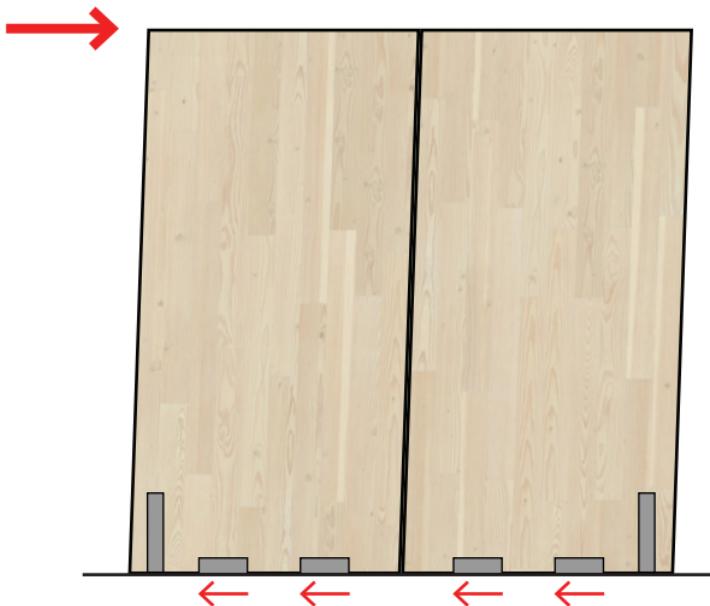
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## Hold downs

- Resist tension from overturning

## Angle brackets

- Resist shear forces at base



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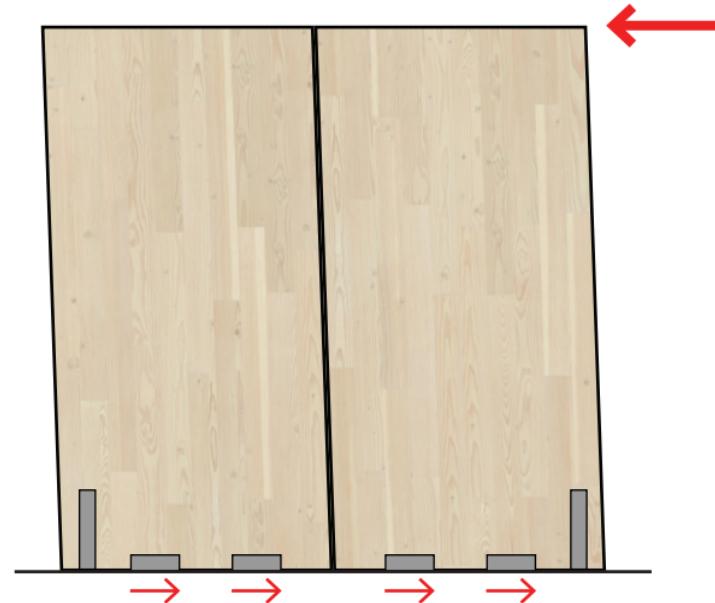
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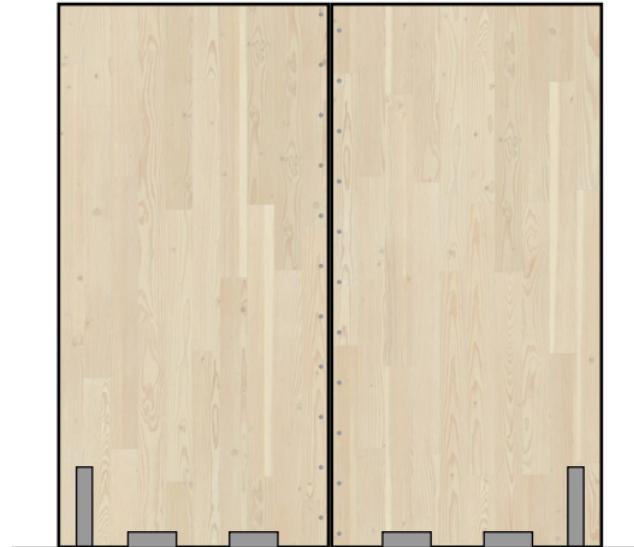
## Hold downs

- Resist tension from overturning

## Angle brackets

- Resist shear forces at base

## Screws in joint



# CLT Wall Construction

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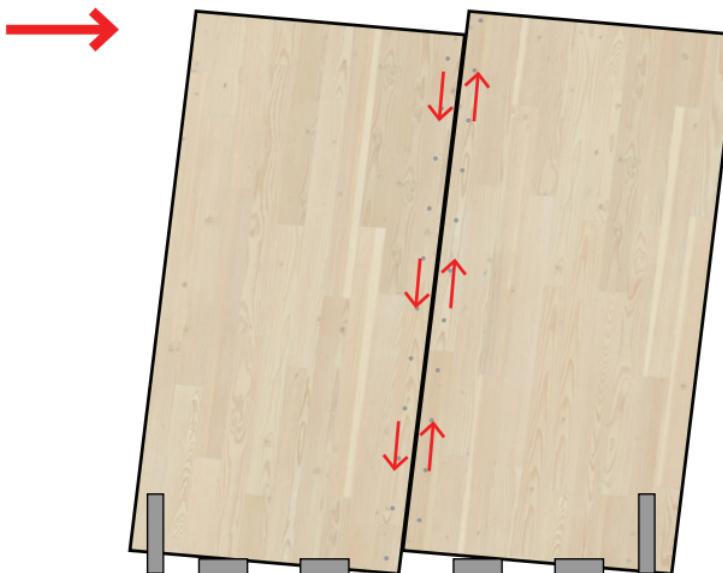
- Resist tension from overturning

## Angle brackets

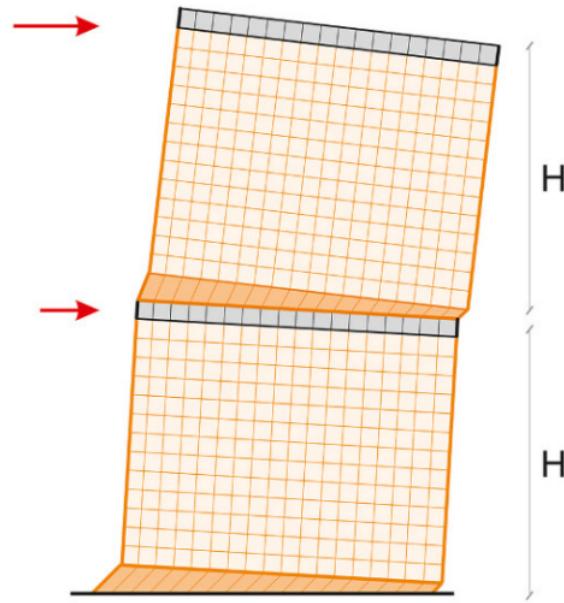
- Resist shear forces at base

## Screws in joint

- Resist sliding forces between panels

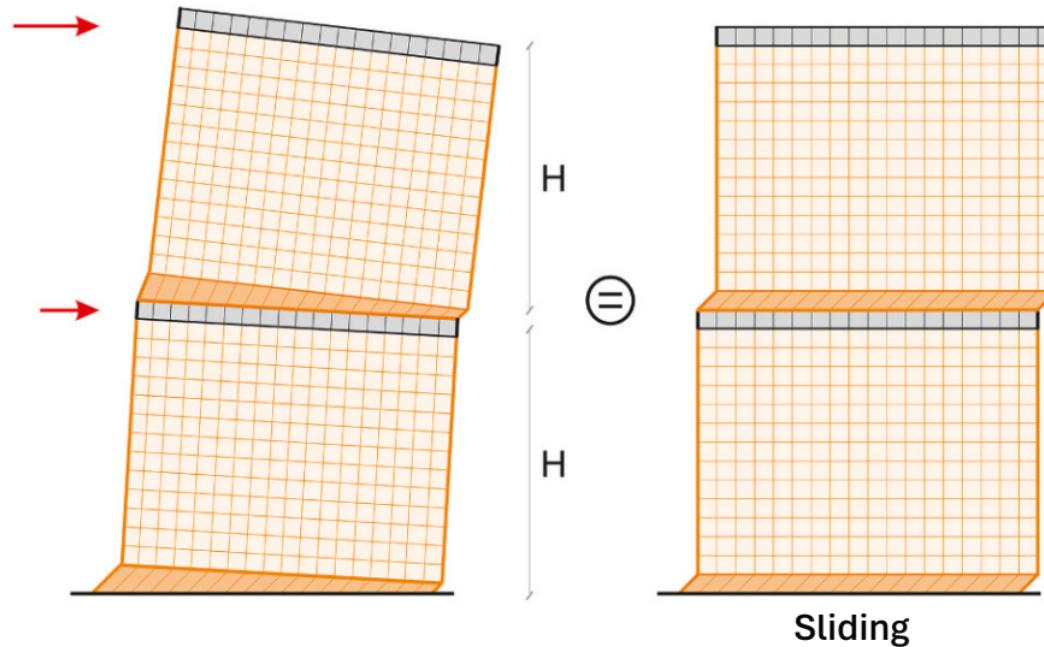


# Deformation Behavior



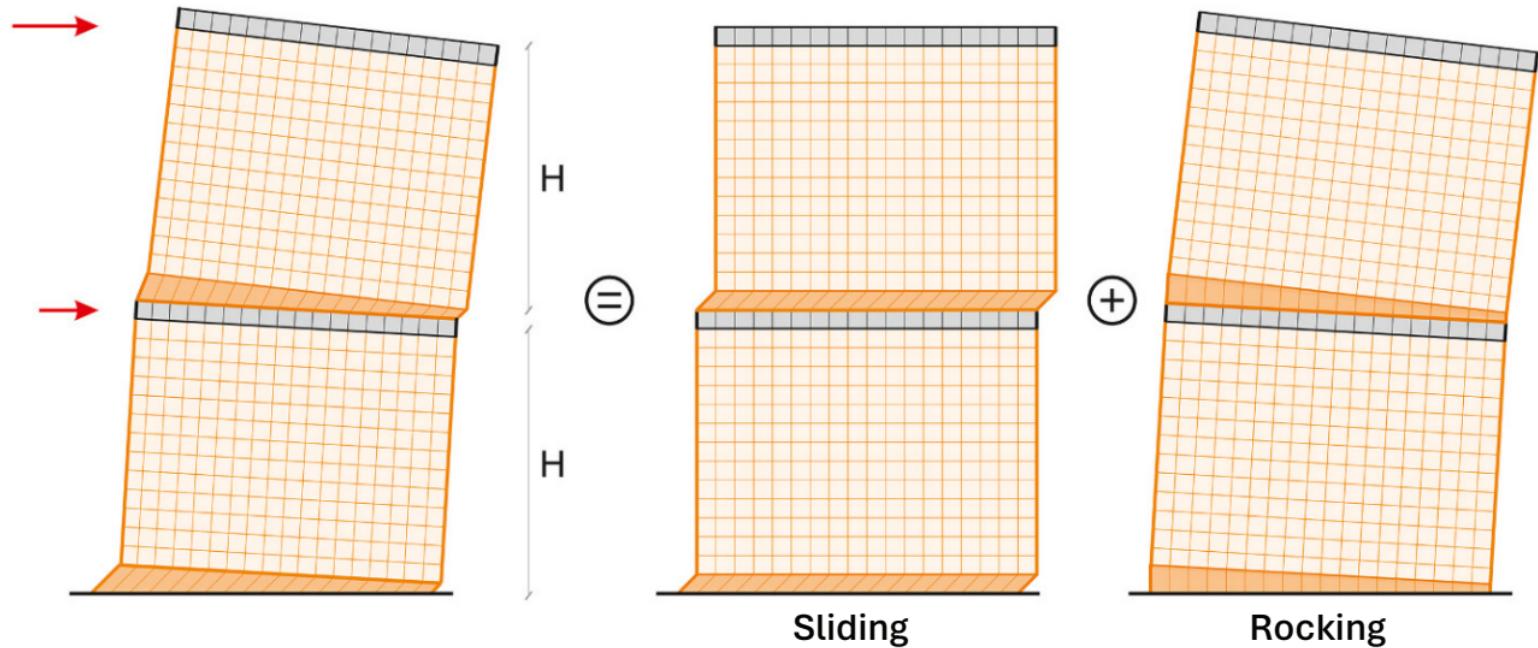
Source: Upgraded Model, Rinaldi et al. 2021

# Deformation Behavior



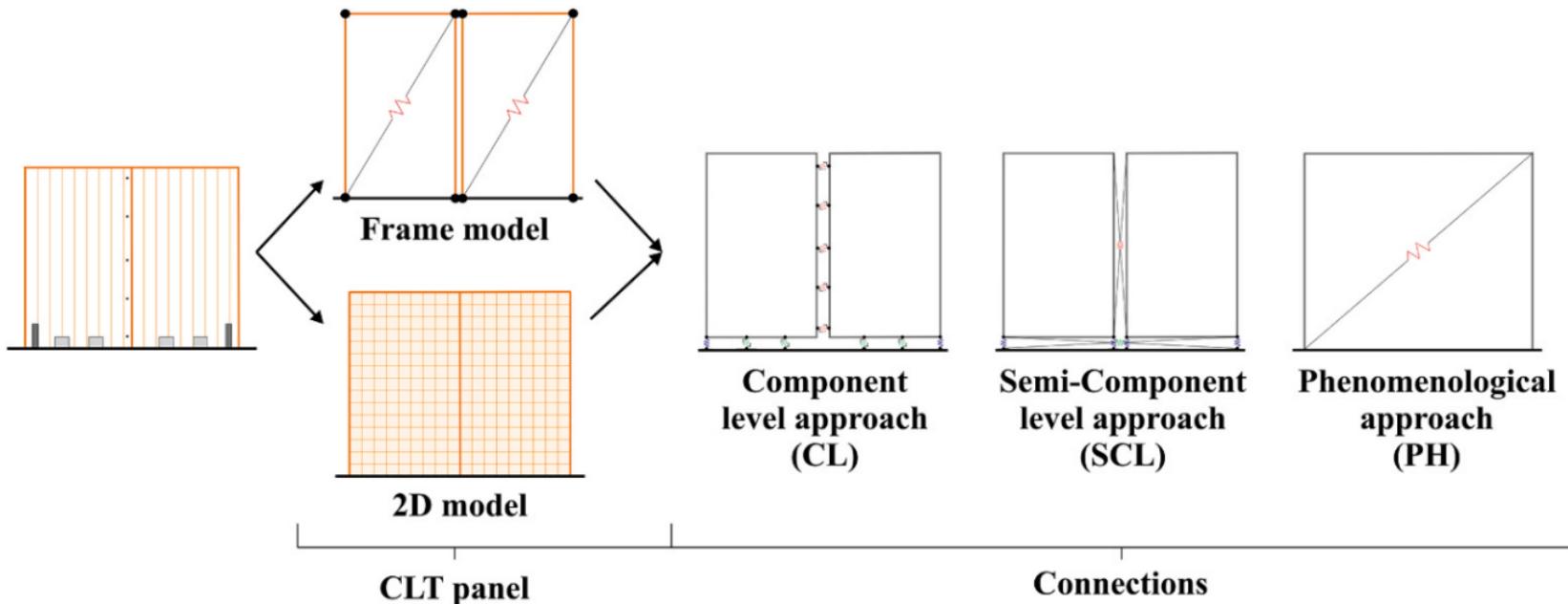
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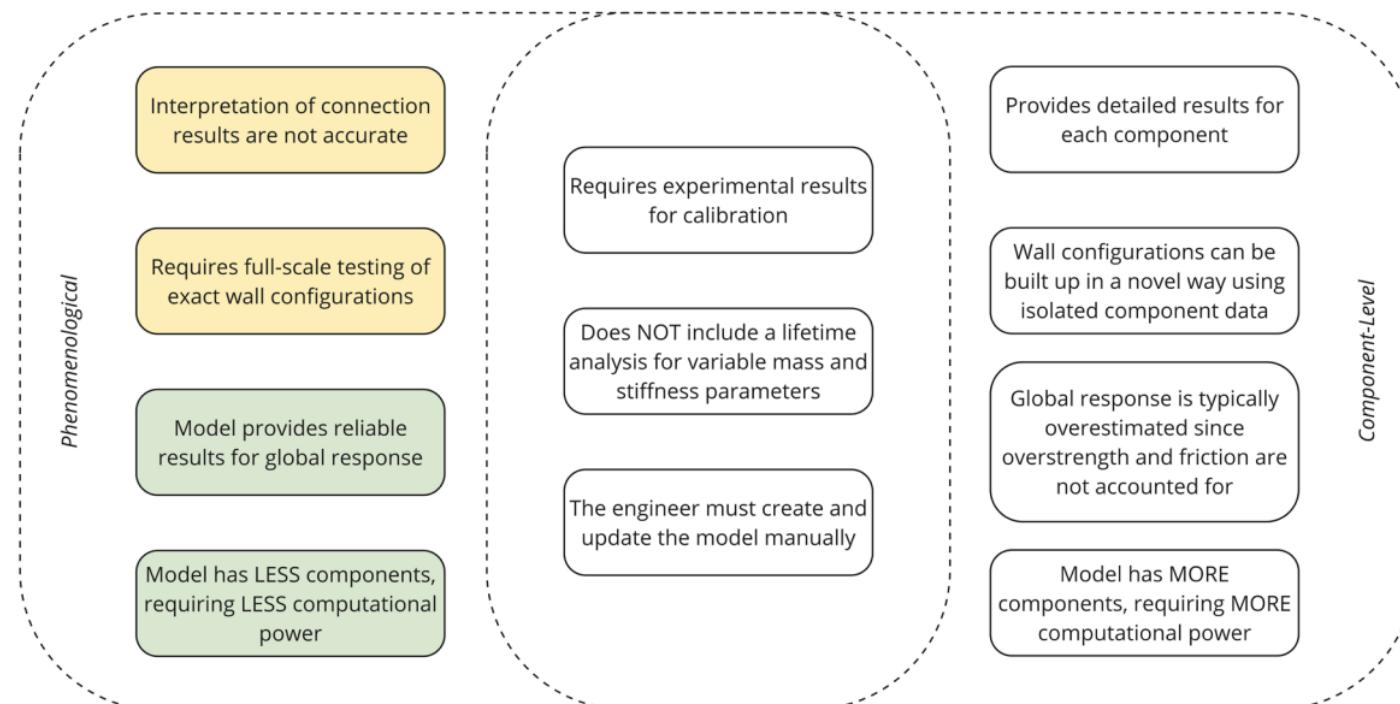
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# Modelling Strategies

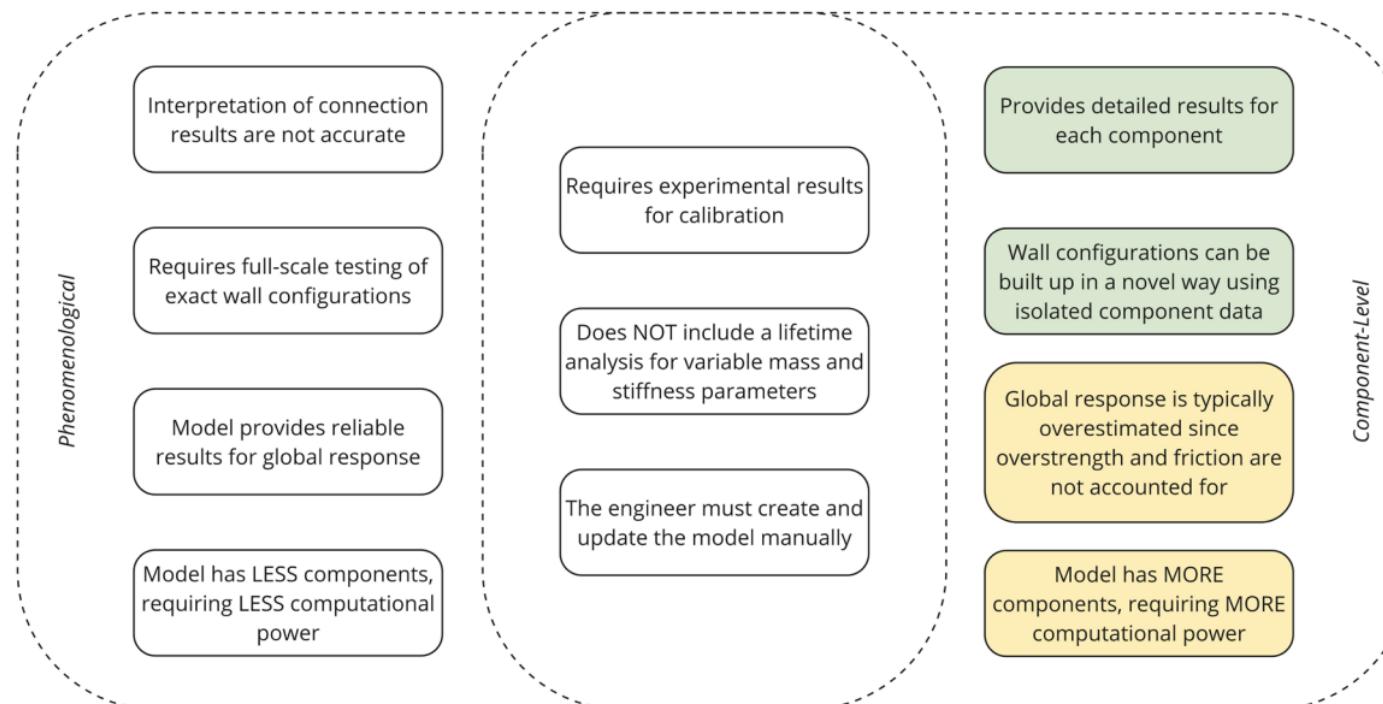


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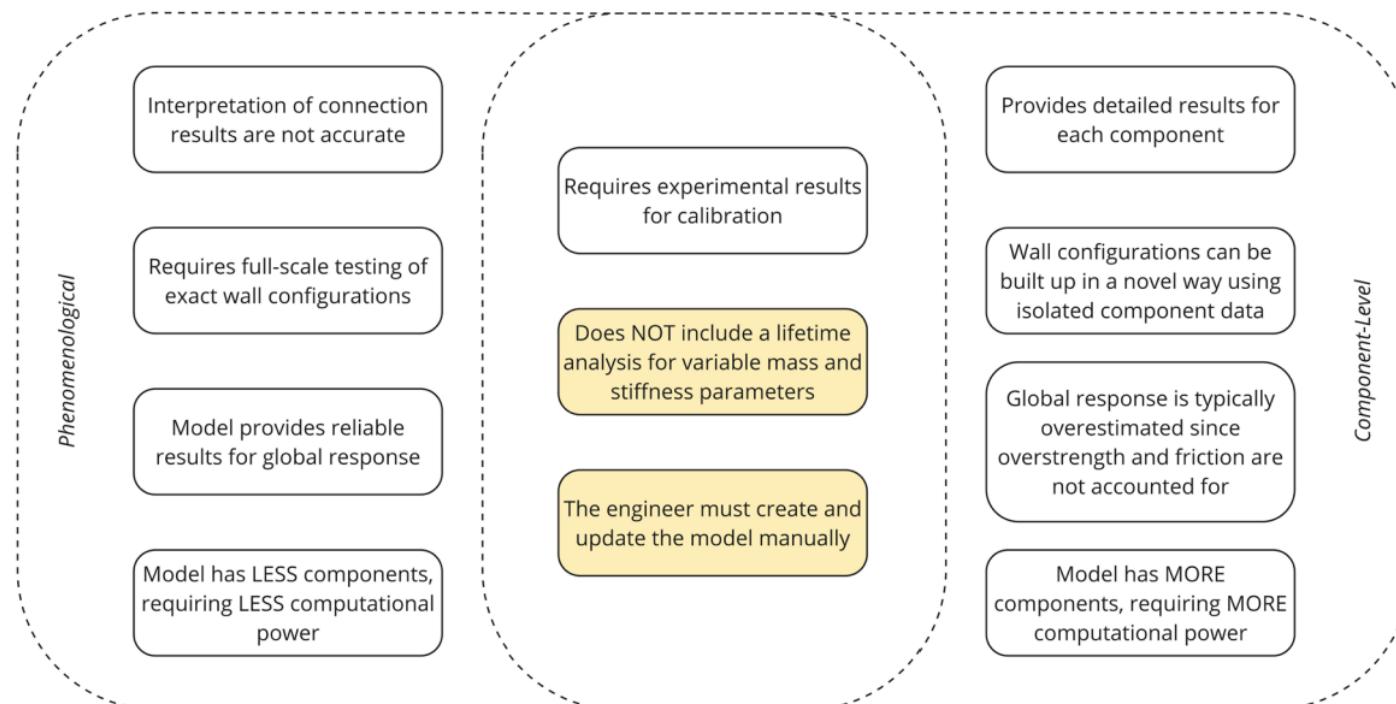
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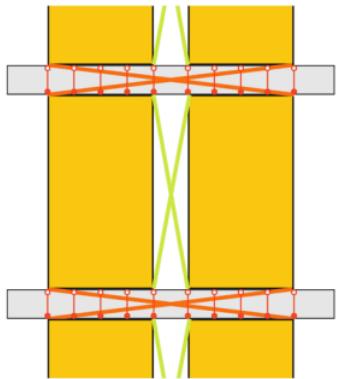
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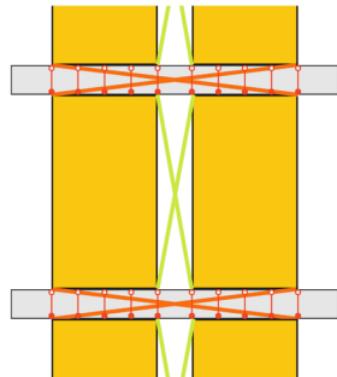
# Modelling Strategies



*Follesa et al. 2013*

*Truss and links for wall-to-floor joints  
Truss for wall-to-wall joints  
Floor stiffness modelled with truss  
Rigid CLT panels  
Includes friction in angle brackets  
Neglects rocking*

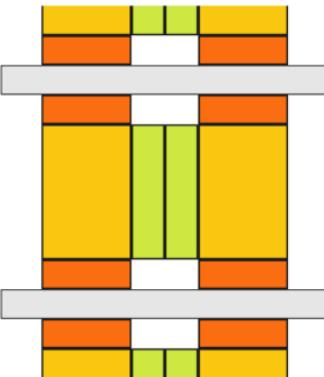
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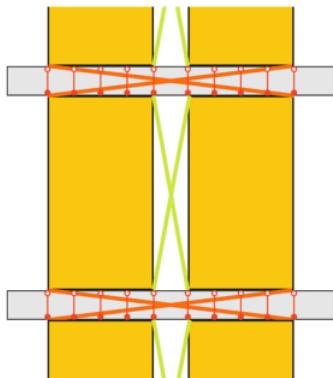
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*Christovasilis et al. 2020*



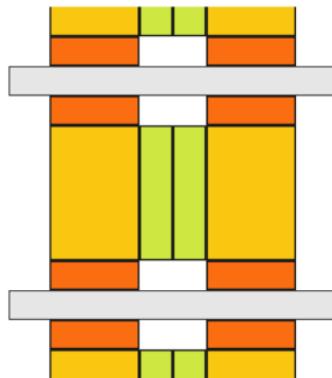
*Horizontal strips for wall-floor joints*  
*Vertical strips for wall-wall joints*  
*Floor stiffness modelled as 2D strip*  
*Rigid CLT panels*  
*Neglects friction*  
*Neglects rocking*

# Modelling Strategies



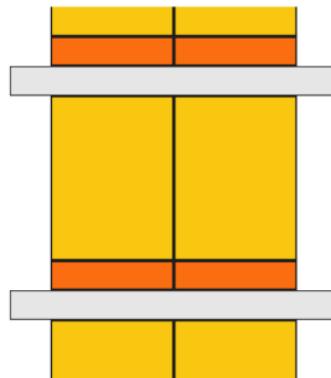
*Follesa et al. 2013*

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*Christovasilis et al. 2020*

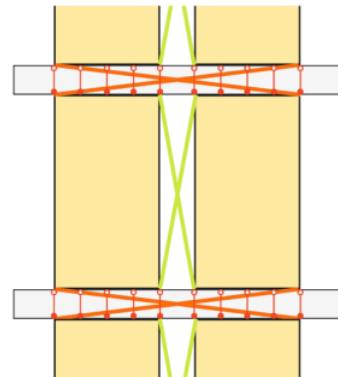
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- Neglects rocking



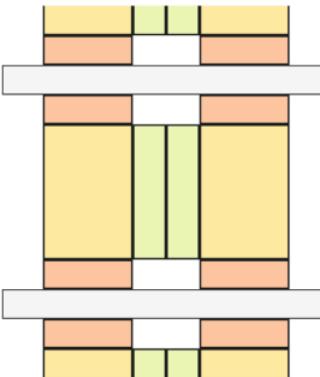
*Rinaldi et al. 2021*

- Horizontal strips for wall-floor joints
- Wall-wall joints in equivalent stiffness
- Floor stiffness modelled as 2D strip
- Rigid CLT panels
- Neglects friction
- Includes rocking in equivalent stiffness

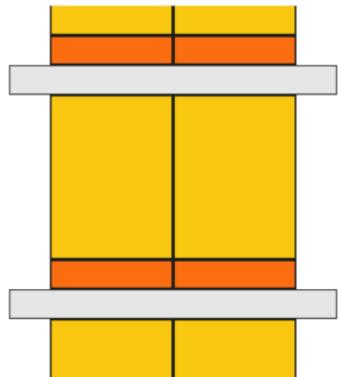
# Modelling Strategies



*Truss and links for wall-to-floor joints  
Truss for wall-to-wall joints  
Floor stiffness modelled with truss  
Rigid CLT panels  
Includes friction in angle brackets  
Neglects rocking*



*Horizontal strips for wall-floor joints  
Vertical strips for wall-wall joints  
Floor stiffness modelled as 2D strip  
Rigid CLT panels  
Neglects friction  
Neglects rocking*



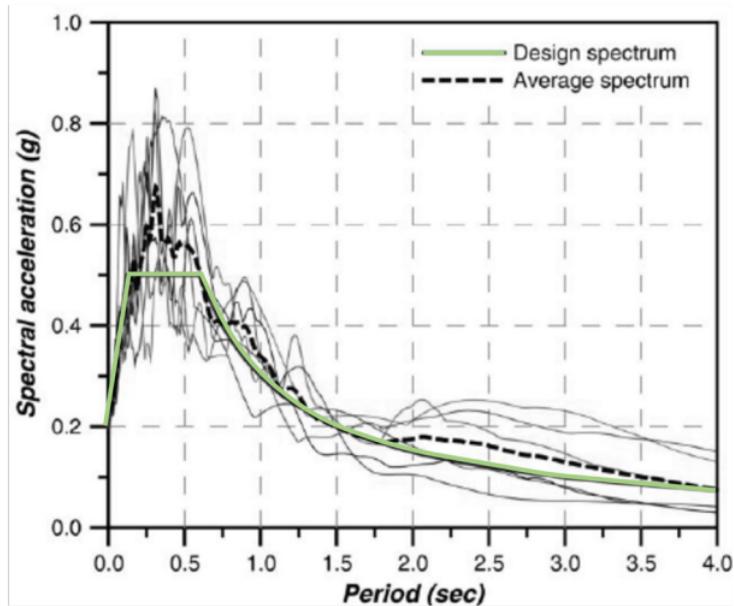
*Horizontal strips for wall-floor joints  
Wall-wall joints in equivalent stiffness  
Floor stiffness modelled as 2D strip  
Rigid CLT panels  
Neglects friction  
Includes rocking in equivalent stiffness*

# Full Structure Analysis

*What is required for full structure analysis?*

## Seismic Parameters

- Spectral acceleration

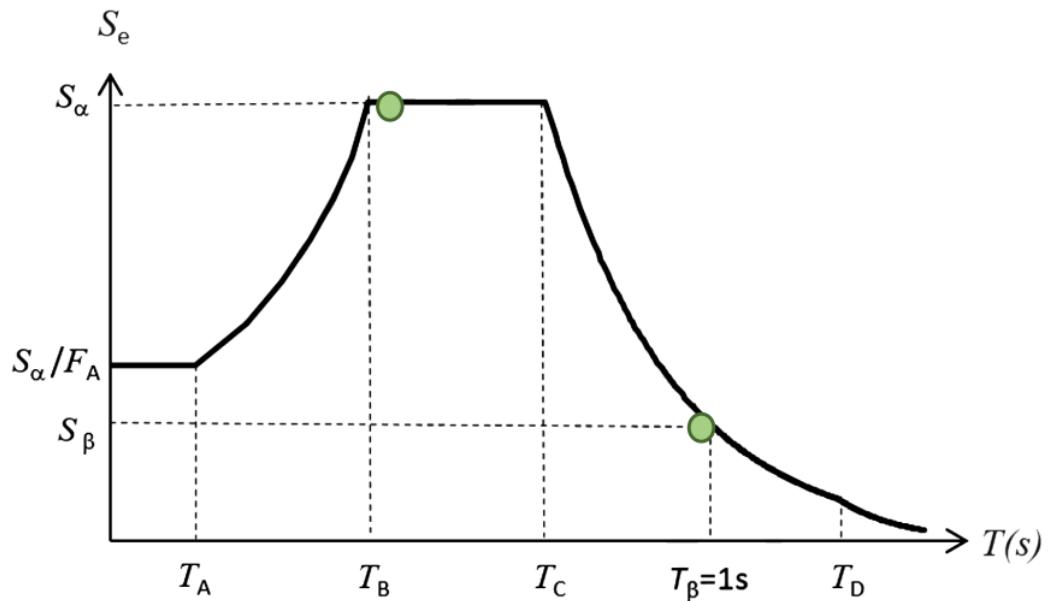


# Full Structure Analysis

*What is required for full structure analysis?*

## Seismic Parameters

- Spectral acceleration



# Full Structure Analysis

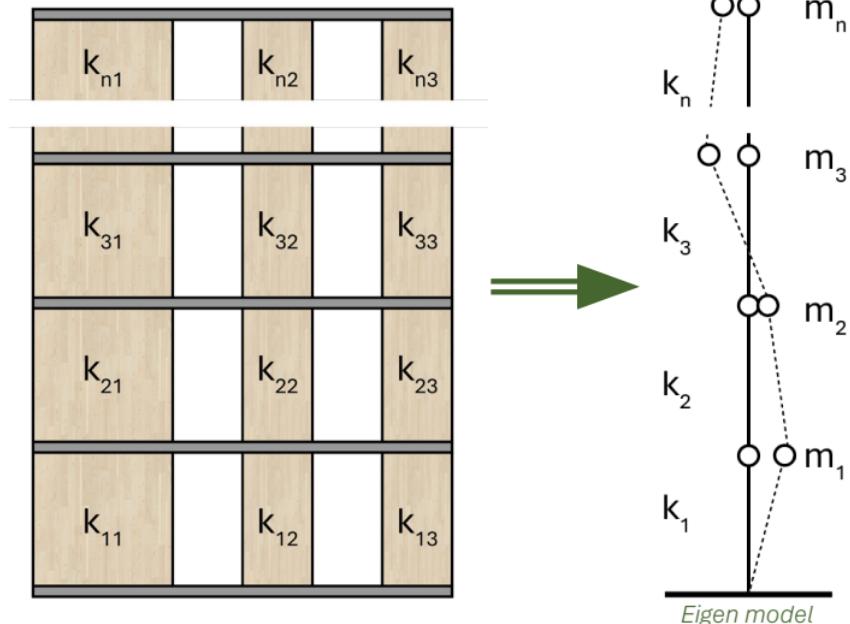
*What is required for full structure analysis?*

## Seismic Parameters

- Spectral acceleration

## Structural Parameters

- Mass and stiffness distribution



# Full Structure Analysis

What is required for full structure analysis?

## Seismic Parameters

- Spectral acceleration

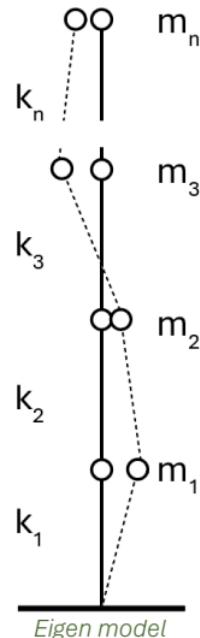
## Structural Parameters

- Mass and stiffness distribution
- Building period

$$\omega = \sqrt{\frac{m}{k}} = \text{eigen vector}$$

$$T = 2\pi\omega$$

*Relationship b/t eigen analysis and building period*



# Full Structure Analysis

What is required for full structure analysis?

## Seismic Parameters

- Spectral acceleration

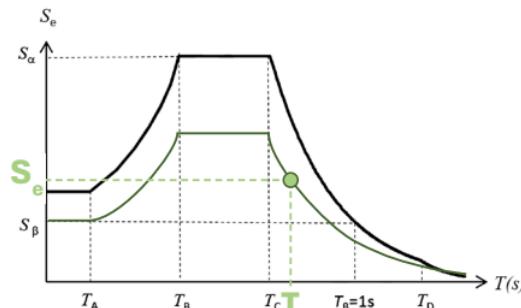
## Structural Parameters

- Mass and stiffness distribution
- Building period
- Seismic coefficient

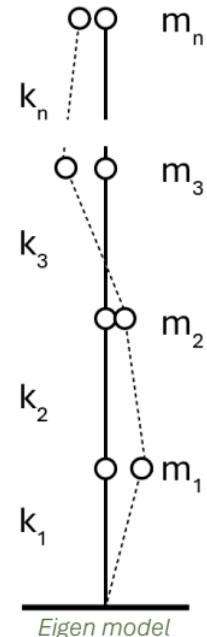
$$\omega = \sqrt{\frac{m}{k}} = \text{eigen vector}$$

$$T = 2\pi\omega$$

Relationship b/t eigen analysis and building period



Determining the seismic coefficient from the design spectra and the building period



# Full Structure Analysis

*What is required for full structure analysis?*

## Seismic Parameters

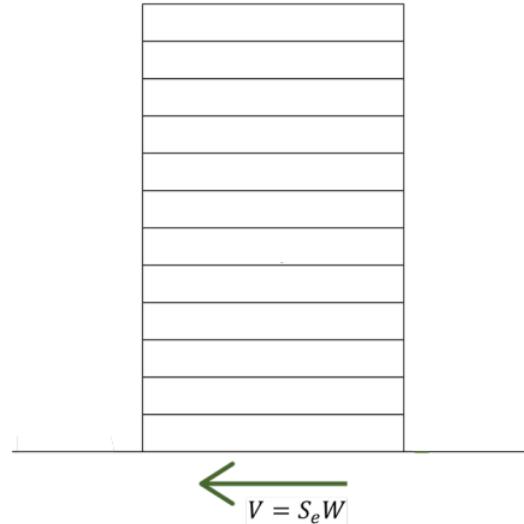
- Spectral acceleration

## Structural Parameters

- Mass and stiffness distribution
- Building period
- Seismic coefficient

## Force Distribution

- Base shear



# Full Structure Analysis

*What is required for full structure analysis?*

## Seismic Parameters

- Spectral acceleration

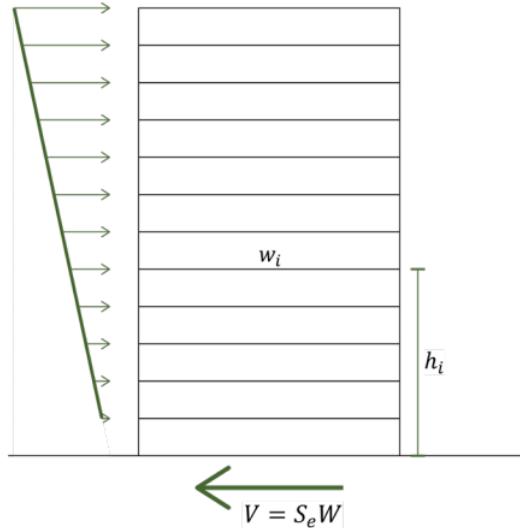
## Structural Parameters

- Mass and stiffness distribution
- Building period
- Seismic coefficient

## Force Distribution

- Base shear
- Distribute to floors based on story height and weight

$$F_x = \frac{w_x h_x}{\sum w_i h_i} V$$



# Full Structure Analysis

*What is required for full structure analysis?*

## Seismic Parameters

- Spectral acceleration

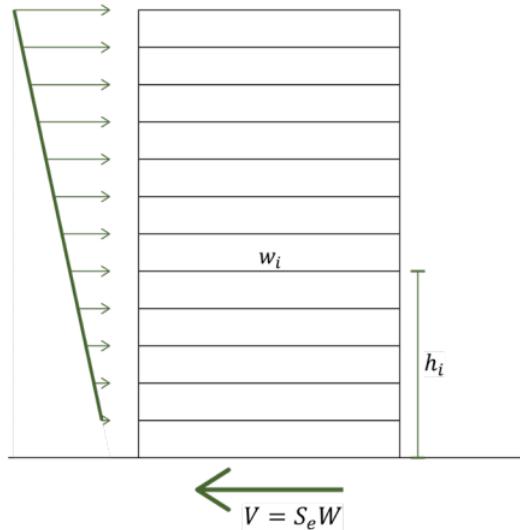
## Structural Parameters

- Mass and stiffness distribution
- Building period
- Seismic coefficient

## Force Distribution

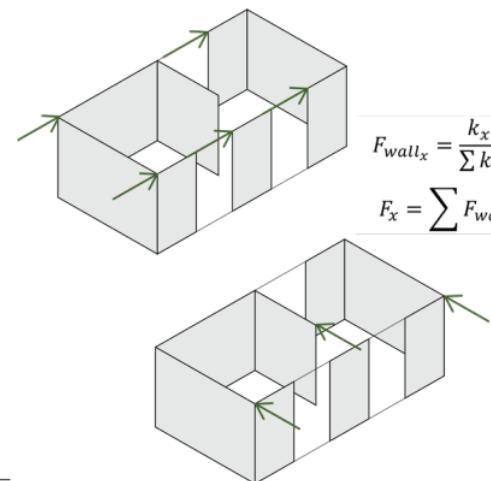
- Base shear
- Distribute to floors based on story height and weight
- Distribute to walls based on effective stiffness

$$F_x = \frac{w_x h_x}{\sum w_i h_i} V$$



$$F_{wall_x} = \frac{k_x}{\sum k_i} F_x$$

$$F_x = \sum F_{wall_i}$$



# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

Previous Seismic Event



# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

- Stiffness reduction from:



Seven story test structure from the SOFIE project

Source: SOFIE project, Ceccotti et al. 2013

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures



Source: SOFIE project, Ceccotti et al. 2013

Seven story test structure from the SOFIE project



*Nail failures at hold-downs*

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals



Seven story test structure from the SOFIE project

Source: SOFIE project, Ceccotti et al. 2013



Nail failures at hold-downs



Nail withdrawal at angle brackets

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing



Seven story test structure from the SOFIE project

Source: SOFIE project, Ceccotti et al. 2013



Nail failures at hold-downs



Nail withdrawal at angle brackets



Wood crushing under hold-downs

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing

## Future Renovation



# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing



## Future Renovation

- Mass distribution

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing



## Future Renovation

- Mass distribution



# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

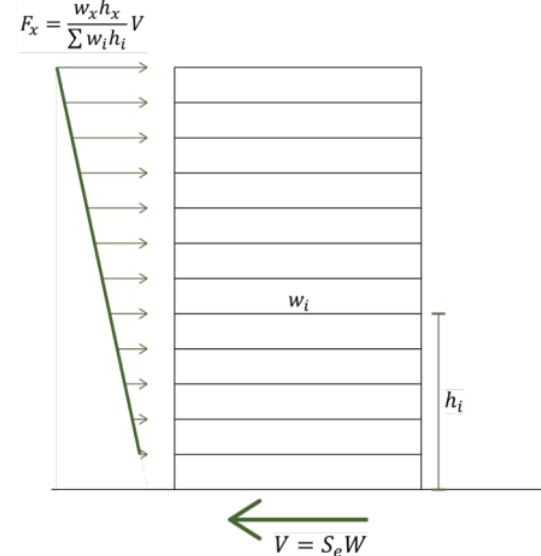
## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing



## Future Renovation

- Mass distribution



*Change in mass and forces*

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

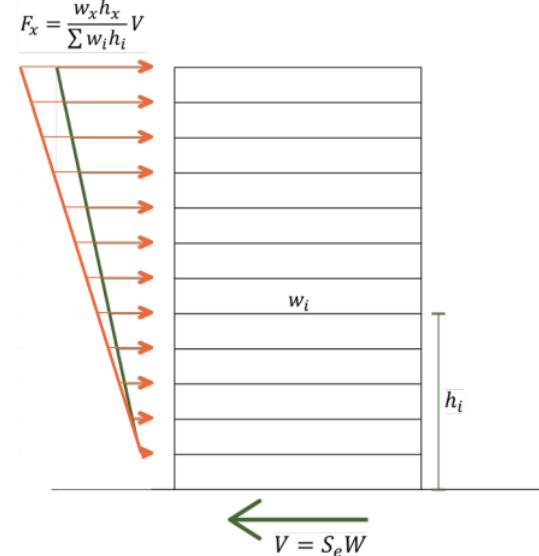
## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing



## Future Renovation

- Mass distribution



Change in mass and forces

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing



*Replace old connections*

## Future Renovation

- Mass distribution
- Stiffness change

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

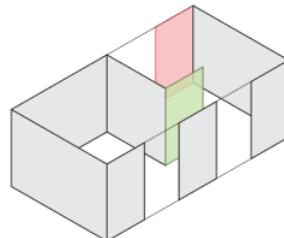
- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing



*Replace old connections*

## Future Renovation

- Mass distribution
- Stiffness change



*Add, remove, or replace walls*

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

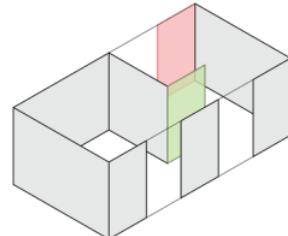
- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing



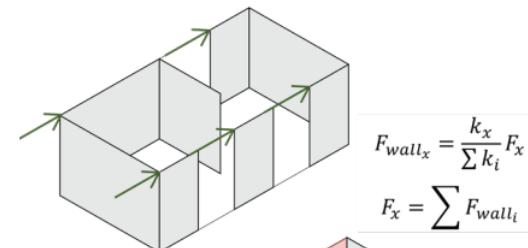
*Replace old connections*

## Future Renovation

- Mass distribution
- Stiffness change



*Add, remove, or replace walls*



*Change in stiffness distribution*

$$F_{wall_x} = \frac{k_x}{\sum k_i} F_x$$

$$F_x = \sum F_{wall_i}$$

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

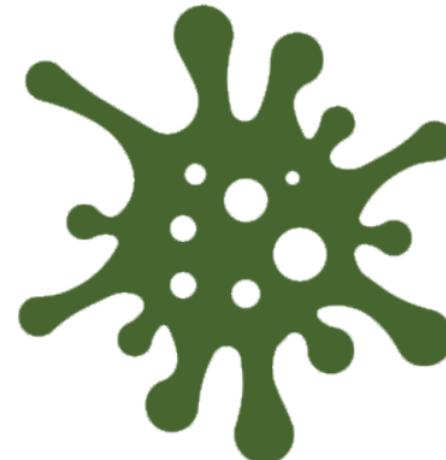
## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing

## Future Renovation

- Mass distribution
- Stiffness change

## Biological Degradation



# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing



*Deterioration causes loss in mass and stiffness*

## Future Renovation

- Mass distribution
- Stiffness change

## Biological Degradation

- Stiffness and mass loss in CLT

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing



*Deterioration causes loss in mass and stiffness*

## Future Renovation

- Mass distribution
- Stiffness change

## Biological Degradation

- Stiffness and mass loss in CLT



*Related to moisture exposure and time*

# Lifetime Analysis

*How can these events or time variables impact seismic performance?*

## Previous Seismic Event

- Stiffness reduction from:
  - Fastener failures
  - Fastener withdrawals
  - Wood crushing



Deterioration causes loss in mass and stiffness

## Future Renovation

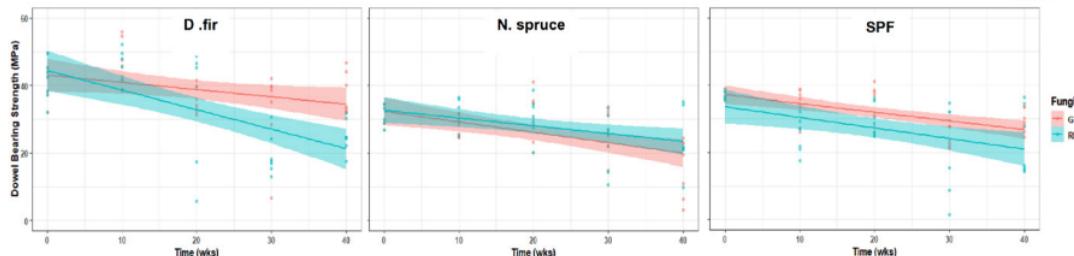
- Mass distribution
- Stiffness change

## Biological Degradation

- Stiffness and mass loss in CLT
- Embedment strength of fasteners in connections



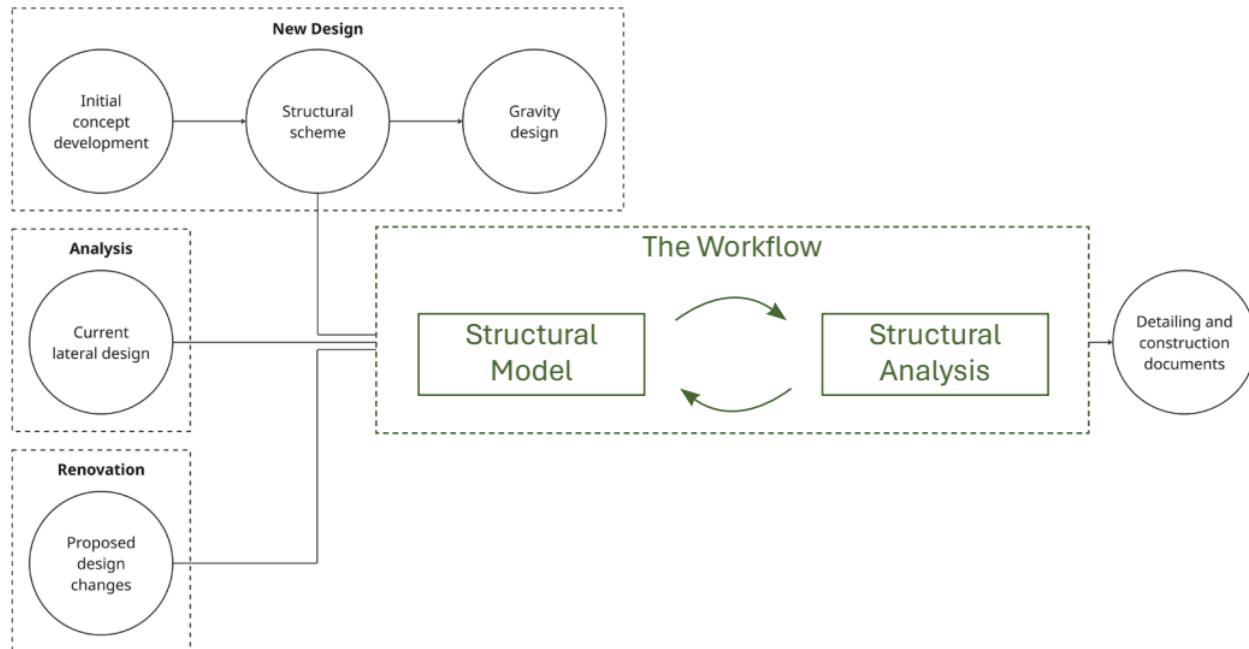
Related to moisture exposure and time



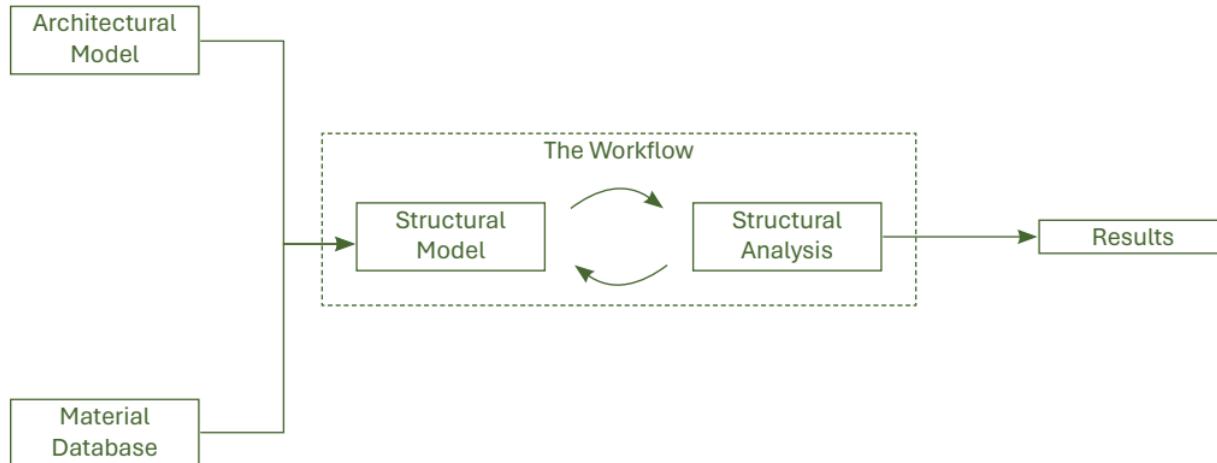
Linear regression for dowel bearing strength over time of fungal exposure for three species of wood: Douglas Fir, Norway Spruce, and Spruce Pine Fir (Source: Udele et al. 2024)

# The Workflow

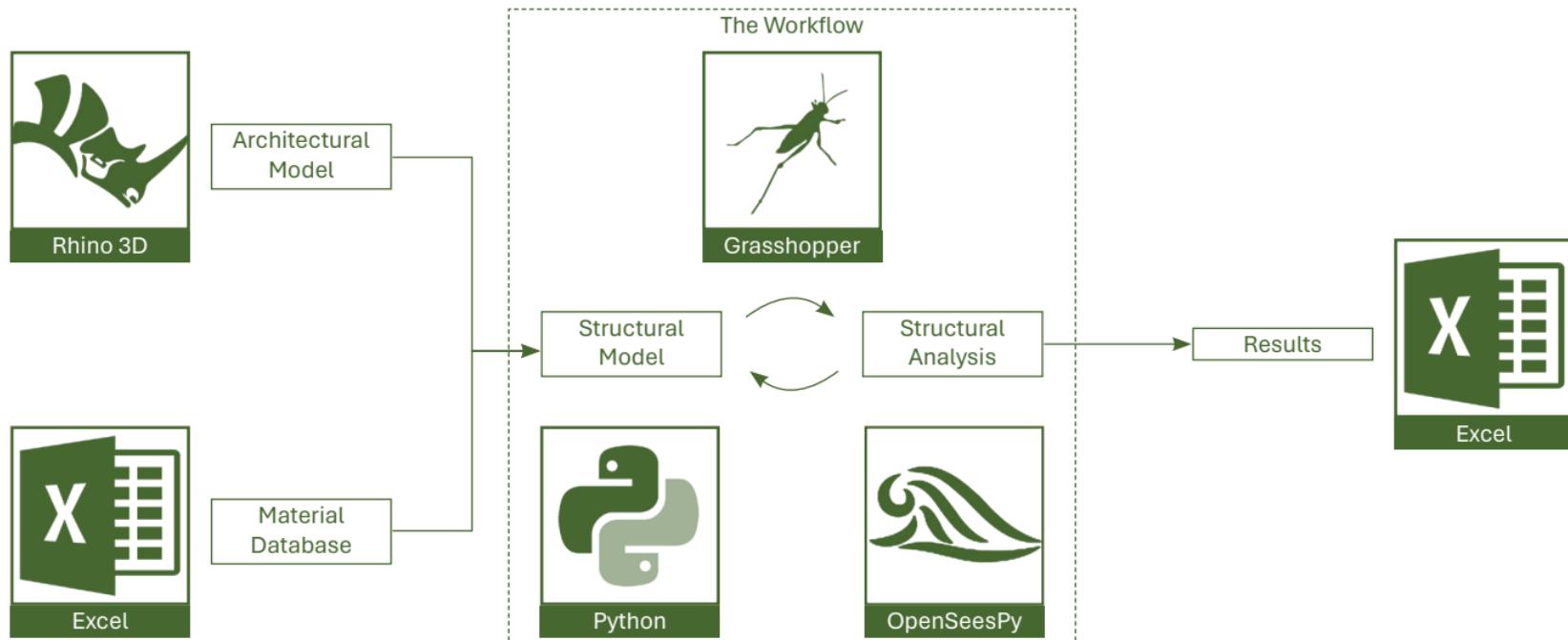
# When to use the tool?



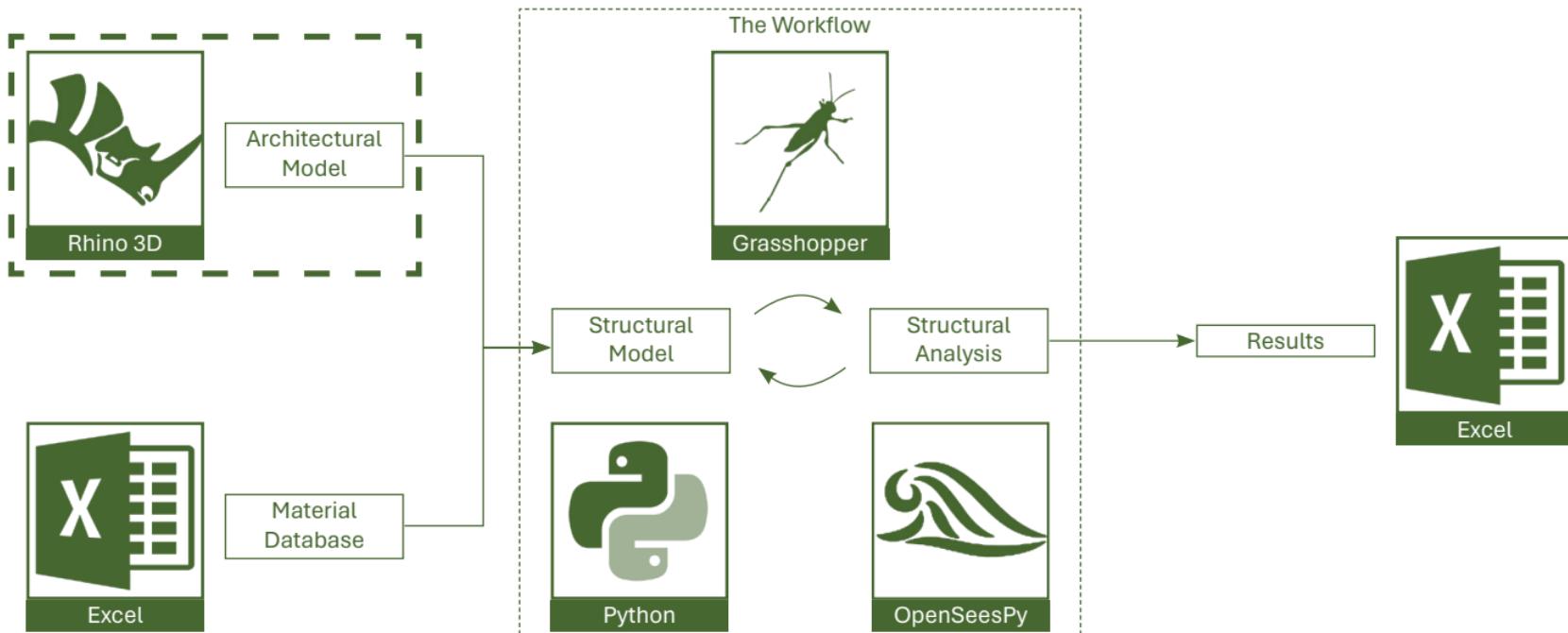
# The Workflow



# Programs and Workflow



# Programs and Workflow

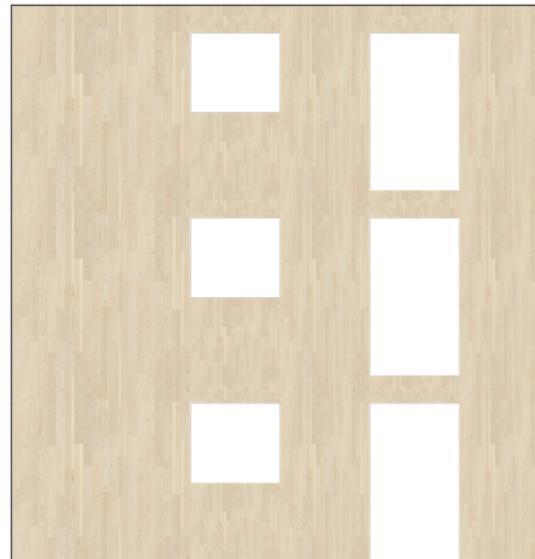




# Model Generation

*Create Geometry*

Wall Geometry



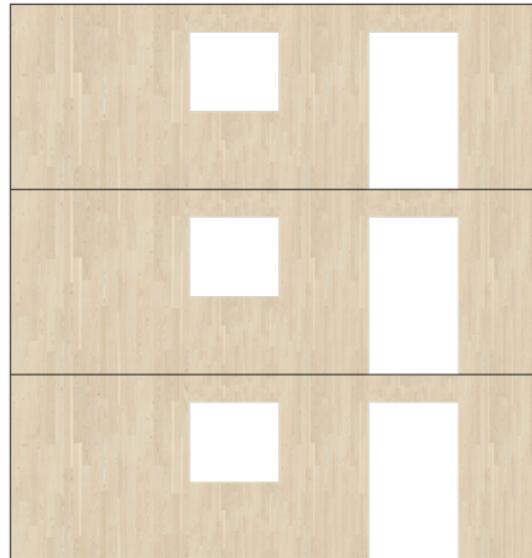


# Model Generation

## *Create Geometry*

### Wall Geometry

- Split at each floor



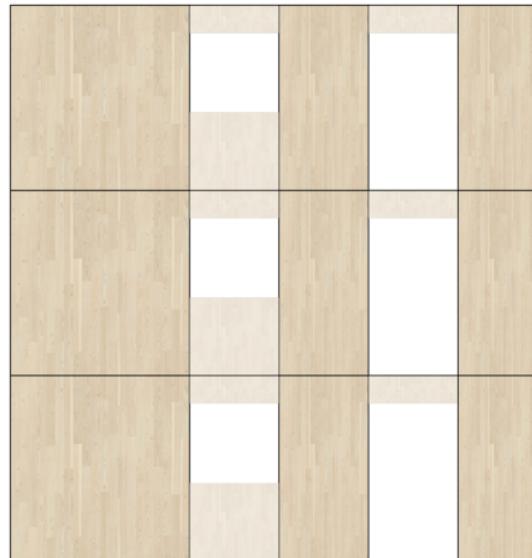
# Model Generation



## *Create Geometry*

### Wall Geometry

- Split at each floor
- Ignore walls with openings





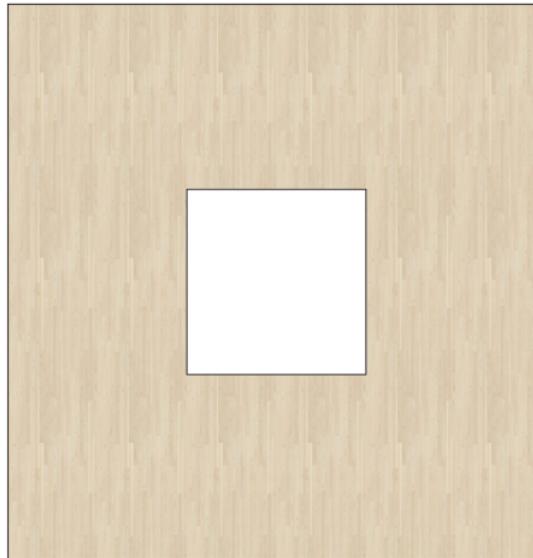
# Model Generation

## *Create Geometry*

### Wall Geometry

- Split at each floor
- Ignore walls with openings

### Floor Geometry





# Model Generation

## *Create Geometry*

### **Wall Geometry**

- Split at each floor
- Ignore walls with openings

### **Floor Geometry**

- Ignore shafts and openings





# Model Generation

## *Create Geometry*

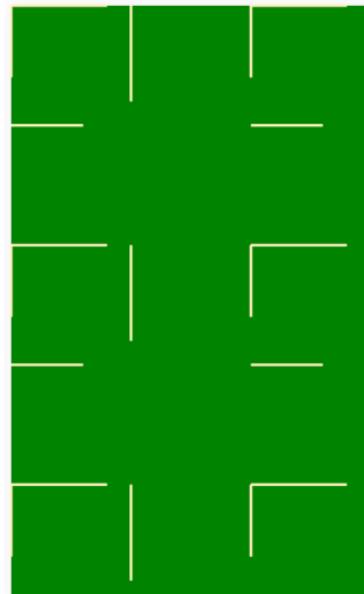
### **Wall Geometry**

- Split at each floor
- Ignore walls with openings

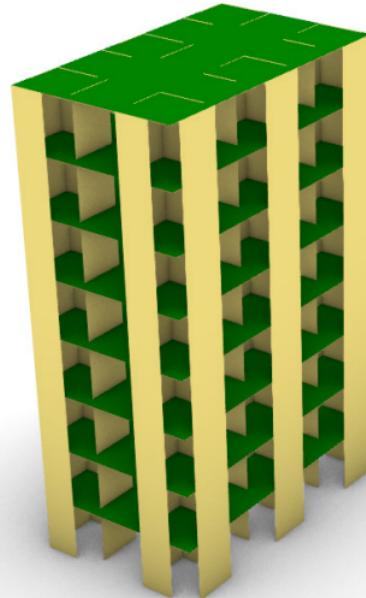
### **Floor Geometry**

- Ignore shafts and openings

*Modelled with Rectangle 3D*

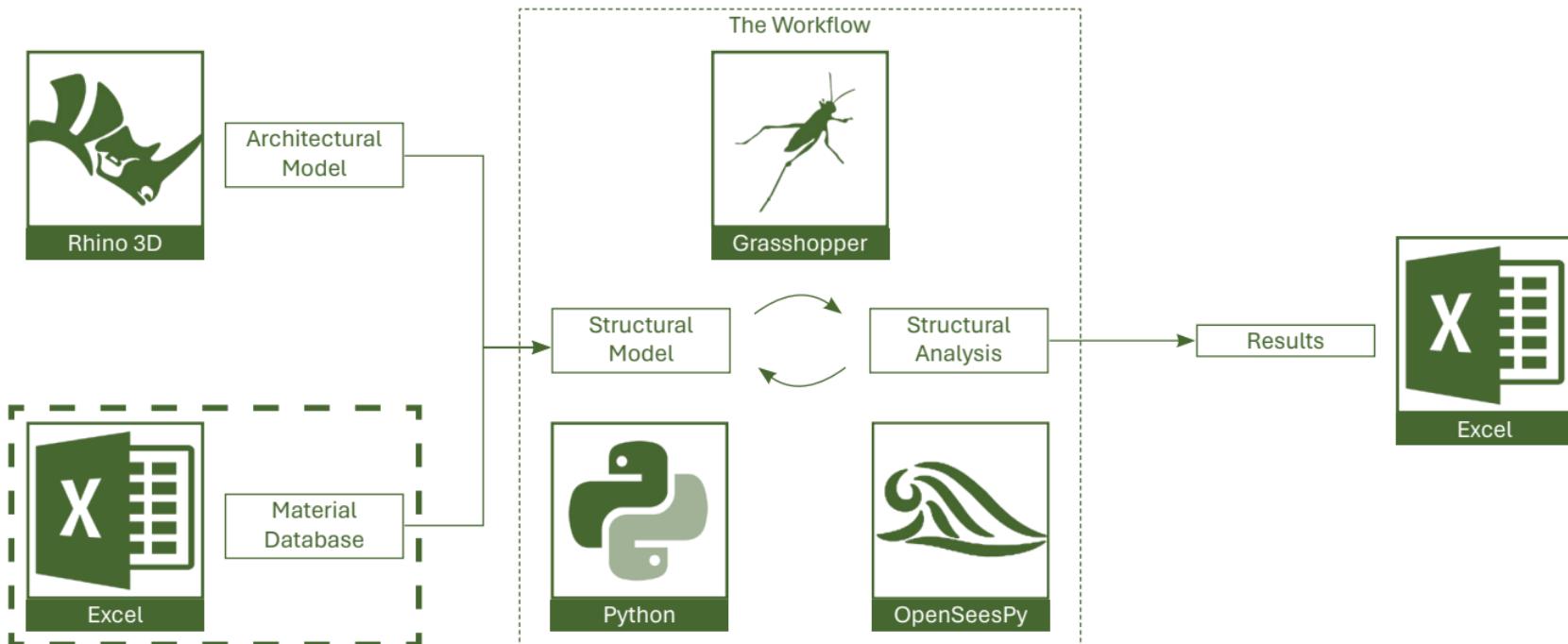


*Plan view of example structure*



*3D model of example structure*

# Programs and Workflow



# Materials Database



*CLT properties*

## Raw Timber Properties

- Species





# Materials Database

## *CLT properties*

### Raw Timber Properties

- Species
- Board thicknesses



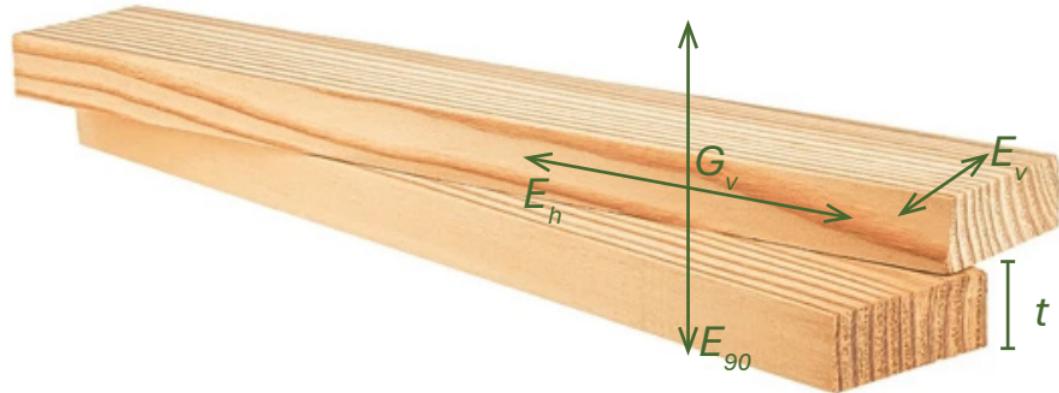


# Materials Database

## *CLT properties*

### Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties





# Materials Database

*CLT properties*

## Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

## Panel Properties



# Materials Database

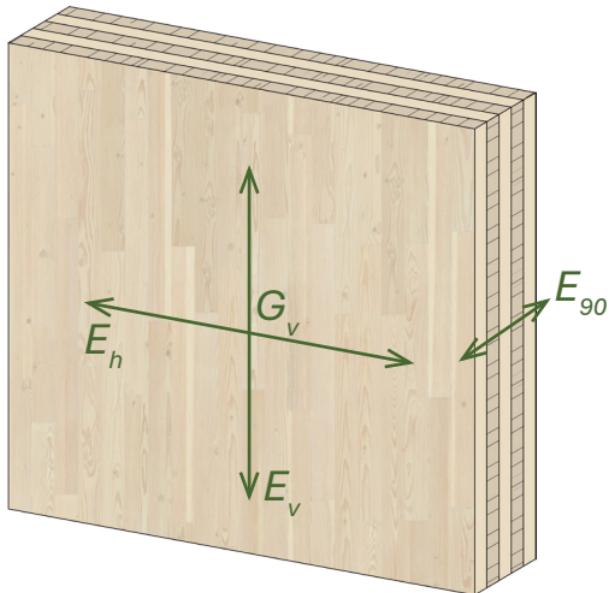
## *CLT properties*

### Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

### Panel Properties

- Elastic and shear modulus



# Materials Database

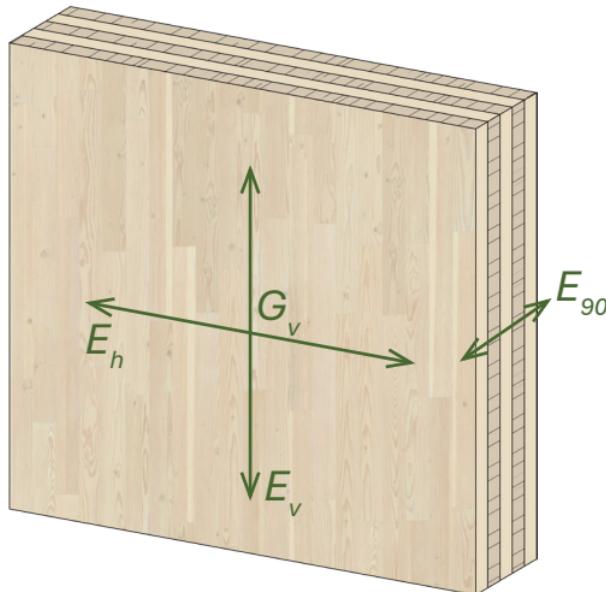
## *CLT properties*

### Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

### Panel Properties

- Elastic and shear modulus
  - Calculated via research



*Elastic Moduli calculated via  
Blass et al. 2004*

*Shear Modulus calculated via  
Bogensperger et al. 2016*



# Materials Database

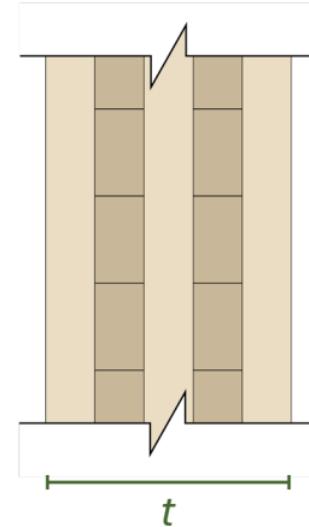
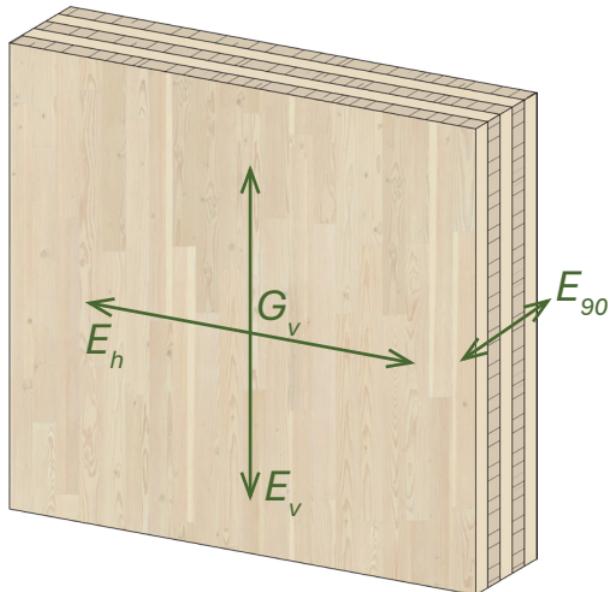
## *CLT properties*

### Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

### Panel Properties

- Elastic and shear modulus
  - Calculated via research
- Thickness of panel





# Materials Database

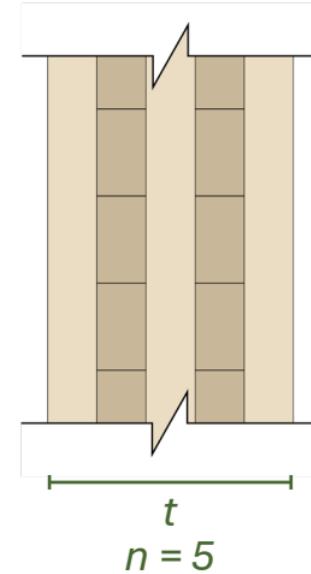
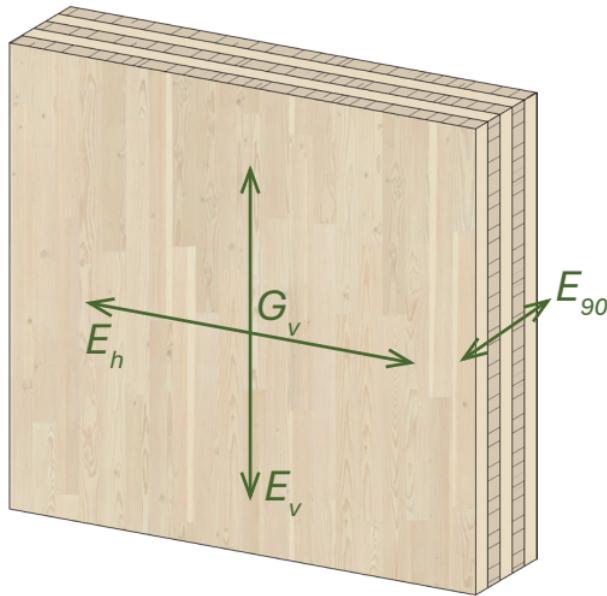
## *CLT properties*

### Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

### Panel Properties

- Elastic and shear modulus
  - Calculated via research
- Thickness of panel
- Number of layers





# Materials Database

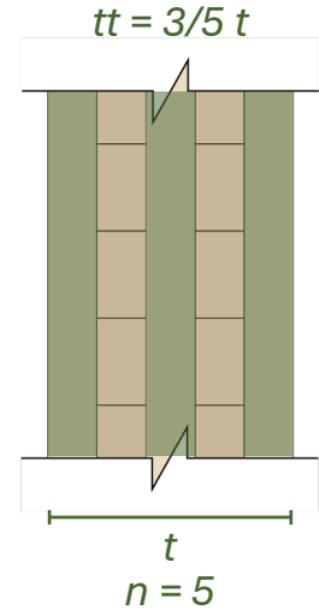
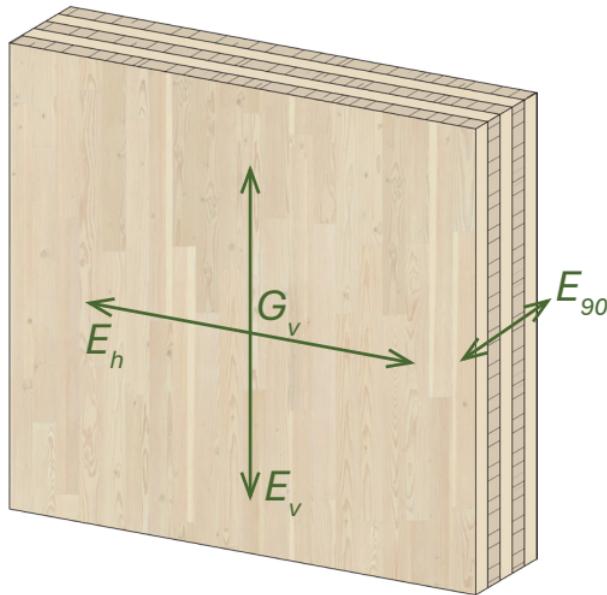
## *CLT properties*

### Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

### Panel Properties

- Elastic and shear modulus
  - Calculated via research
- Thickness of panel
- Number of layers
- Layer thickness ratio
  - Calculated via research



# Materials Database



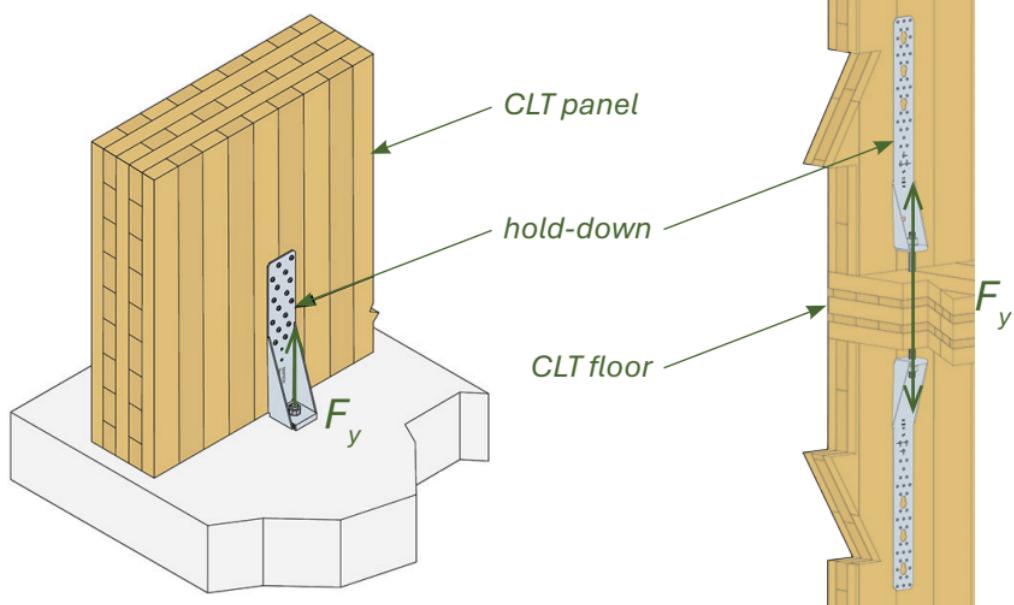
## *Hold Down properties*

### Connection Properties

- Yielding load and deflection
- Stiffness in both directions
- Plate thickness
- Overstrength

### Fastener Properties

- Length
- Diameter
- Amount



Source: Simpson Strong Tie



# Materials Database

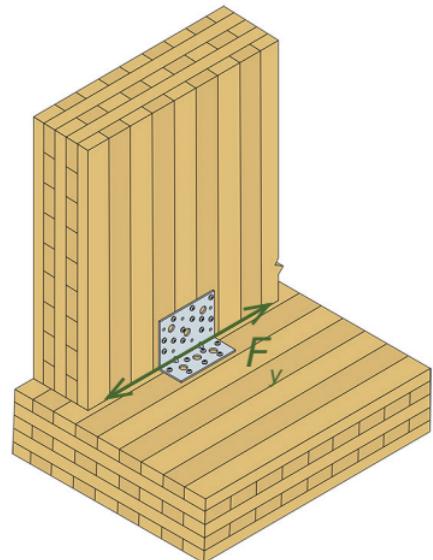
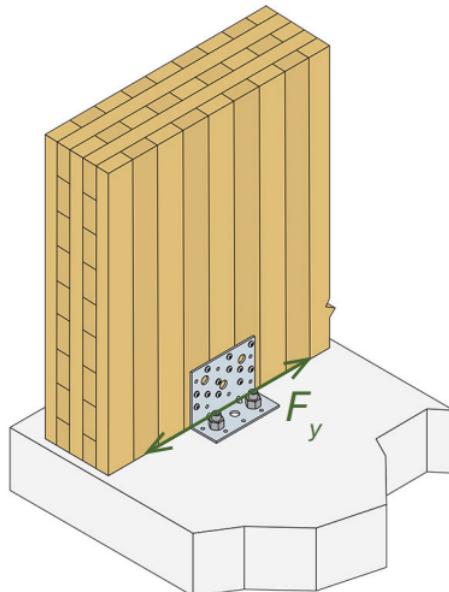
## *Angle Bracket properties*

### Connection Properties

- Yielding load and deflection
- Stiffness in both directions
- Plate thickness
- Overstrength

### Fastener Properties

- Length
- Diameter
- Amount



Source: Simpson Strong Tie

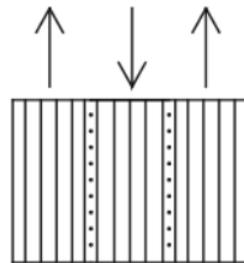


# Materials Database

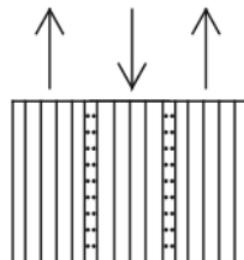
## *Screw Joint properties*

### **Joint Connection Properties**

- Joint type (splice or lap)
- Load direction (vertical in plane)
- Orientation (Parallel)
- Yield strength
- Stiffness



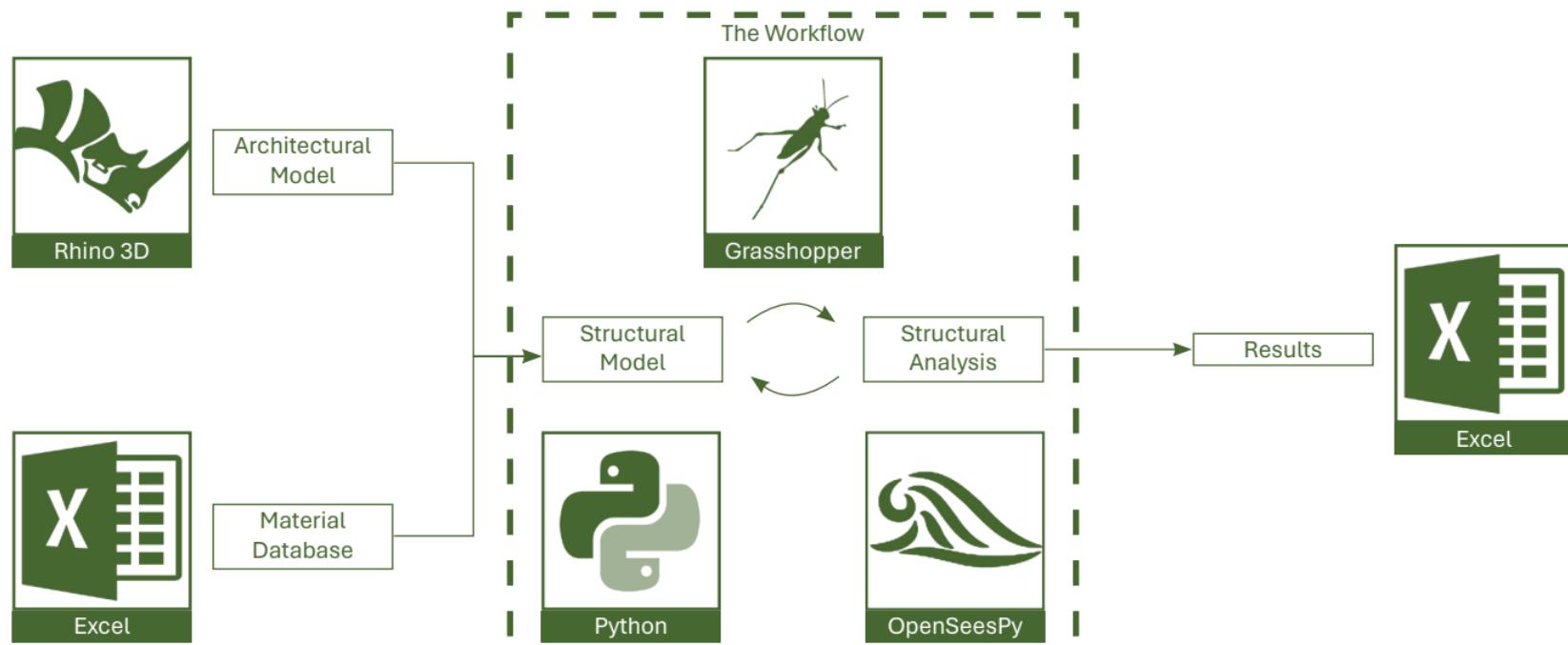
**Lateral parallel  
(lap joint)**



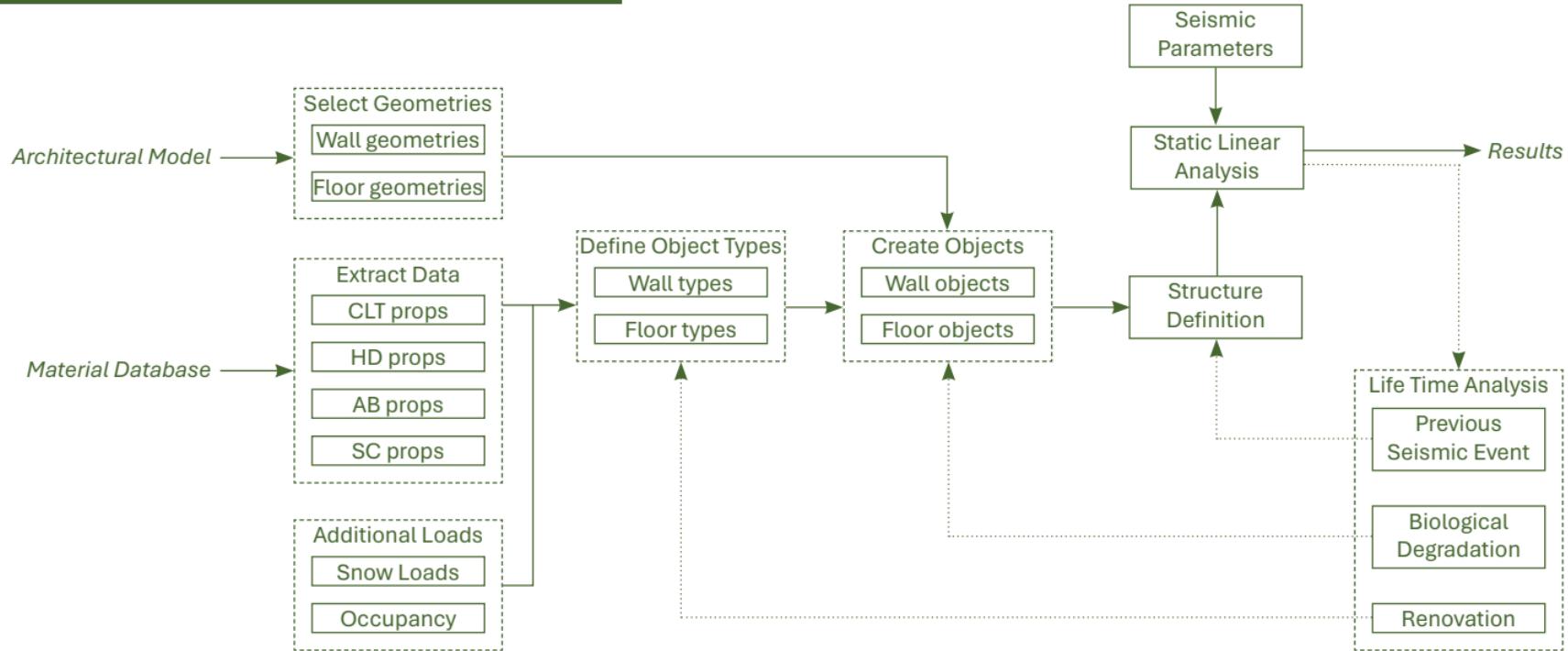
**Lateral parallel  
(spline joint)**

Source: Gavric et al. 2015

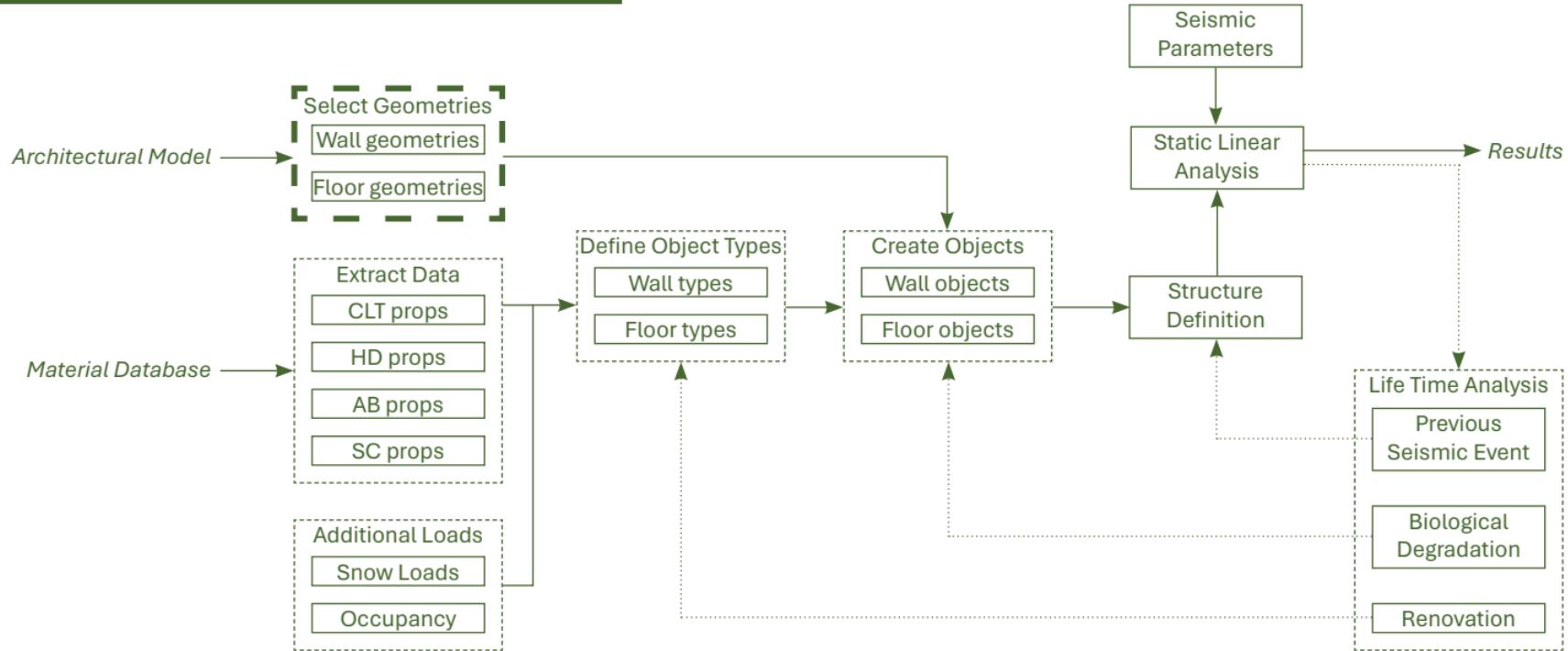
# Programs and Workflow



# The Workflow



# The Workflow



# Selecting Geometries



## Organized by Layers

- Separate wall and floor layers



Walls

Floors

# Selecting Geometries



## Organized by Layers

- Separate wall and floor layers
- Separate by types into sublayers



### Walls

Wall type 1

Wall type 2

Wall type 3

Wall type 4

### Floors

Floor type 1

Floor type 2

# Selecting Geometries



## Organized by Layers

- Separate wall and floor layers
- Separate by types into sublayers

## Bring into Grasshopper



### Walls

Wall type 1

Wall type 2

Wall type 3

Wall type 4

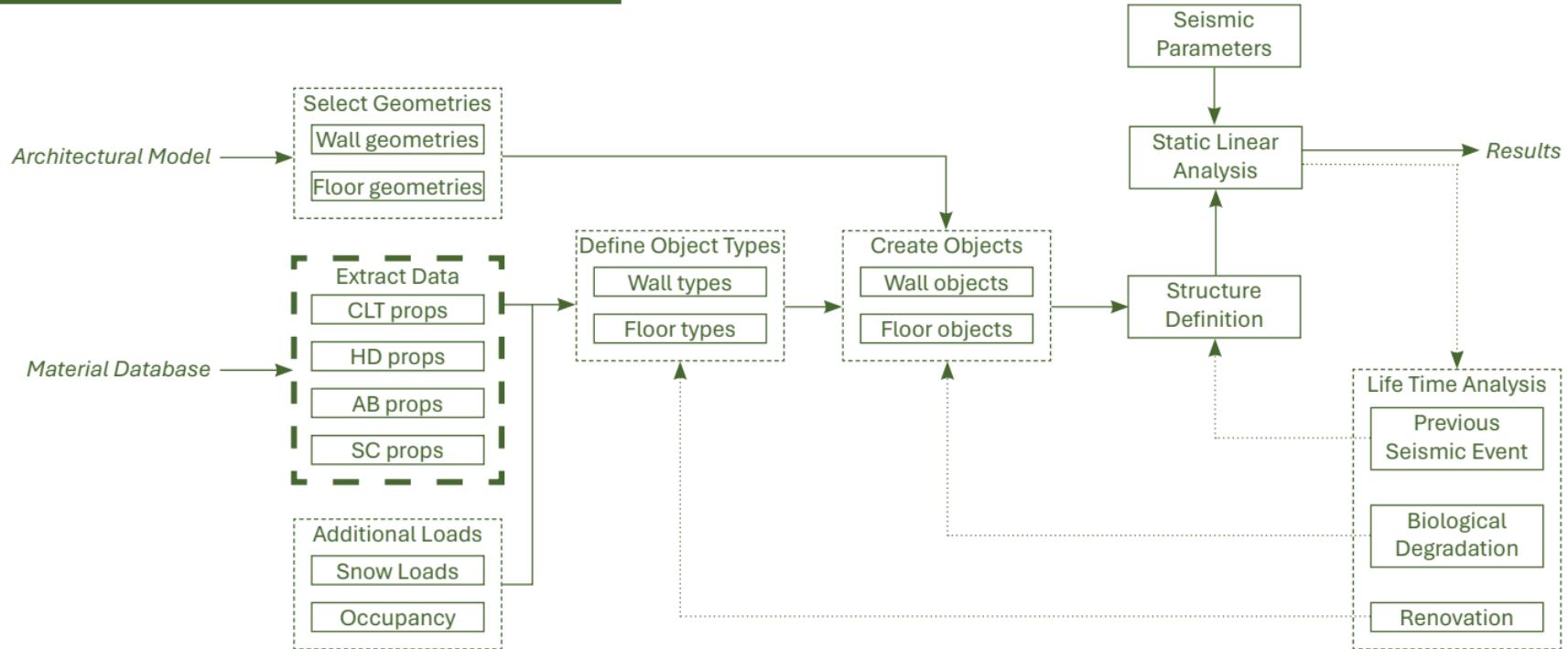
### Floors

Floor type 1

Floor type 2



# The Workflow

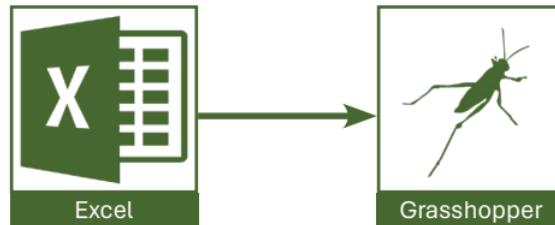


# Model Generation



*Import Material Data to Grasshopper*

- Connect file path to materials database

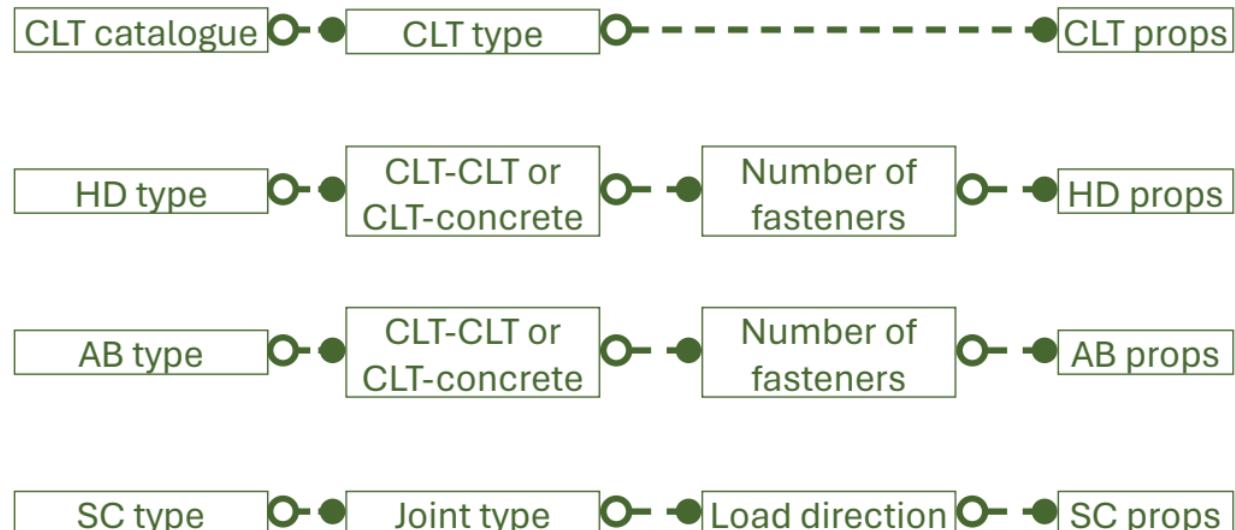


# Model Generation



## *Import Material Data to Grasshopper*

- Connect file path to materials database
- Select identifying properties to select CLT, HD, AB, or SC

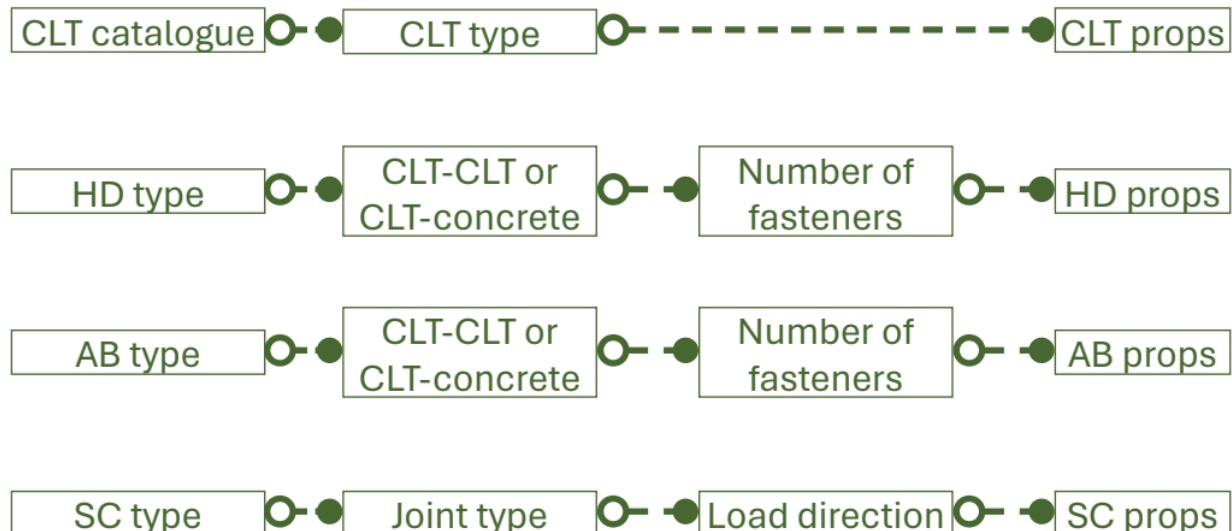


# Model Generation

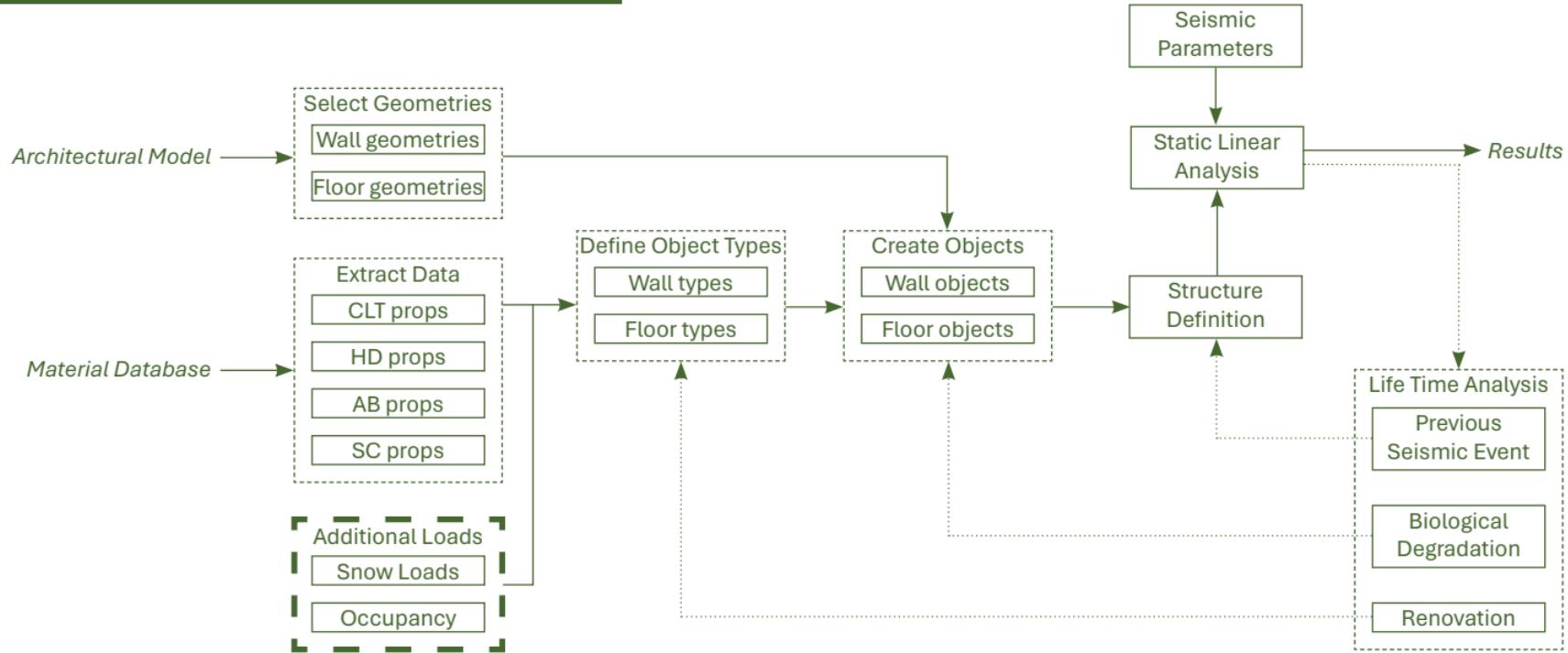


## *Import Material Data to Grasshopper*

- Connect file path to materials database
- Select identifying properties to select CLT, HD, AB, or SC
- Mechanical and mass properties



# The Workflow



# Model Generation



## *Additional Variable Floor Loads*

- Eurocodes require inclusion of reduced variable loads due to snow or occupancy types



*Snow load*



*Variable load*

# Model Generation



## *Additional Variable Floor Loads*

- Eurocodes require inclusion of reduced variable loads due to snow or occupancy types
- Standard loads and reduction factors per Eurocodes provided, user can also input their own

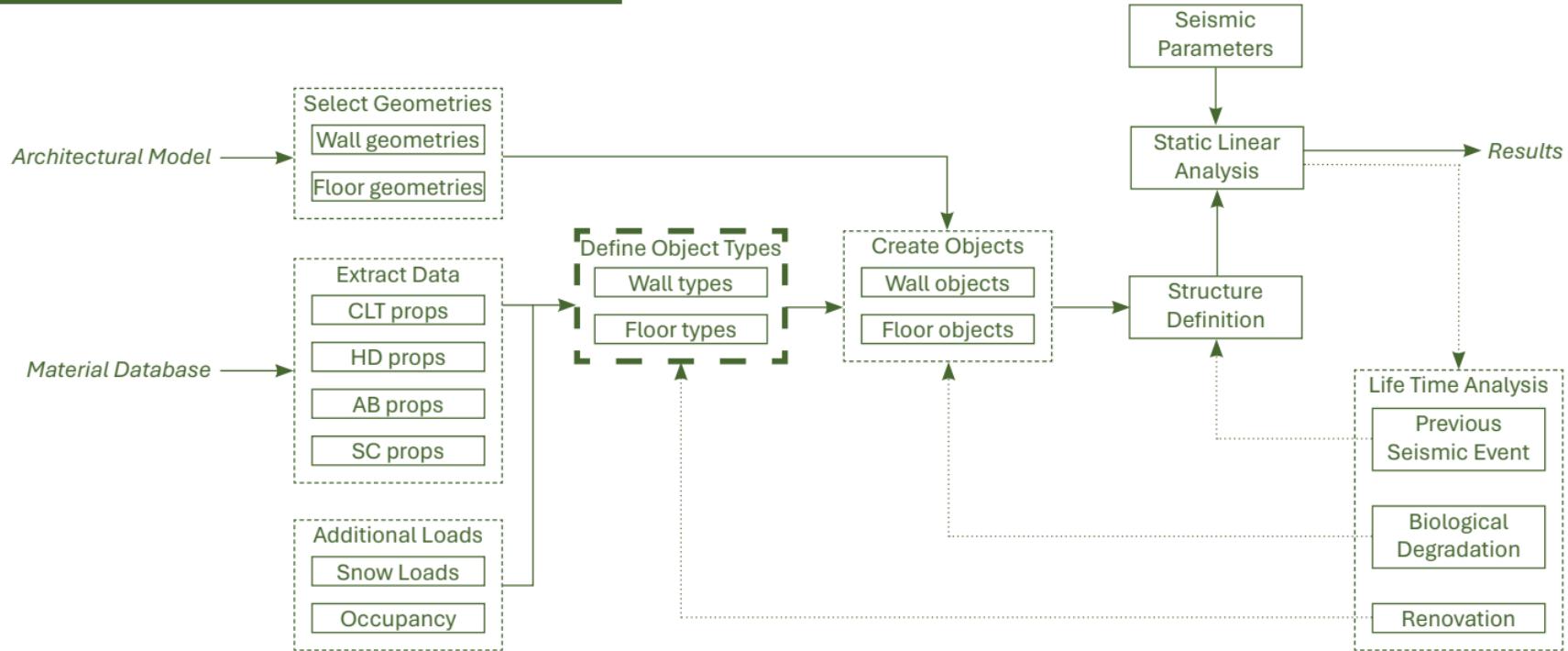


*Snow load*



*Variable load*

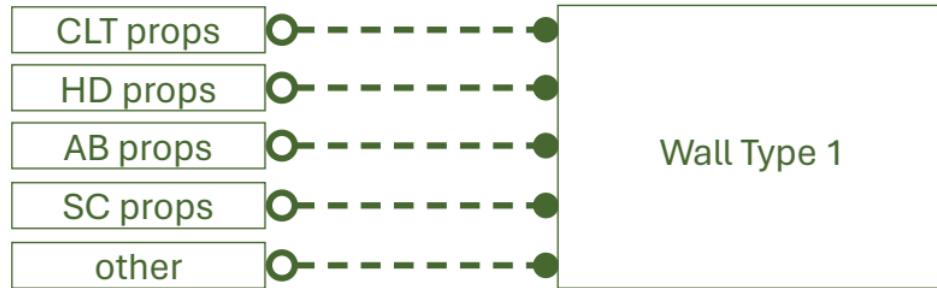
# The Workflow



# Model Generation



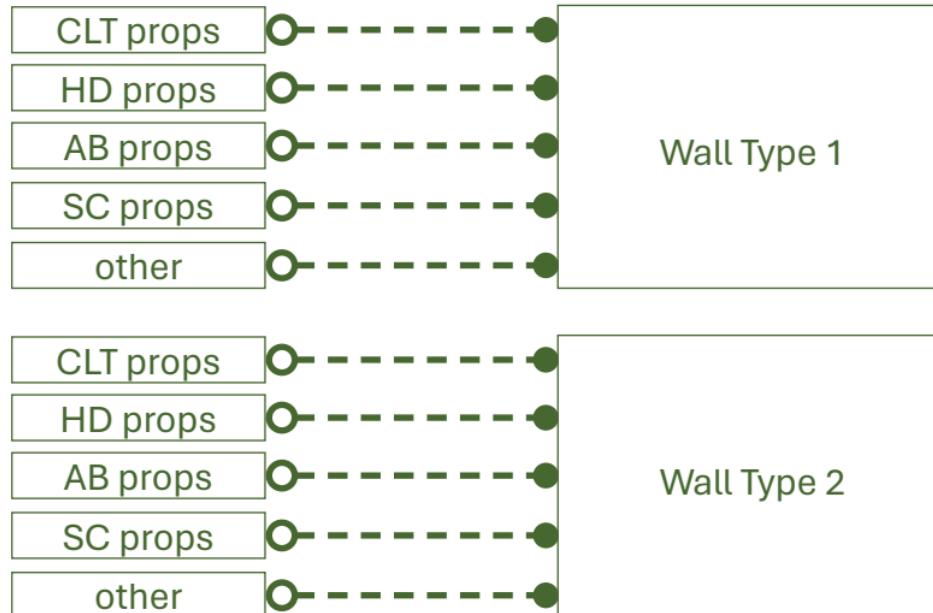
## Wall and Floor Types



# Model Generation



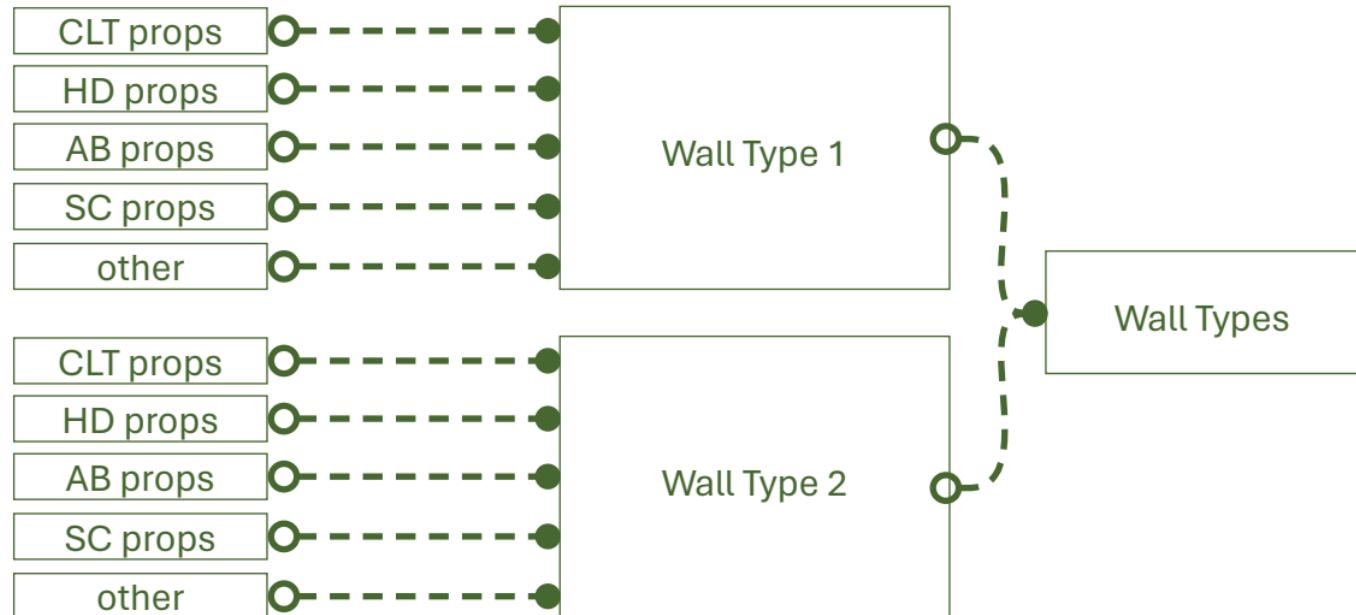
## Wall and Floor Types



# Model Generation



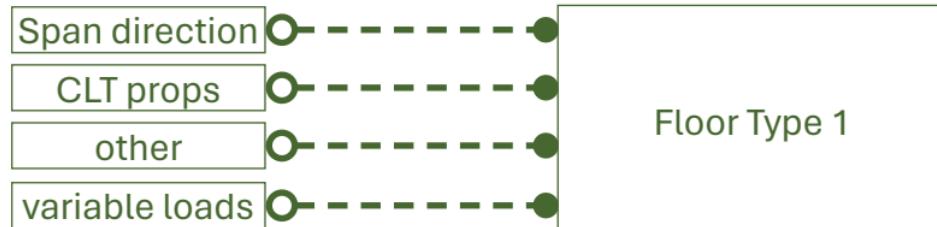
## Wall and Floor Types



# Model Generation



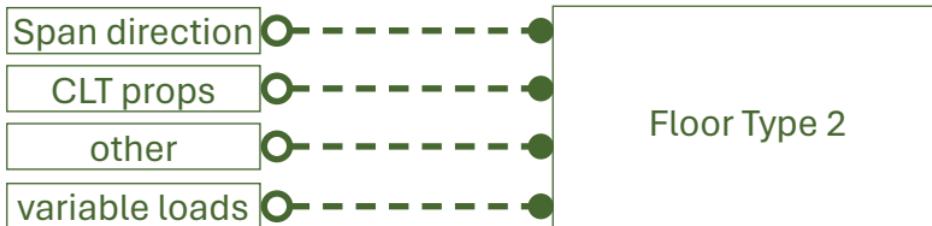
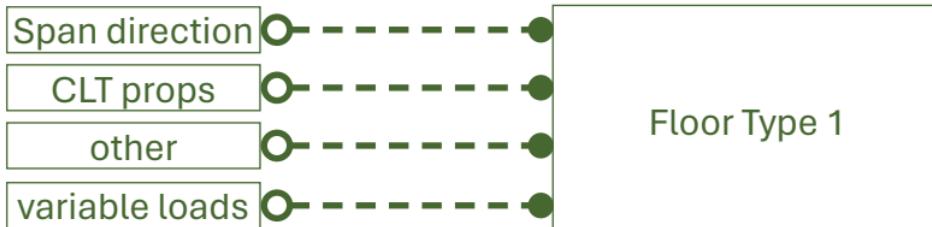
## Wall and Floor Types



# Model Generation



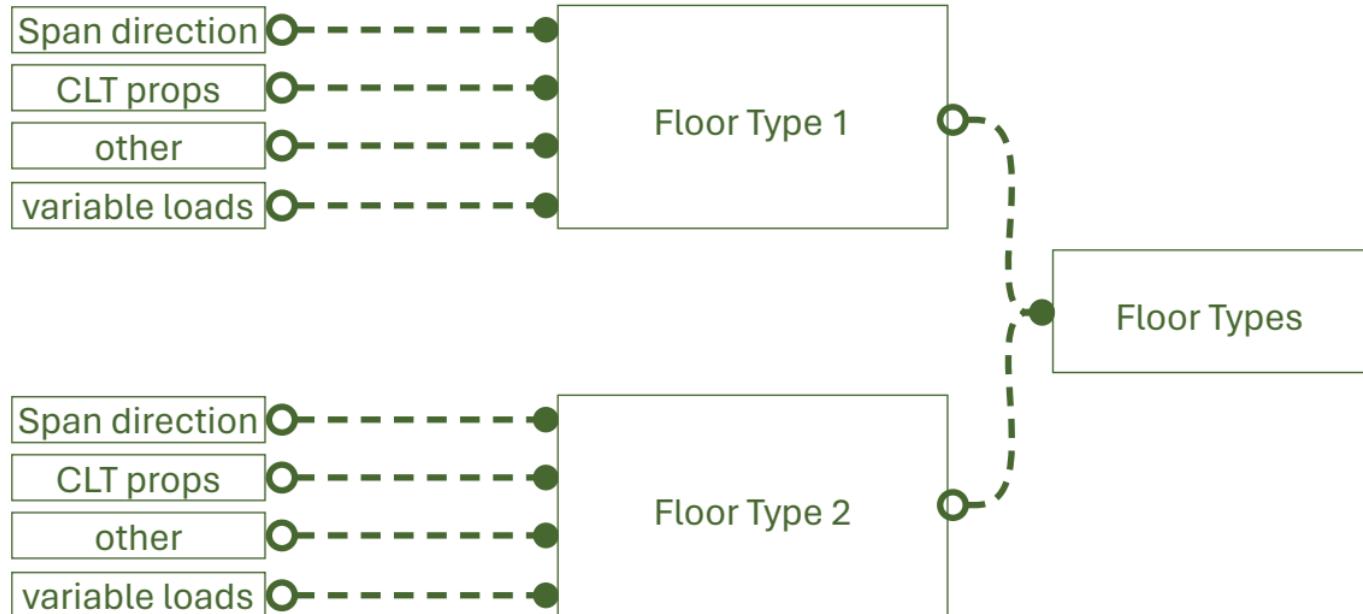
## Wall and Floor Types



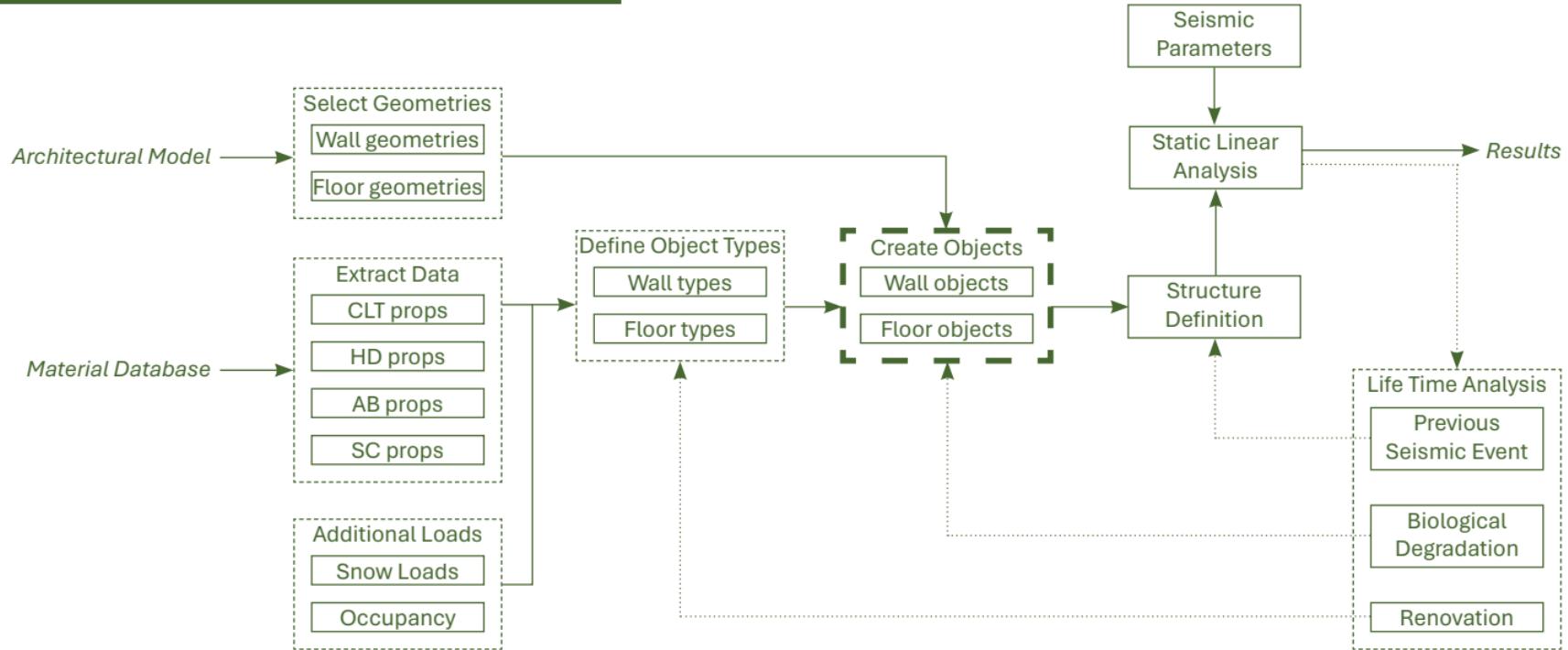
# Model Generation



## Wall and Floor Types



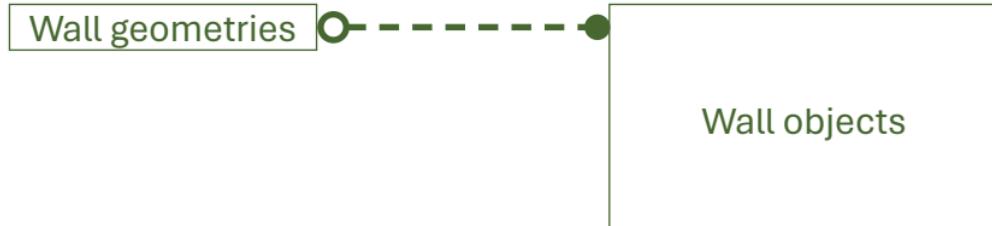
# The Workflow





# Model Generation

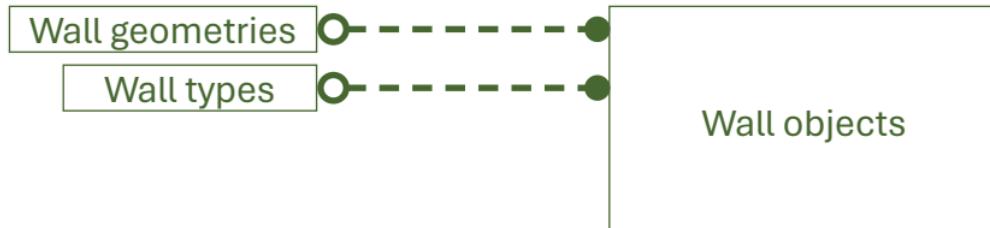
*Wall and Floor Objects*





# Model Generation

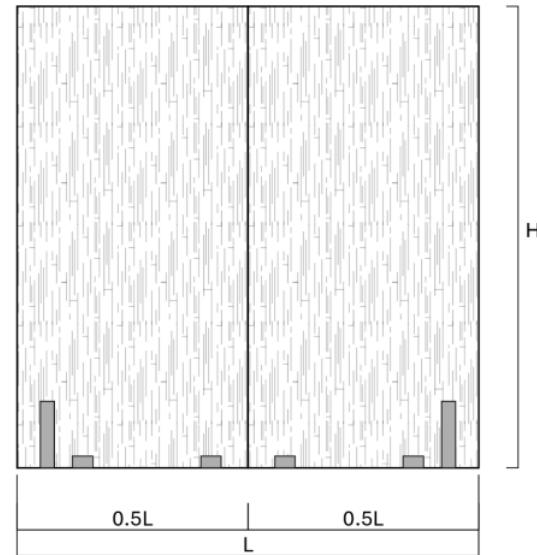
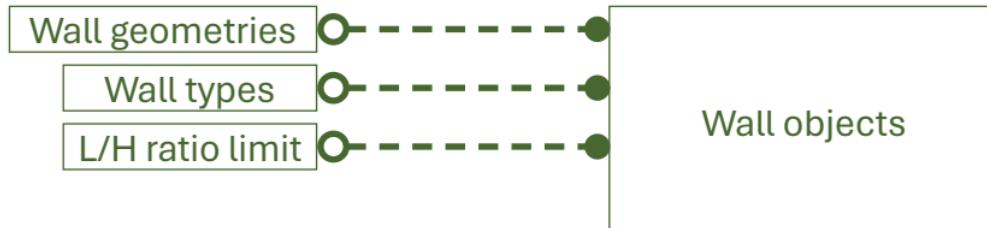
## *Wall and Floor Objects*



# Model Generation



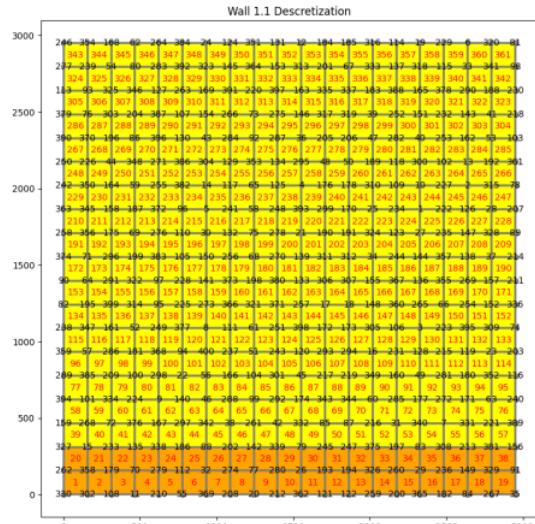
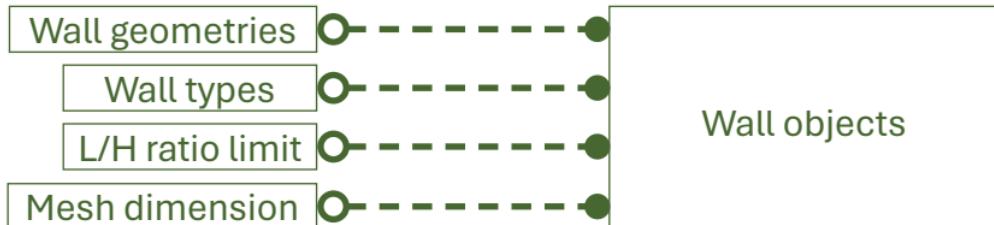
## Wall and Floor Objects



# Model Generation



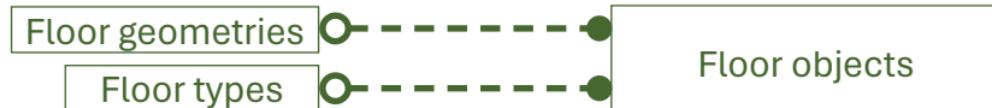
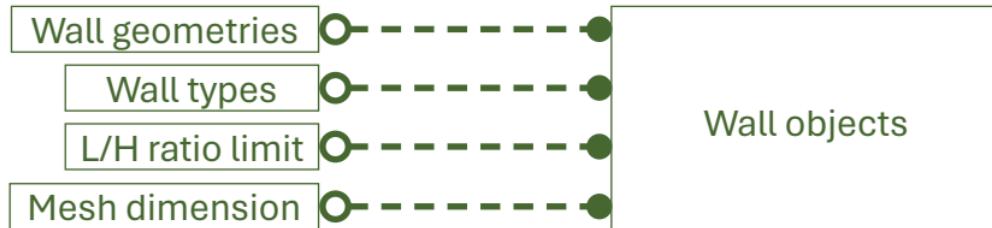
## Wall and Floor Objects





# Model Generation

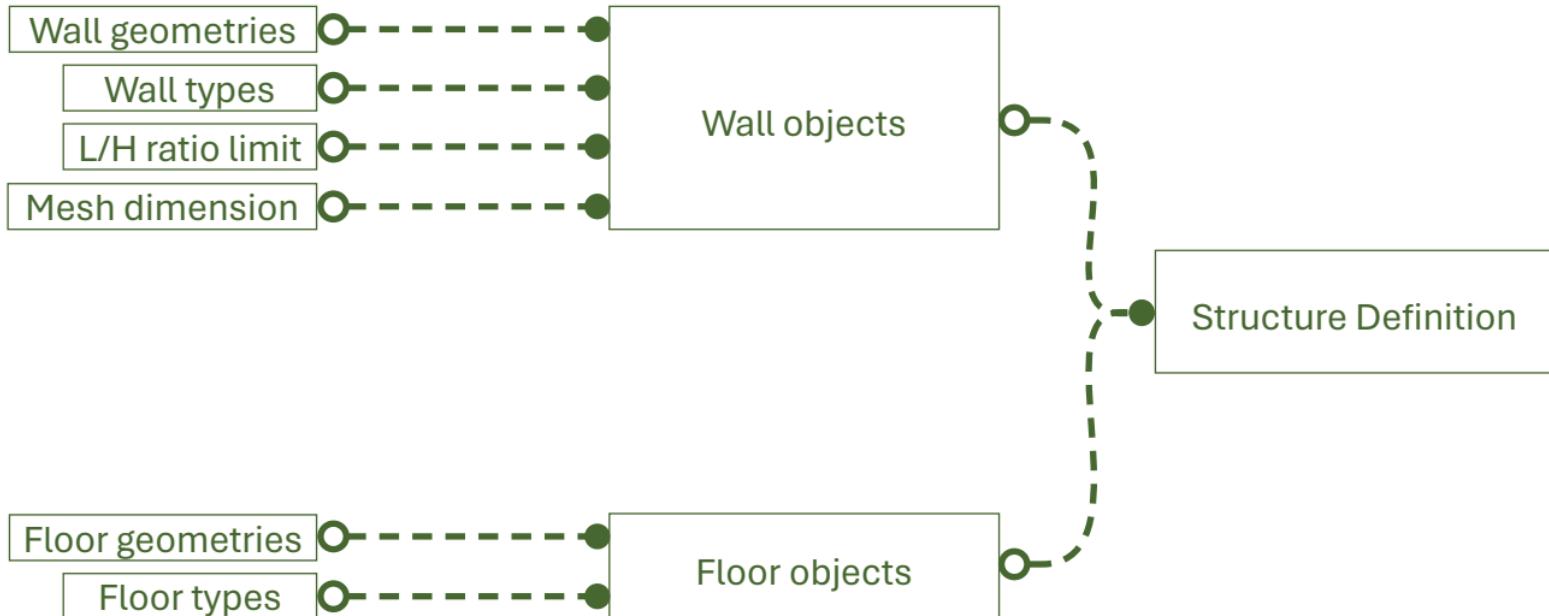
## *Wall and Floor Objects*



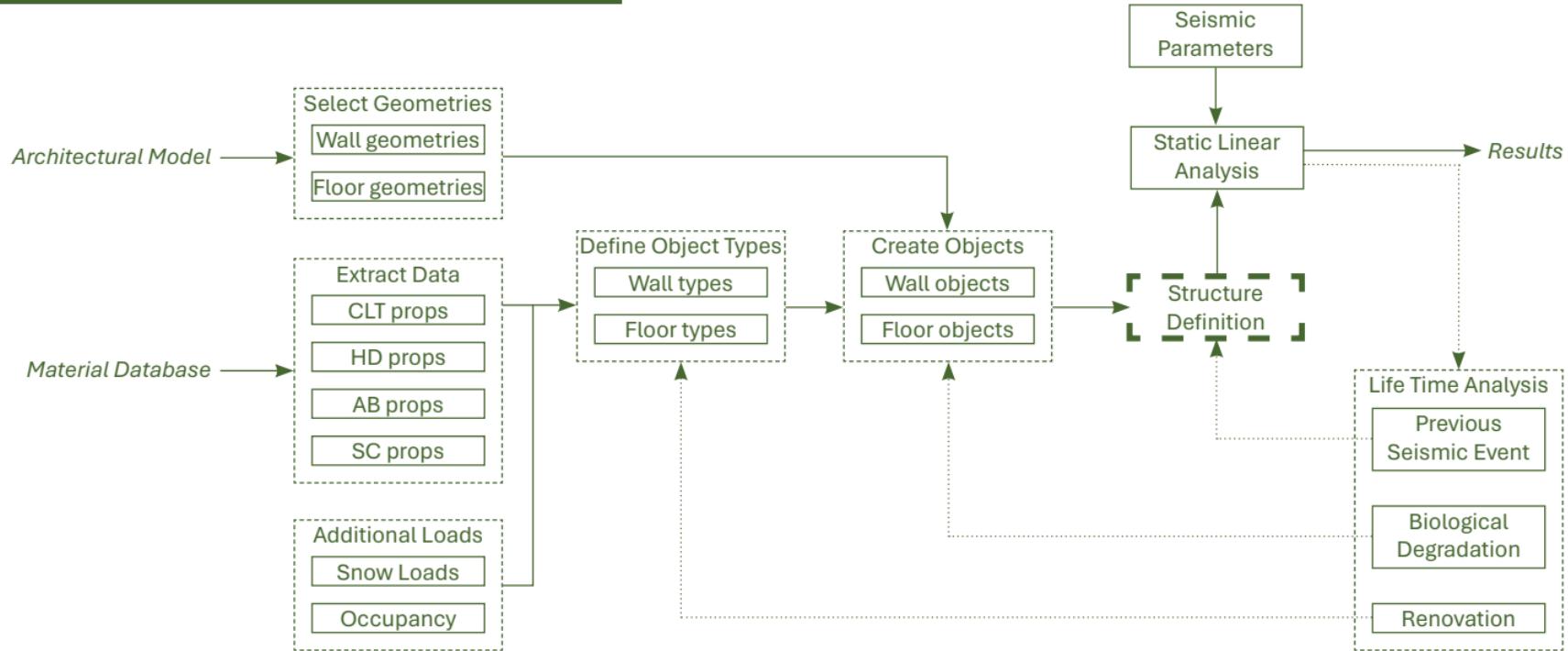


# Model Generation

## Wall and Floor Objects



# The Workflow

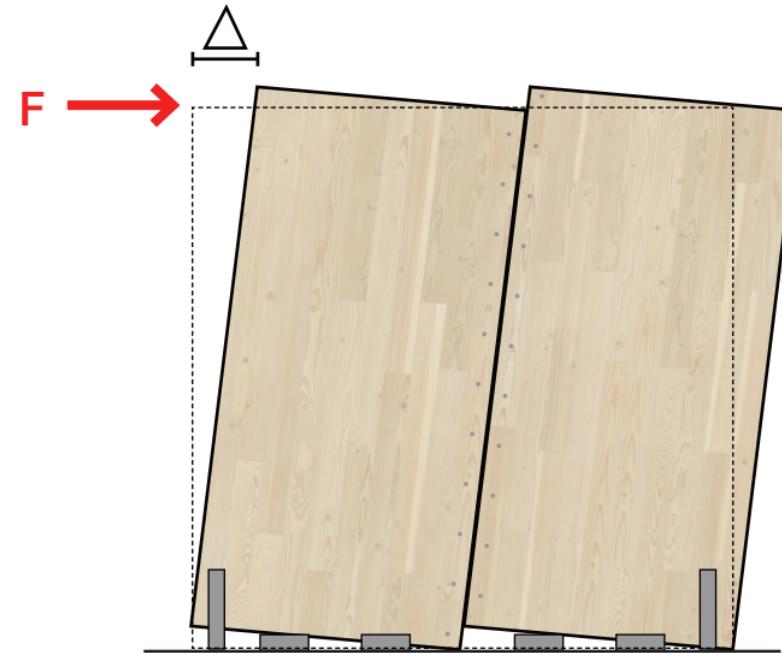




# Model Generation

*Full Structure Definition*

Wall Stiffnesses



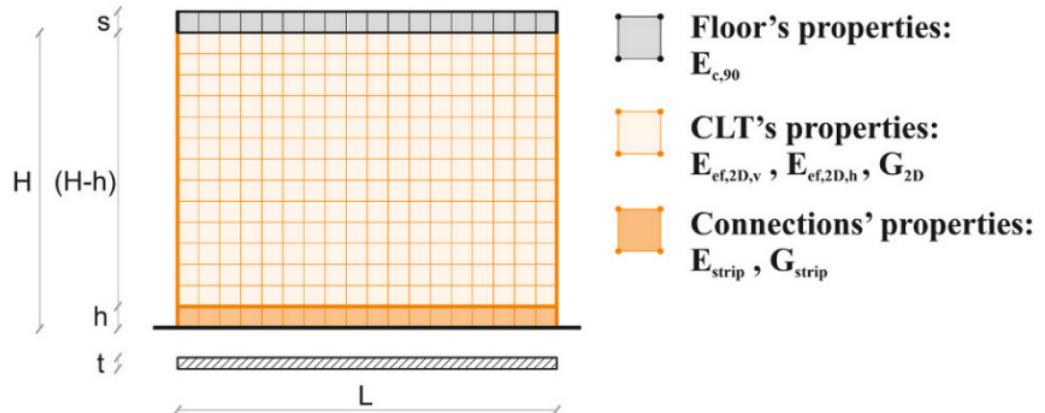


# Model Generation

## Full Structure Definition

### Wall Stiffnesses

- Walls modelled with Rinaldi et al. 2021 strategy



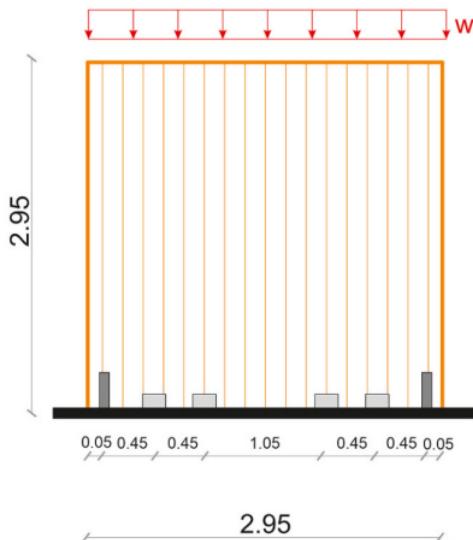
Source: Rinaldi et al. 2021

# Model Generation

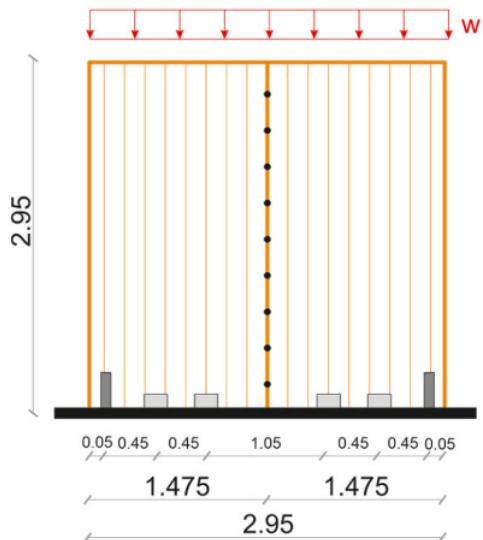


*Full Structure Definition*

Comparative Analysis



*Single-panel wall configurations*



*Multi-panel wall configurations*

# Model Generation



## Full Structure Definition

### Comparative Analysis

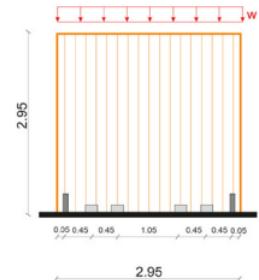
- Compare between implemented model and that of Rinaldi et al. 2021

Table - Comparative analysis results for implementation of Rinaldi modelling strategy

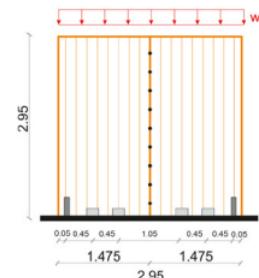
Number of panels	$E_{strip}$ (MPa)			$G_{strip}$ (MPa)			Stiffness (N/mm)			
	Value	Rinaldi	% Diff.	Value	Rinaldi	% Diff.	Value	Rinaldi	% Diff.	
Wall I.1	324.00	371.92	-12.88	3.14	3.13	+0.32	3487	3490	-0.09	
Wall I.2	1	254.00	253.61	+0.15	6.27	6.25	+0.32	6324	5490	+15.19
Wall I.3		129.00	128.70	+0.23	6.27	6.25	+0.32	6288	4620	+36.1
Wall II.1				3.67	3.66	+0.27	4068	4360	-6.7	
Wall III.1				3.11	3.10	+0.32	3501	3770	-7.14	
Wall III.2	2		-	3.11	3.10	+0.32	3501	3770	-7.14	
Wall III.3				3.11	3.10	+0.32	3501	3770	-7.14	
Wall III.4				2.12	2.11	+0.47	2454	2670	-8.09	

\* mesh size of 100 mm

Comparative analysis results



Single-panel wall configurations



Multi-panel wall configurations

# Model Generation



## Full Structure Definition

### Comparative Analysis

- Compare between implemented model and that of Rinaldi et al. 2021
- Mesh dimension needs to be calibrated to match the values

Table – Comparative analysis results for implementation of Rinaldi modelling strategy

	Number of panels	$E_{strip}$ (MPa)			$G_{strip}$ (MPa)			Stiffness (N/mm)		
		Value	Rinaldi	% Diff.	Value	Rinaldi	% Diff.	Value	Rinaldi	% Diff.
Wall I.1		324.00	371.92	-12.88	3.14	3.13	+0.32	3487	3490	-0.09
Wall I.2	1	254.00	253.61	+0.15	6.27	6.25	+0.32	6324	5490	+15.19
Wall I.3		129.00	128.70	+0.23	6.27	6.25	+0.32	6288	4620	+36.1
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Wall III.3					3.11	3.10	+0.32	3501	3770	-7.14
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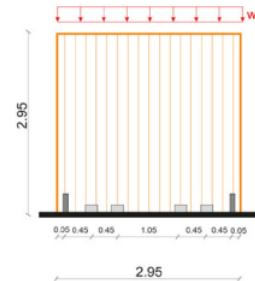
\* mesh size of 100 mm

Comparative analysis results

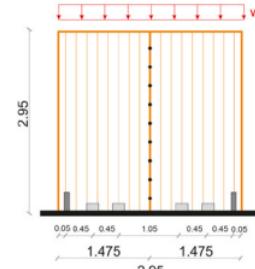
Table – Comparative analysis of stiffnesses with varied mesh dimensions

Mesh dim.	No.	R.	Stiffness (N/mm)									
			-	50	%	100	%	112.5	%	125	%	
Wall I.1		3490	2888	-17.25	3487	-0.09	3159	-9.48	2825	-19.05	2367	-32.18
Wall I.2	1	5490	5316	-3.17	6324	+15.19	5780	+5.28	5216	-4.99	4428	-19.34
Wall I.3		4620	5285	+14.39	6288	+36.1	5751	+24.48	5192	+12.38	4410	-4.55
Wall II.1		4360	3376	-22.57	4068	-6.7	3688	-15.41	3299	-24.33	2767	-36.54
Wall III.1		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.2	2	3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.3		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.4		2670	2021	-24.31	2454	-8.09	2215	-17.04	1973	-26.01	1645	-38.39

Mesh dimension calibration



Single-panel wall configurations



Multi-panel wall configurations

# Model Generation



## Full Structure Definition

### Comparative Analysis

- Compare between implemented model and that of Rinaldi et al. 2021
- Mesh dimension needs to be calibrated to match the values
- Currently underestimating by 8% or less\* with a mesh dimension of 100 mm

Table – Comparative analysis results for implementation of Rinaldi modelling strategy

Number of panels	$E_{strip}$ (MPa)			$G_{strip}$ (MPa)			Stiffness (N/mm)			
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Wall I.3		129.00	128.70	+0.23	6.27	6.25	+0.32	6288	4620	+36.1
Wall II.1				3.67	3.66	+0.27	4068	4360	-6.7	
Wall III.1				3.11	3.10	+0.32	3501	3770	-7.14	
Wall III.2	2		-	3.11	3.10	+0.32	3501	3770	-7.14	
Wall III.3				3.11	3.10	+0.32	3501	3770	-7.14	
Wall III.4				2.12	2.11	+0.47	2454	2670	-8.09	

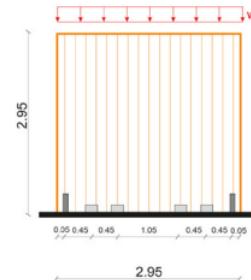
\* mesh size of 100 mm

### Comparative analysis results

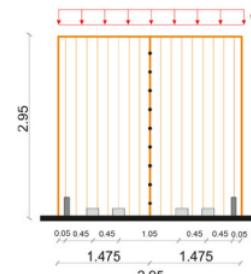
Table – Comparative analysis of stiffnesses with varied mesh dimensions

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Wall II.1		4360	3376	-22.57	4068	-6.7	3688	-15.41	3299	-24.33	2767	-36.54
Wall III.1		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.2	2	3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.3		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.4		2670	2021	-24.31	2454	-8.09	2215	-17.04	1973	-26.01	1645	-38.39

### Mesh dimension calibration



Single-panel wall configurations



Multi-panel wall configurations

# Model Generation



## Full Structure Definition

### Comparative Analysis

- Compare between implemented model and that of Rinaldi et al. 2021
- Mesh dimension needs to be calibrated to match the values
- Currently underestimating by 8% or less\* with a mesh dimension of 100 mm

- \*Walls with 4 rather than 2 angle brackets are outliers, overestimating stiffness by up to 36%

Table – Comparative analysis results for implementation of Rinaldi modelling strategy

	Number of panels	$E_{strip}$ (MPa)			$G_{strip}$ (MPa)			Stiffness (N/mm)		
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Wall III.3					3.11	3.10	+0.32	3501	3770	-7.14
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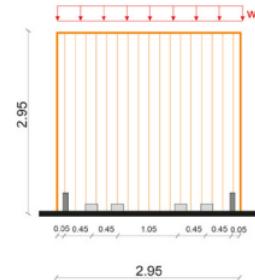
\* mesh size of 100 mm

Comparative analysis results

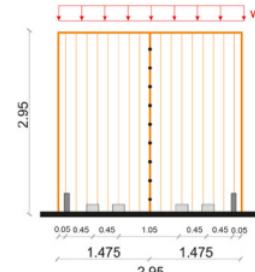
Table – Comparative analysis of stiffnesses with varied mesh dimensions

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		-	50	%	100	%	112.5	%	125	%	150	%
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Wall III.1		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.2	2	3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.3		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.4		2670	2021	-24.31	2454	-8.09	2215	-17.04	1973	-26.01	1645	-38.39

Mesh dimension calibration



Single-panel wall configurations



Multi-panel wall configurations

# Model Generation



## Full Structure Definition

### Wall Stiffnesses

- Walls modelled with Rinaldi et al. 2021 strategy
- Pushover analysis



246	354	168	62	264	284	24	124	291	181	10	184	185	216	114	10	219	6	230	91	
343	344	345	446	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361		
277	239	54	17	263	302	233	145	314	153	313	201	67	313	157	110	115	313	311	98	
324	325	326	277	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342		
113	93	325	345	127	263	169	391	210	297	163	335	337	183	368	165	378	290	188	230	
305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323		
379	75	303	204	367	107	154	266	73	219	146	317	319	319	252	151	232	143	41	248	
286	287	288	289	290	291	292	293	294	295	296	297	298	299	299	300	301	302	303	304	
360	370	196	116	266	300	130	43	284	32	267	36	205	206	47	262	40	253	142	53	103
267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285		
250	216	41	148	211	266	304	129	333	134	295	48	50	189	118	300	102	13	192	361	
241	249	270	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266		
242	350	164	59	258	382	4	117	65	125	4	176	178	310	109	10	217	3	315	78	
222	230	731	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247		
363	345	154	187	372	98	5	241	58	248	393	299	170	25	234	1	232	136	29	207	
210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228		
268	356	175	69	236	110	30	132	75	278	71	190	181	324	123	27	235	147	328	89	
191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209		
374	71	216	109	382	105	150	256	68	199	202	170	311	312	20	244	144	357	138	37	244
172	177	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190		
90	61	61	312	97	178	141	313	198	380	133	306	307	155	367	136	355	269	157	241	
153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171		
62	195	314	95	235	273	366	231	371	257	17	18	149	326	265	66	254	152	336		
134	145	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152		
288	347	161	52	249	317	0	111	61	251	398	172	173	305	106	3	233	395	309	74	
115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133		
350	57	266	161	268	94	400	237	53	243	120	203	204	16	231	128	215	119	23	203	
96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114		
269	345	209	100	296	22	56	166	104	301	45	217	219	349	160	49	261	160	352	116	
71	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95		
304	101	334	224	0	140	46	288	69	292	174	343	344	60	285	177	272	171	63	240	
56	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76		
159	268	72	316	167	297	342	38	261	42	332	85	87	210	31	340	7	331	211	389	
9	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57		
347	15	233	135	338	166	88	202	142	339	79	245	247	375	197	83	308	213	361	156	
10	21	22	23	74	25	26	27	78	29	30	31	32	33	34	35	36	37	38		
261	358	179	70	279	112	32	274	77	280	20	193	194	316	260	29	236	149	329	41	
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19		
380	302	100	11	280	55	369	200	20	242	62	121	122	259	200	56	192	85	267	45	



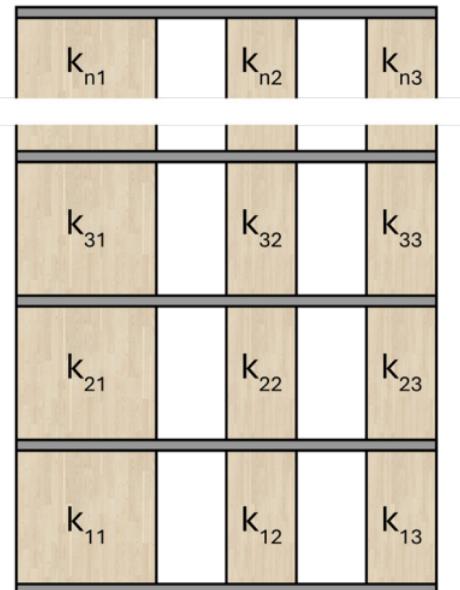
# Model Generation

## *Full Structure Definition*

### Wall Stiffnesses

- Walls modelled with Rinaldi et al. 2021 strategy
- Pushover analysis

### Structure Parameters





# Model Generation

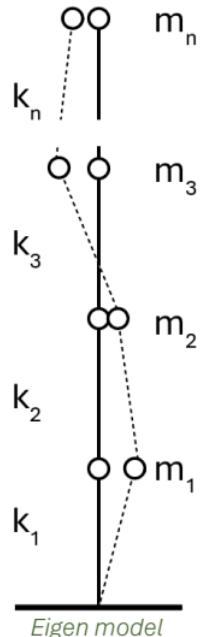
## *Full Structure Definition*

### Wall Stiffnesses

- Walls modelled with Rinaldi et al. 2021 strategy
- Pushover analysis

### Structure Parameters

- Eigen analysis





# Model Generation

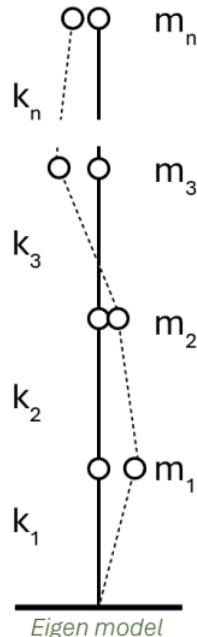
## Full Structure Definition

### Wall Stiffnesses

- Walls modelled with Rinaldi et al. 2021 strategy
- Pushover analysis

### Structure Parameters

- Eigen analysis
- Natural building period

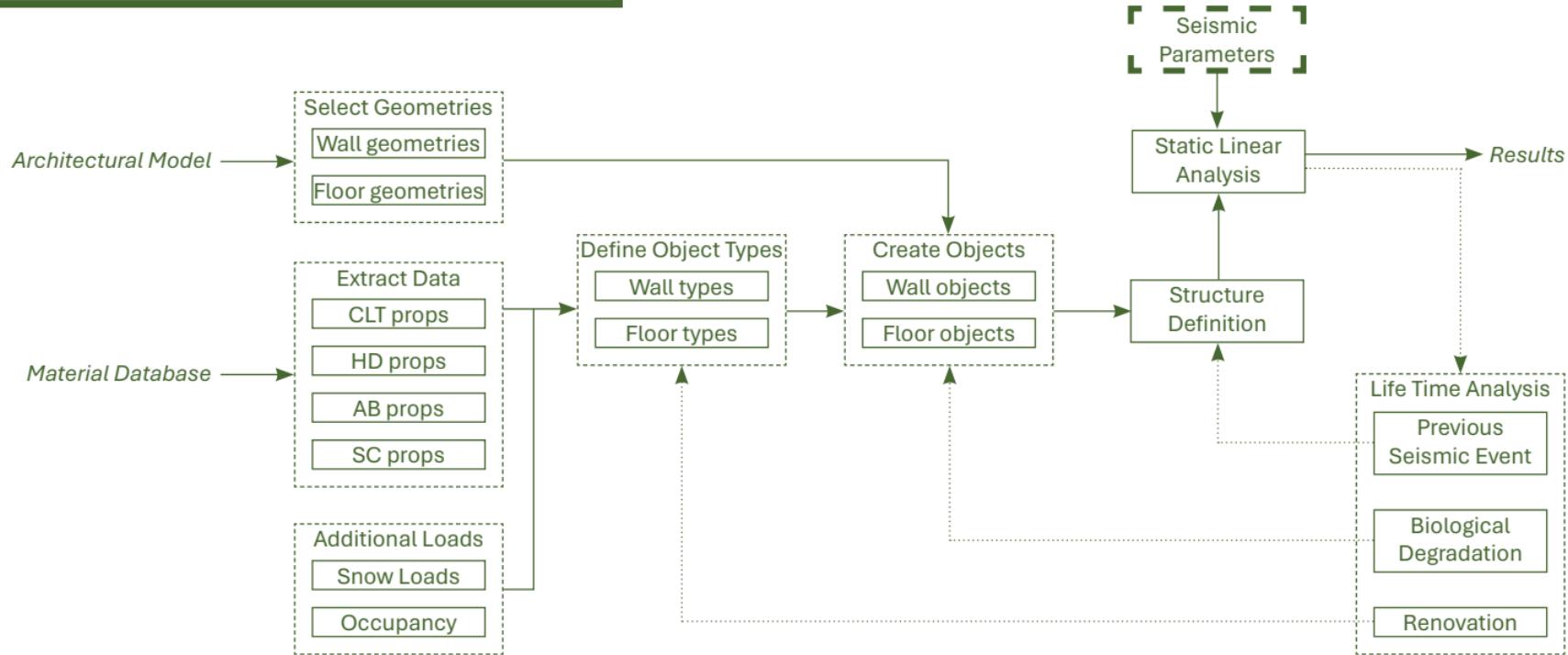


$$\omega = \sqrt{\frac{m}{k}} = \text{eigen vector}$$

$$T = 2\pi\omega$$

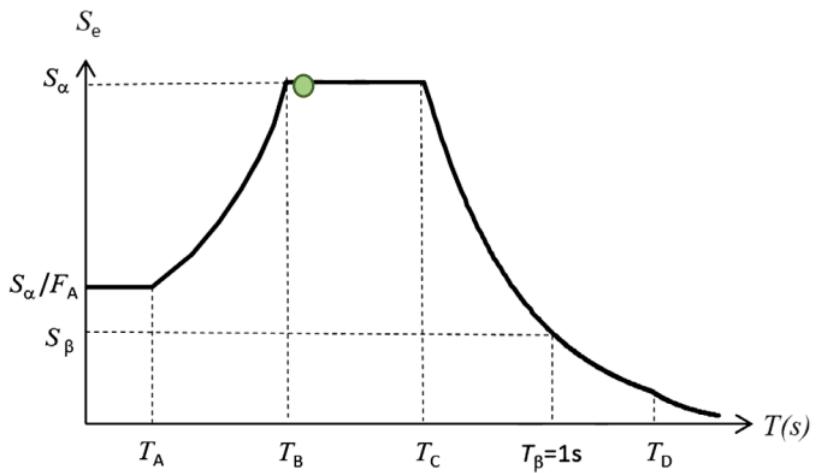
Relationship b/t eigen analysis and building period

# The Workflow



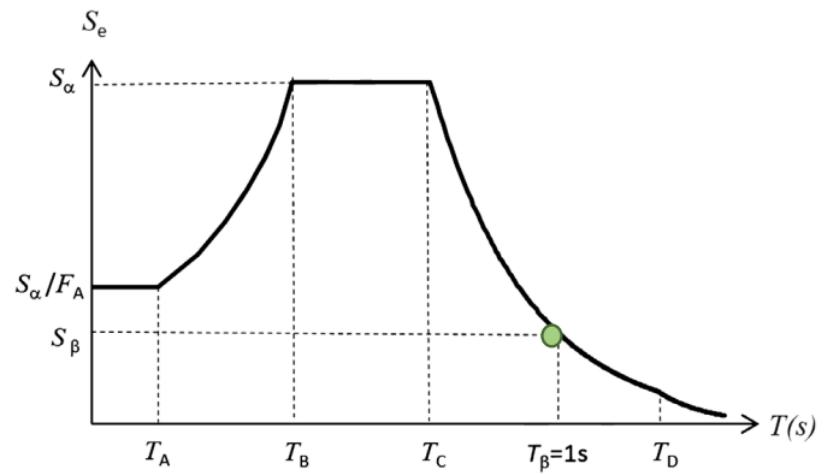
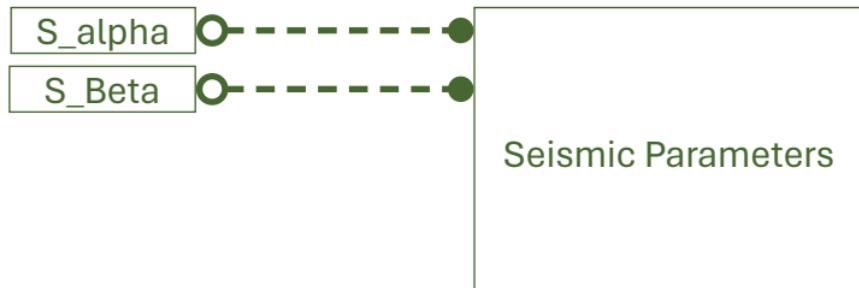


# Seismic Parameters



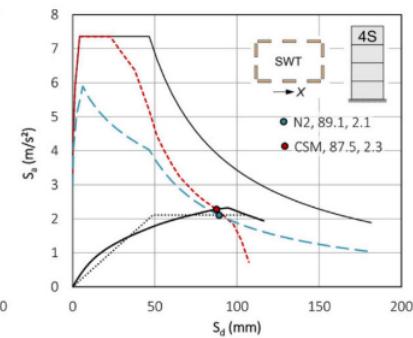
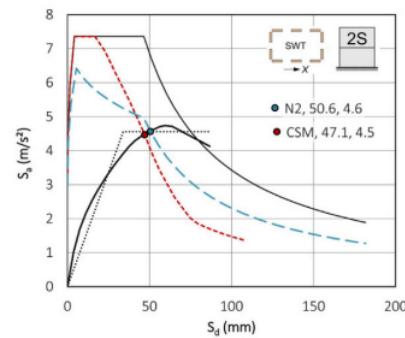
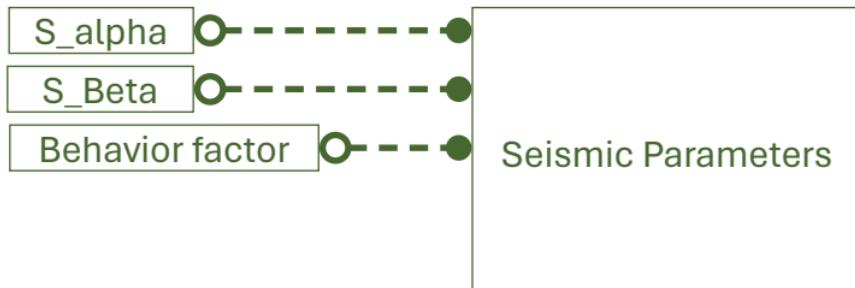


# Seismic Parameters





# Seismic Parameters



Behavior factor  $q = 3.0$

Source: Hummel and Seim, 2019



# Seismic Parameters

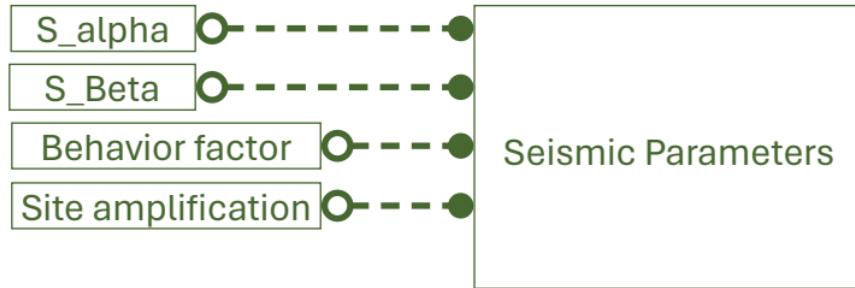


Table 5.4 — Site amplification factors  $F_\alpha$  and  $F_\beta$  for the standard site categories of Table 5.1

Site category	$F_\alpha$		$F_\beta$	
	$H_{800}$ and $v_{s,H}$ available	Default value	$H_{800}$ and $v_{s,H}$ available	Default value
A	1,0	1,0	1,0	1,0
B	$\left(\frac{v_{s,H}}{800}\right)^{-0,40 r_\alpha}$	1,3 (1 - 0,1 $S_{\alpha,RP}/g$ )	$\left(\frac{v_{s,H}}{800}\right)^{-0,70 r_\beta}$	1,6 (1 - 0,2 $S_{\beta,RP}/g$ )
C		1,6 (1 - 0,2 $S_{\alpha,RP}/g$ )		2,3 (1 - 0,3 $S_{\beta,RP}/g$ )
D		1,8 (1 - 0,3 $S_{\alpha,RP}/g$ )		3,2 (1 - $S_{\beta,RP}/g$ )
E	$\left(\frac{v_{s,H}}{800}\right)^{-0,40 r_\alpha \frac{H}{50} \left(4 - \frac{H}{10}\right)}$	2,2 (1 - 0,5 $S_{\alpha,RP}/g$ )	$\left(\frac{v_{s,H}}{800}\right)^{-0,70 r_\beta \frac{H}{50}}$	3,2 (1 - $S_{\beta,RP}/g$ )
F	$0,90 \left(\frac{v_{s,H}}{800}\right)^{-0,40 r_\alpha}$	1,7 (1 - 0,3 $S_{\alpha,RP}/g$ )	$1,25 \left(\frac{v_{s,H}}{800}\right)^{-0,70 r_\beta}$	4,0 (1 - $S_{\beta,RP}/g$ )
	with $r_\alpha = 1 - \frac{S_{\alpha,RP}/g}{v_{s,H}/150}$ and $r_\beta = 1 - \frac{S_{\beta,RP}/g}{v_{s,H}/150}$			

Site amplification factors per EN 1998

# Seismic Parameters

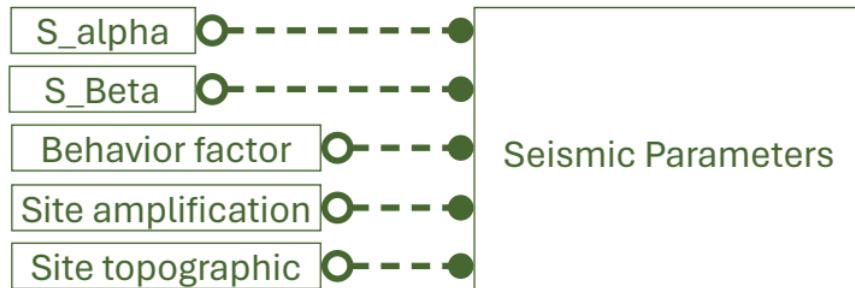
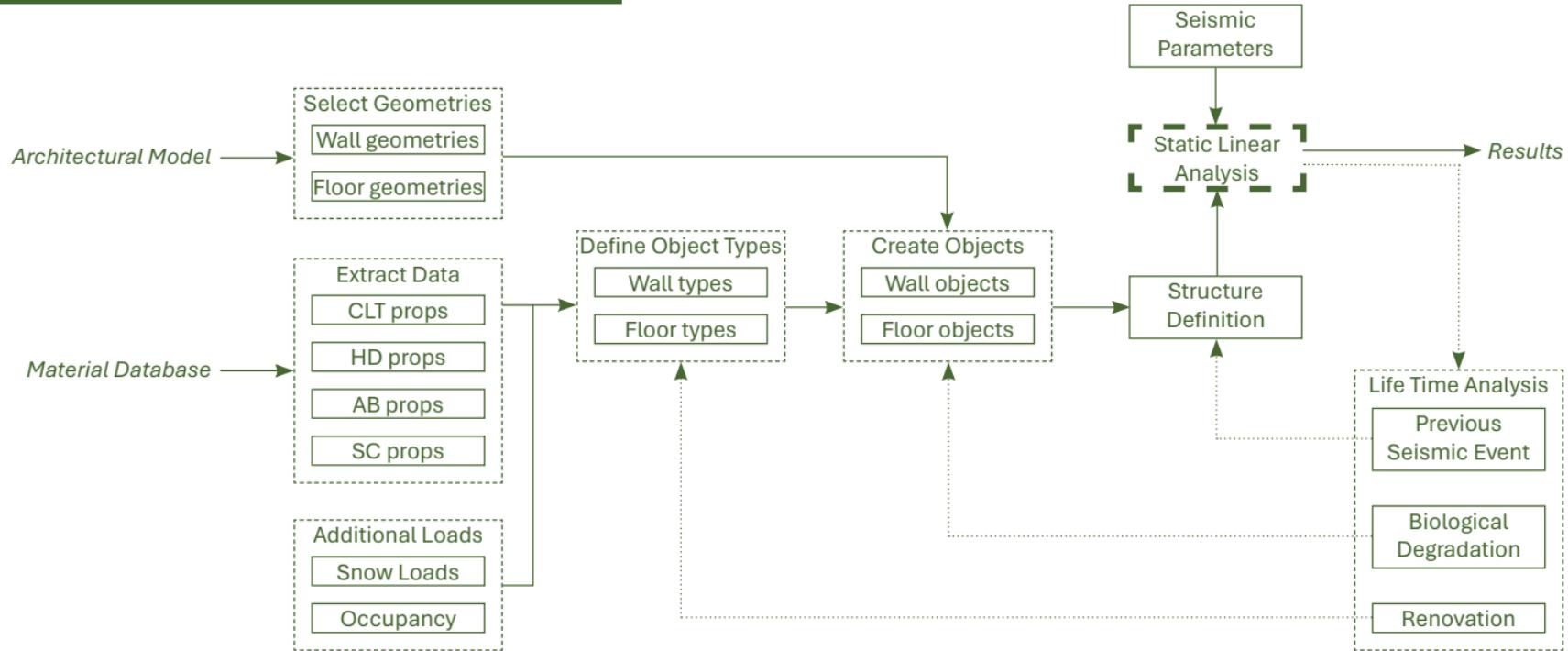


Table 5.5 — Topography amplification factors for simple topographic irregularities

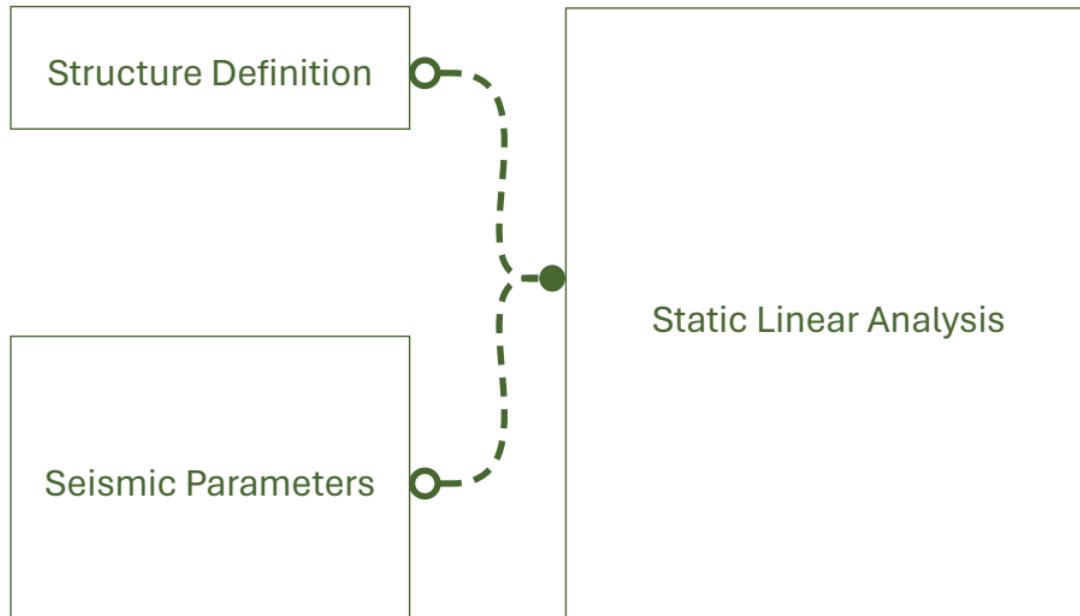
Topography description	$F_T$	Simplified sketch*
Flat ground surface, slopes and isolated ridges with average slope angle $i < 15^\circ$ or height < 30 m	1,0	
Slopes with average slope angle $i > 15^\circ$	1,2	
Ridges with width at the top much smaller than at the base and average slope angle $15^\circ < i < 30^\circ$	1,2	
Ridges with width at the top much smaller than at the base and average slope angle $i > 30^\circ$	1,4	

Topographic amplification factors per EN 1998

# The Workflow



# Static Linear Analysis

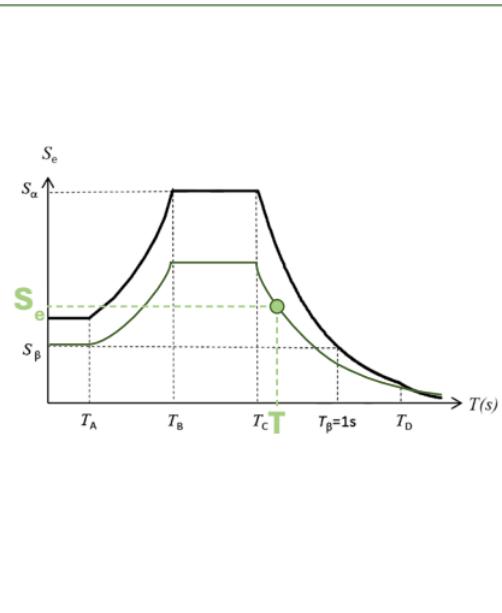


# Static Linear Analysis



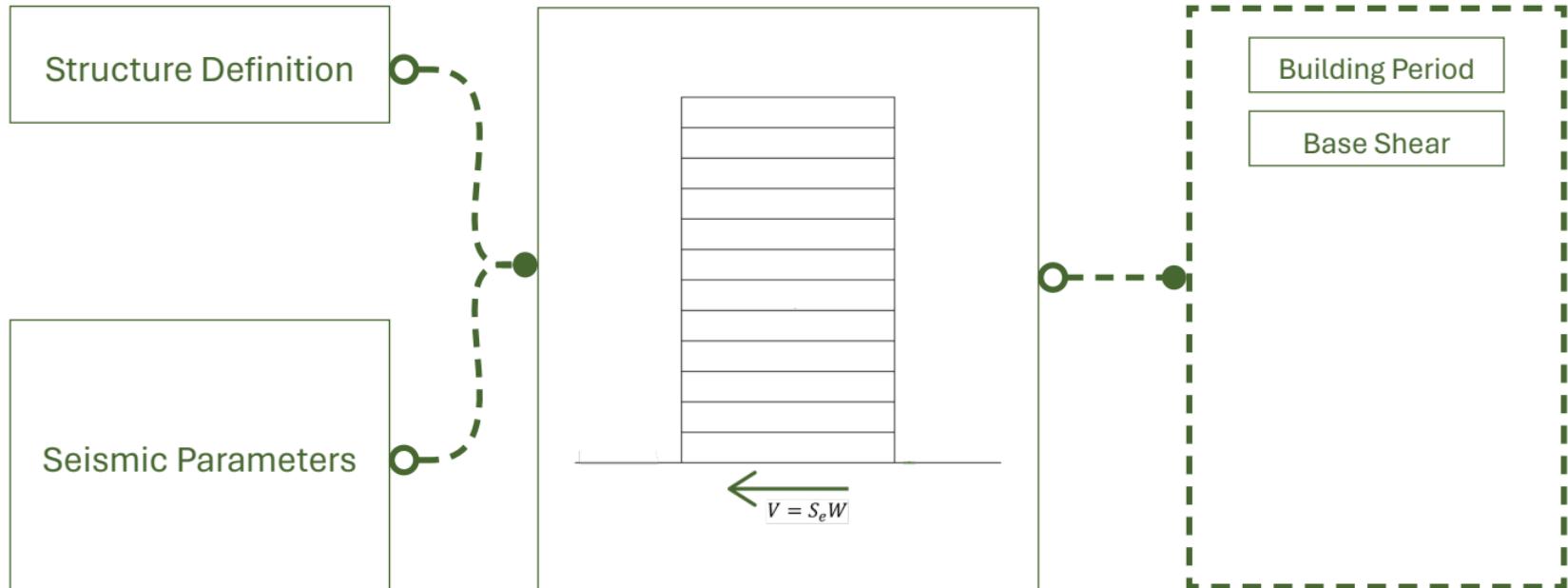
Structure Definition

Seismic Parameters

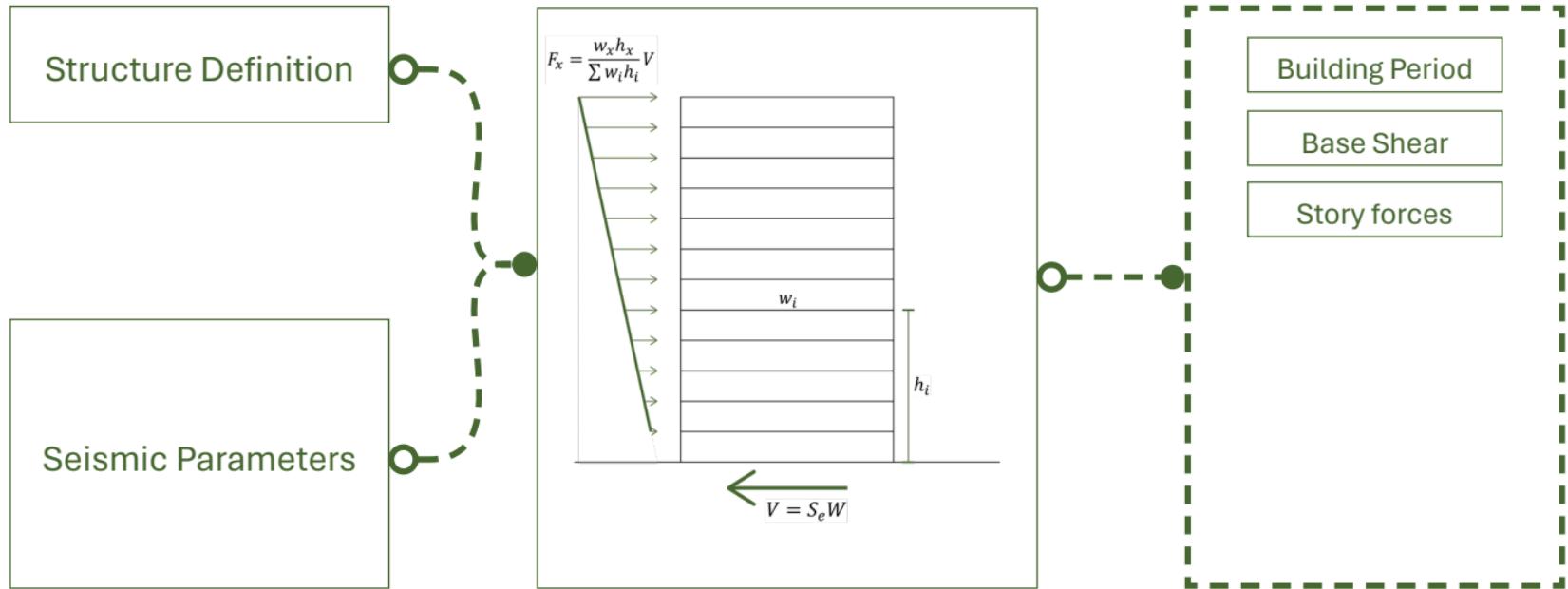


Building Period

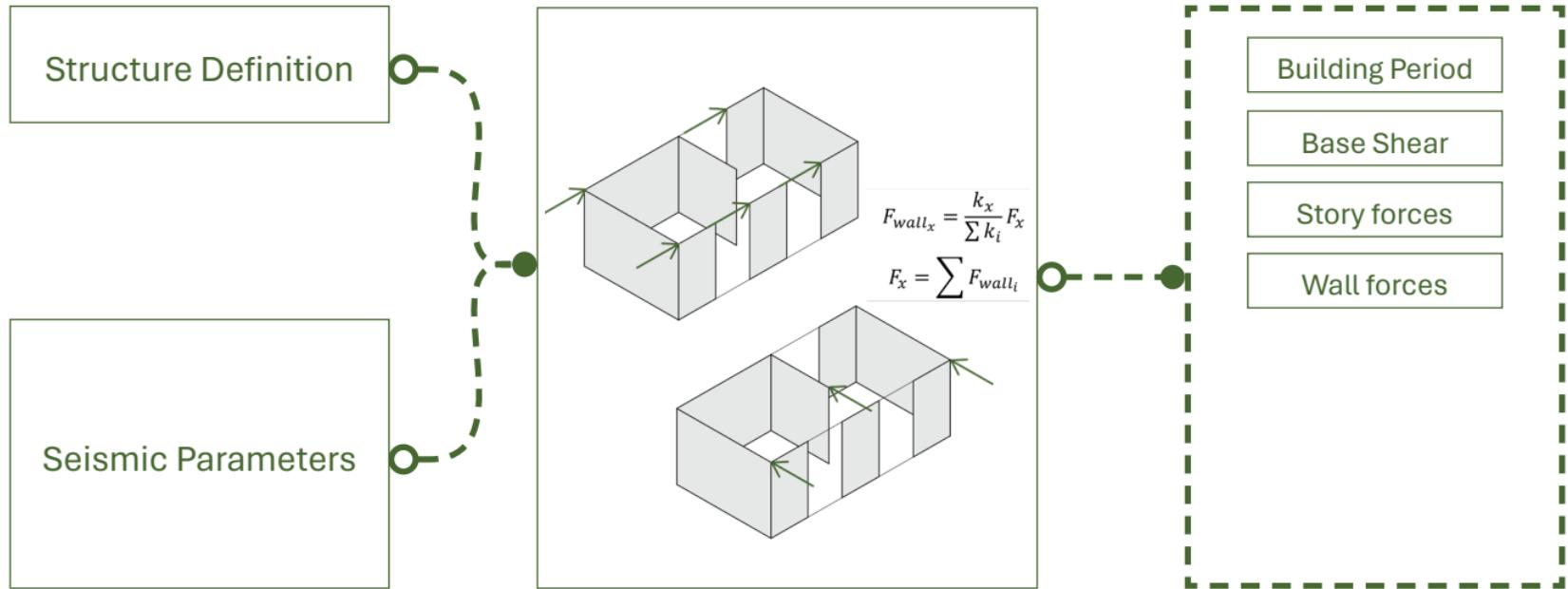
# Static Linear Analysis



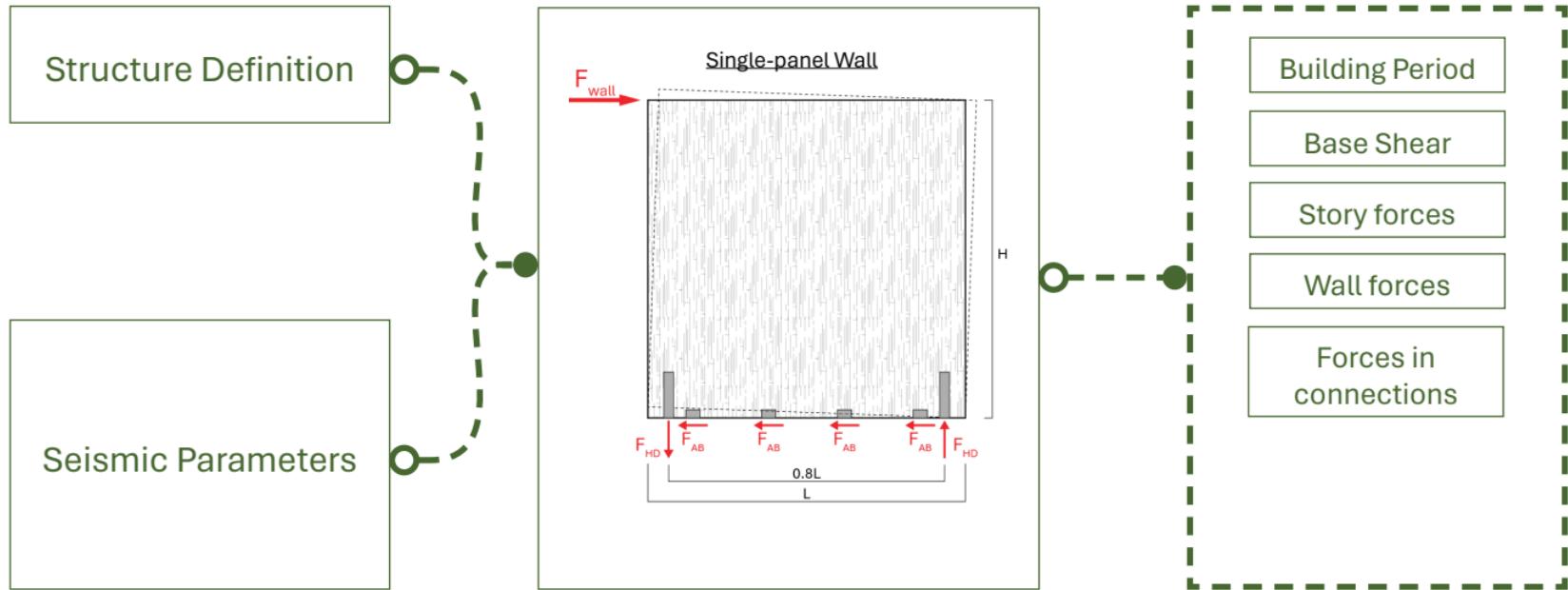
# Static Linear Analysis



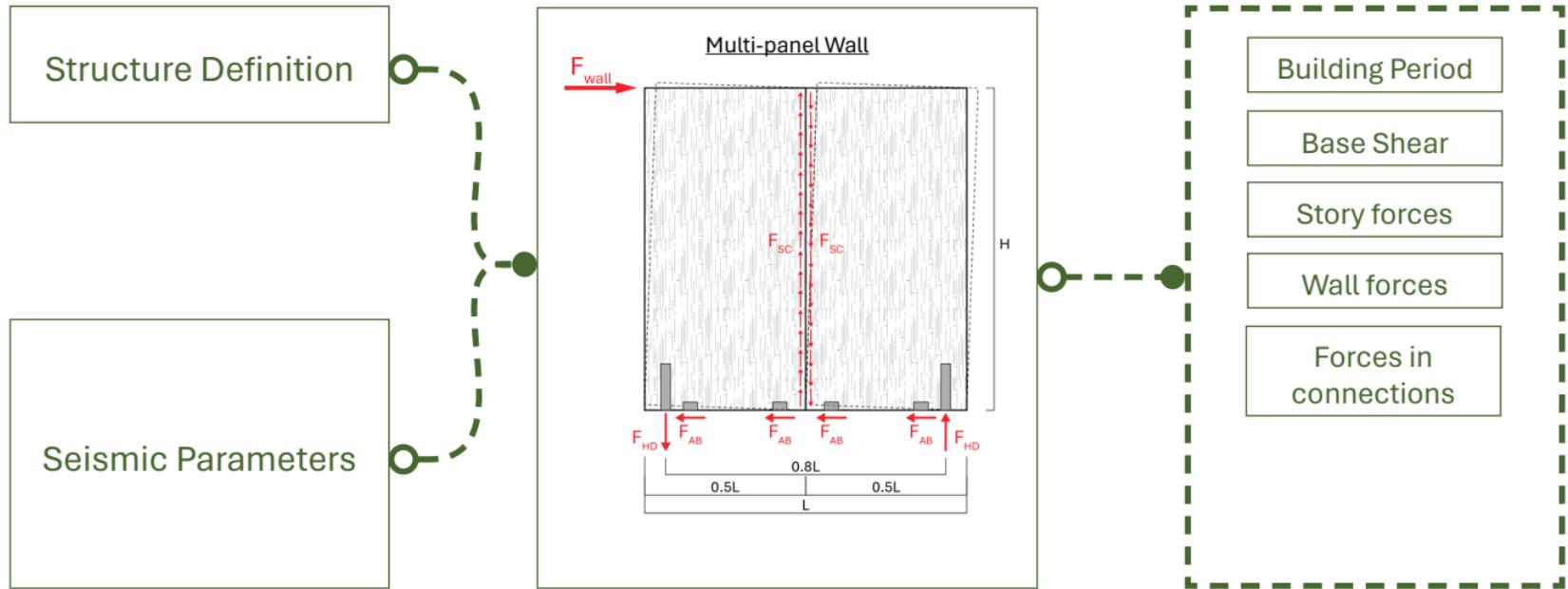
# Static Linear Analysis



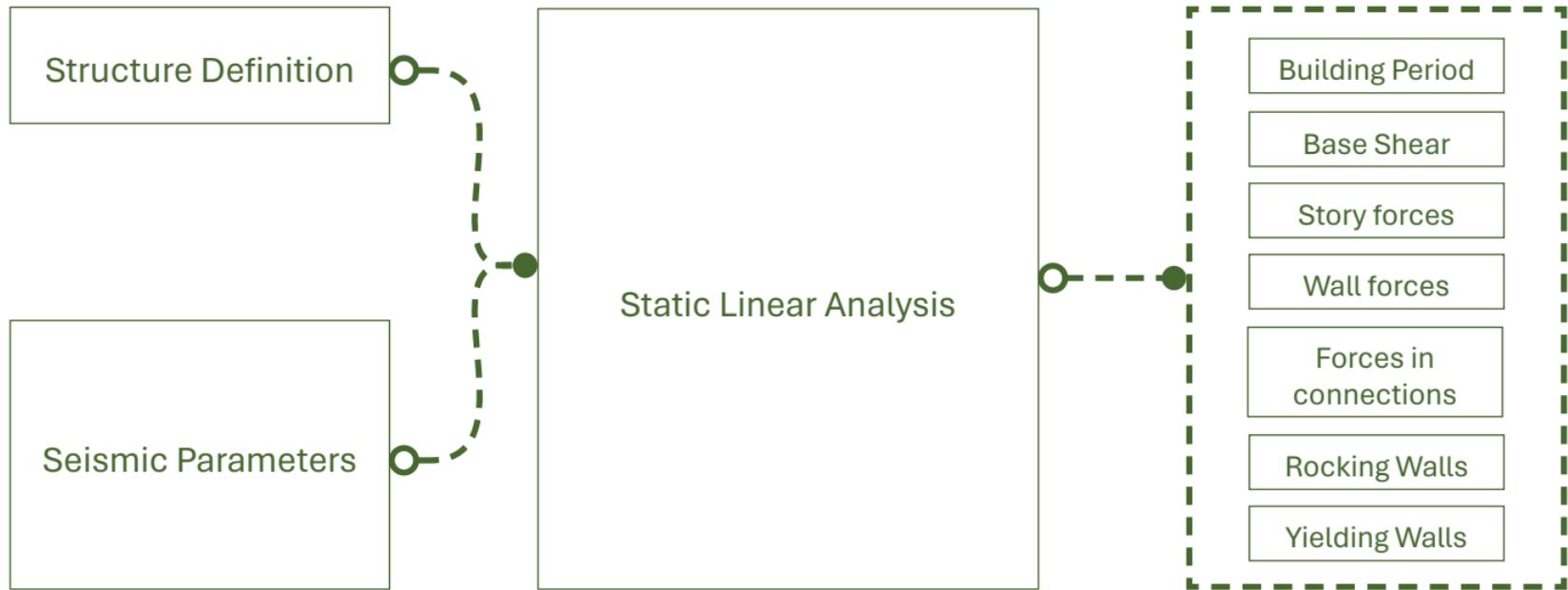
# Static Linear Analysis



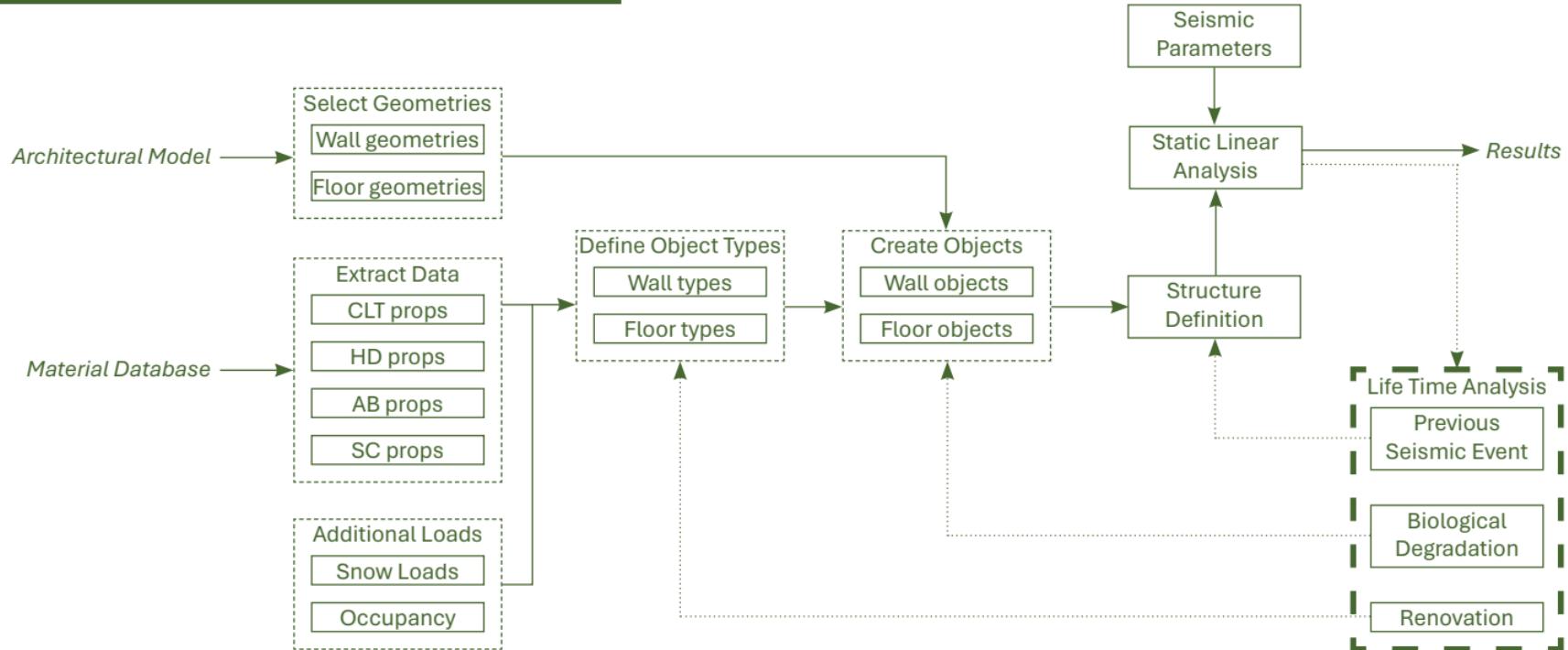
# Static Linear Analysis



# Static Linear Analysis



# The Workflow



# Life Time Analysis



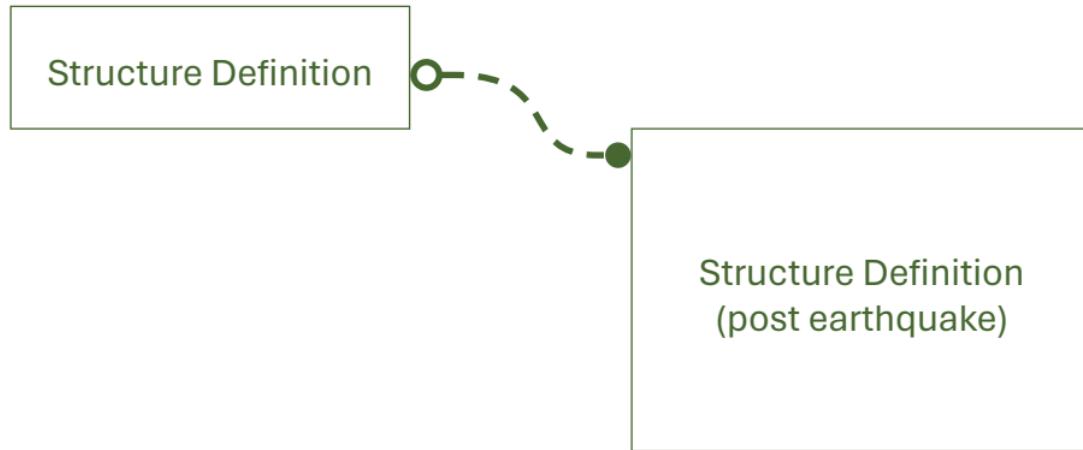
*Previous Seismic Event*

Structure Definition  
(post earthquake)

# Life Time Analysis



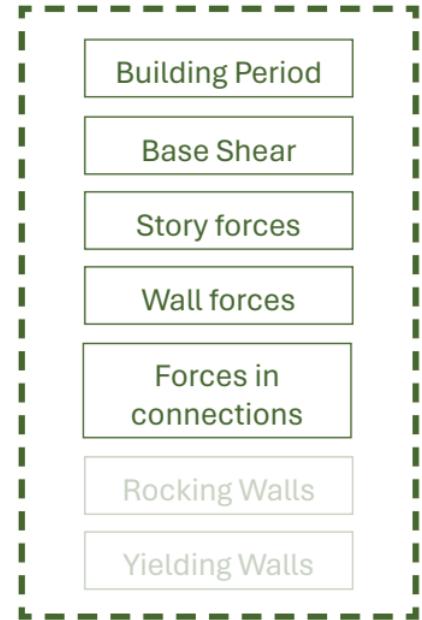
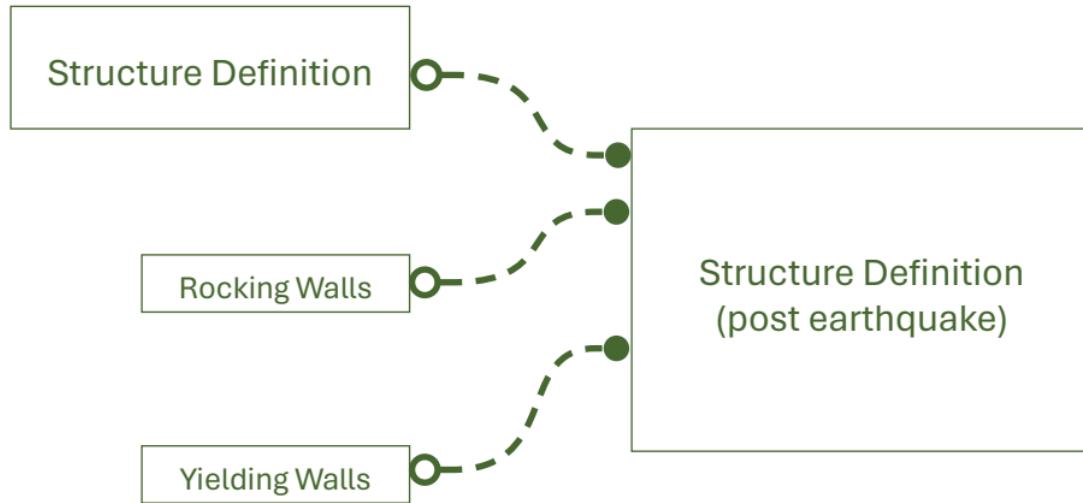
*Previous Seismic Event*



# Life Time Analysis



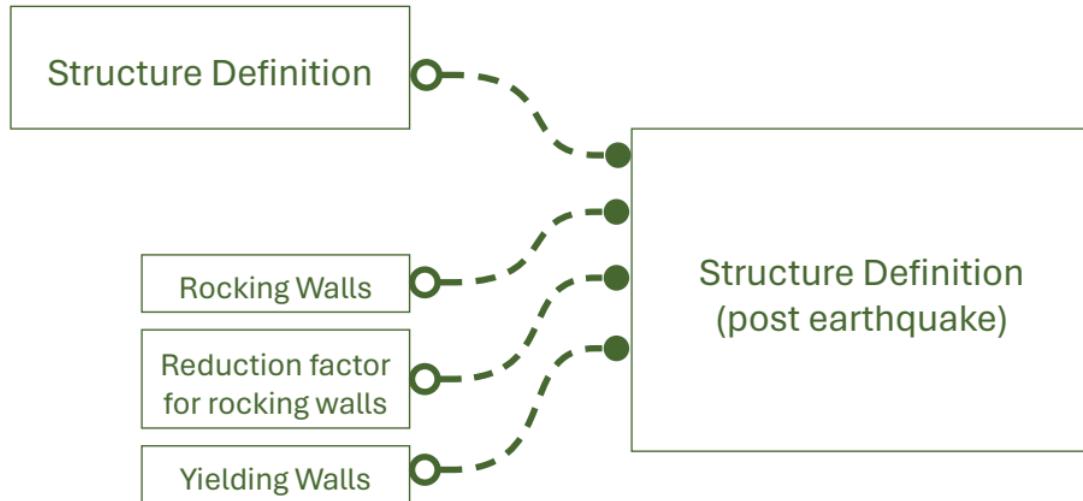
*Previous Seismic Event*



# Life Time Analysis



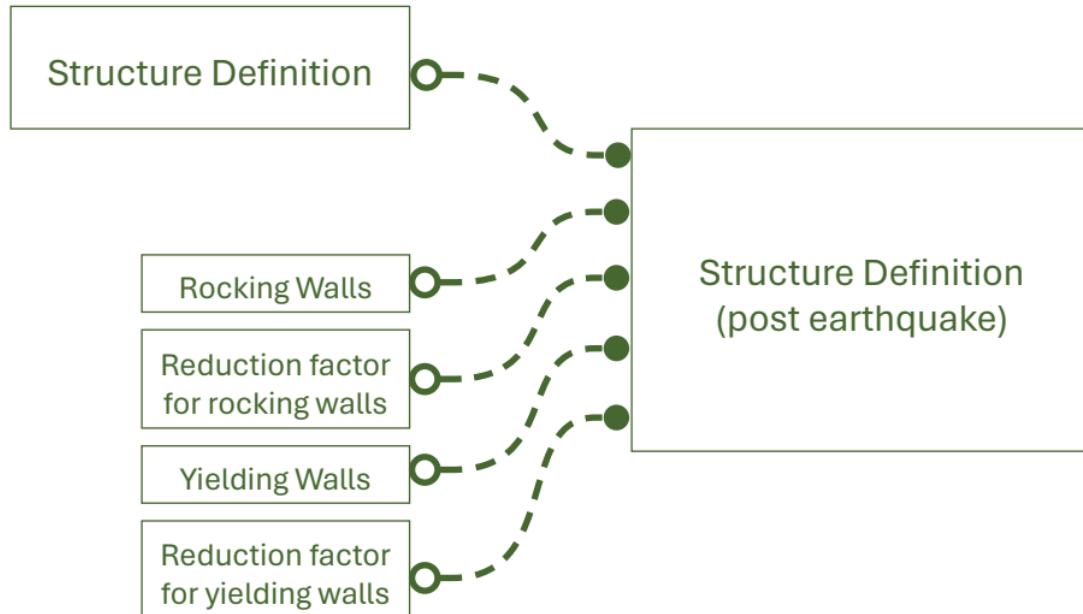
*Previous Seismic Event*



# Life Time Analysis



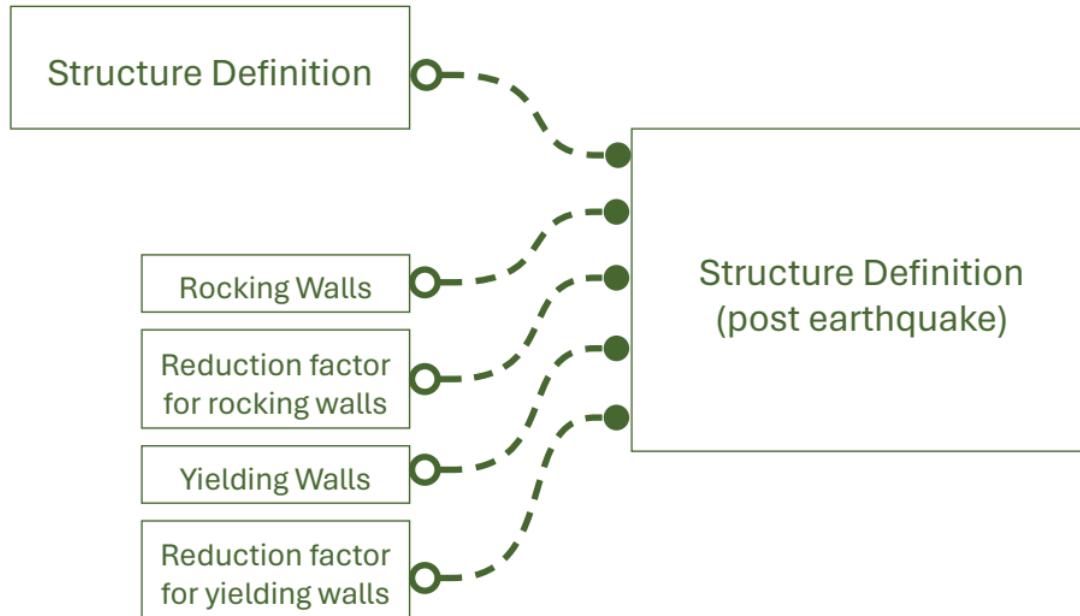
## Previous Seismic Event



# Life Time Analysis



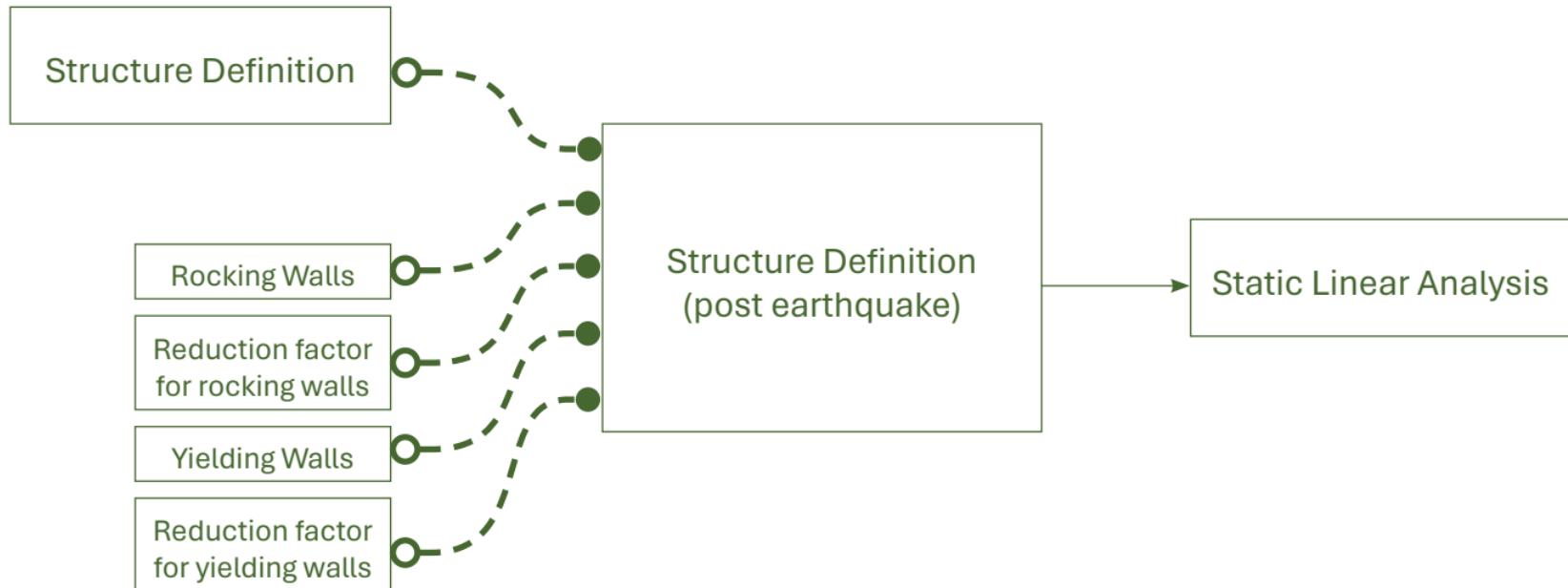
## Previous Seismic Event



# Life Time Analysis



*Previous Seismic Event*



# Life Time Analysis



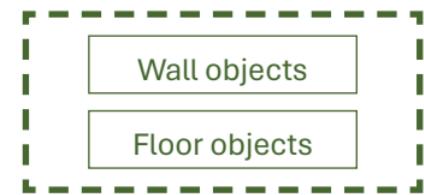
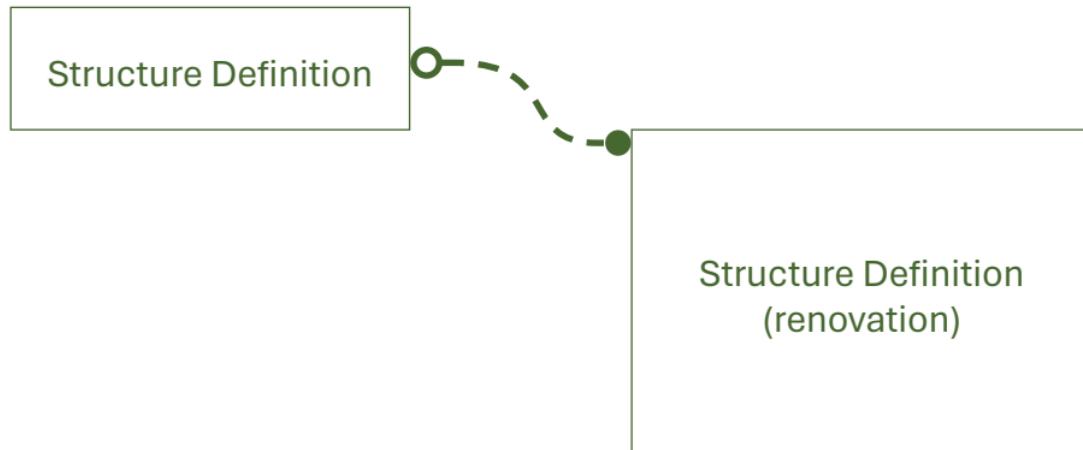
*Renovation*

Structure Definition  
(renovation)

# Life Time Analysis



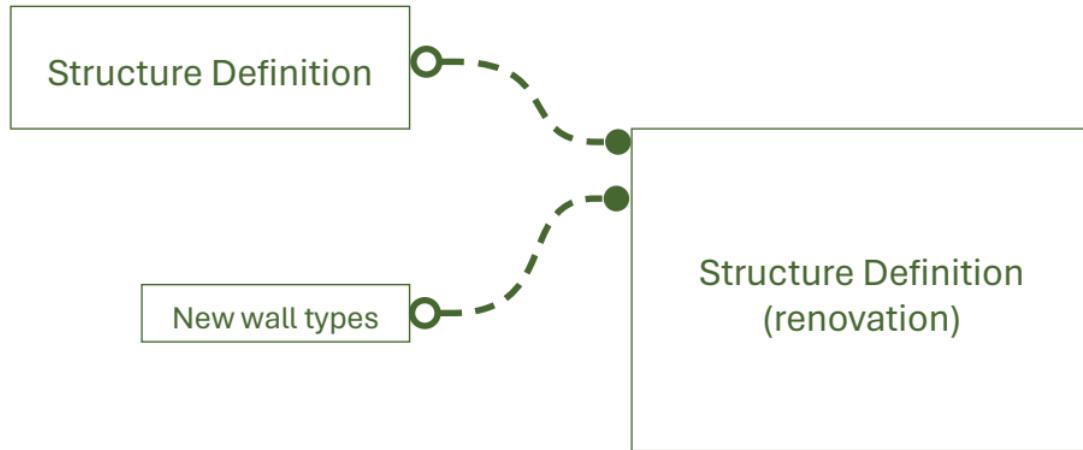
*Renovation*



# Life Time Analysis



*Renovation*

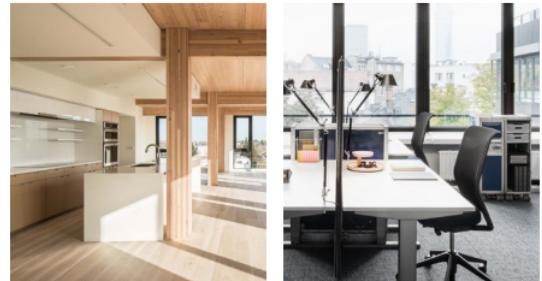
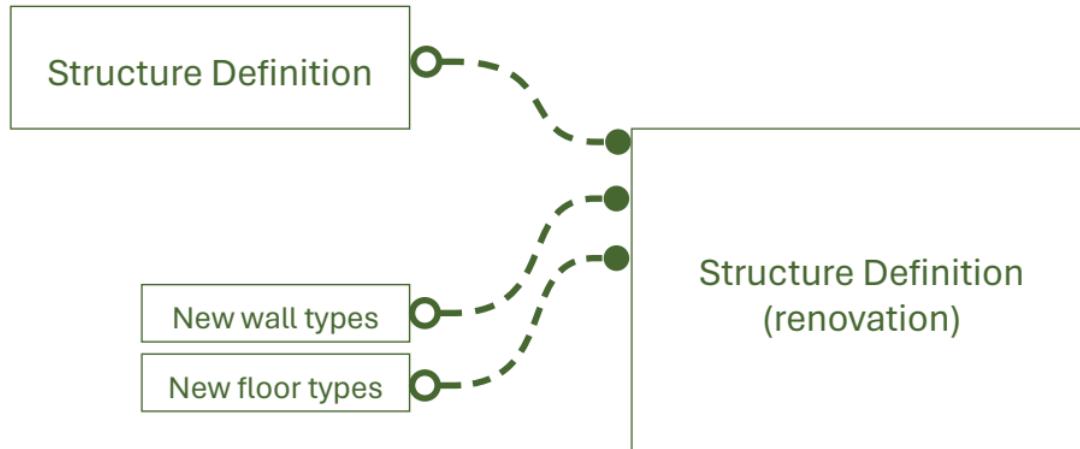


*Facade renovation*

# Life Time Analysis



*Renovation*

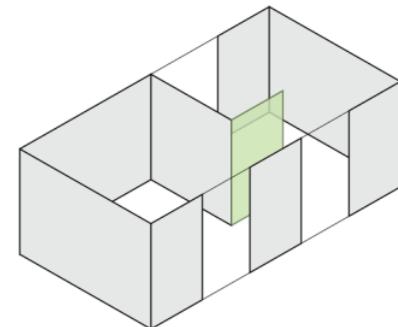
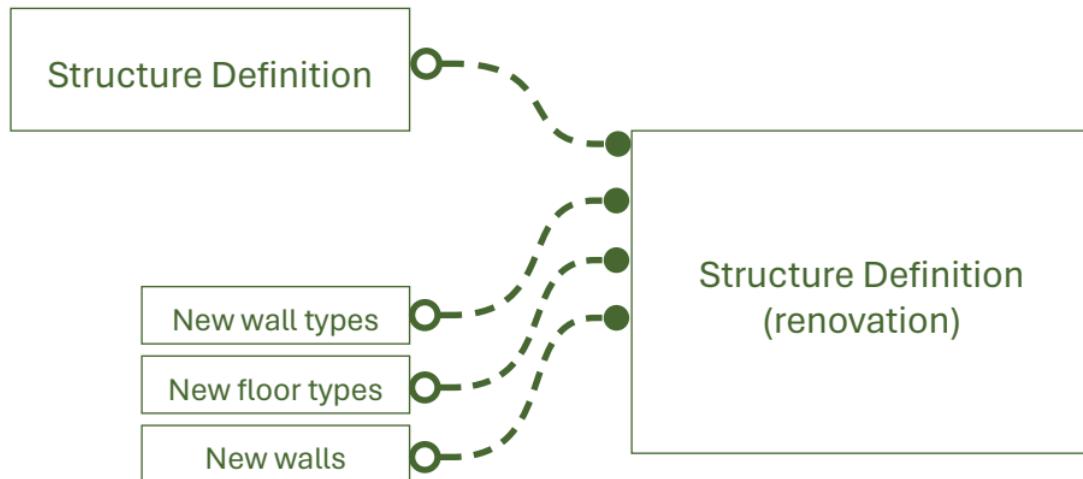


*Change in occupancy*

# Life Time Analysis



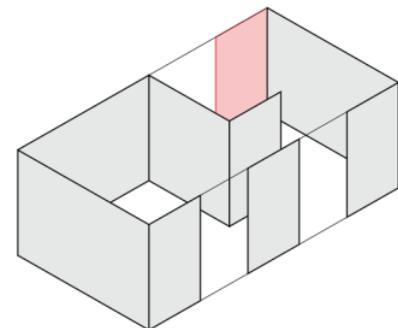
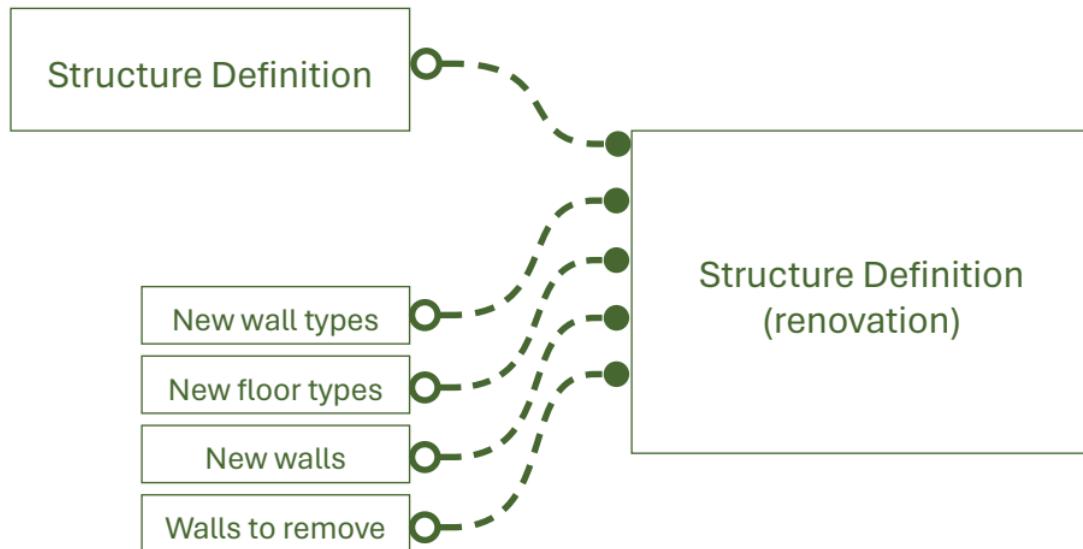
*Renovation*



# Life Time Analysis



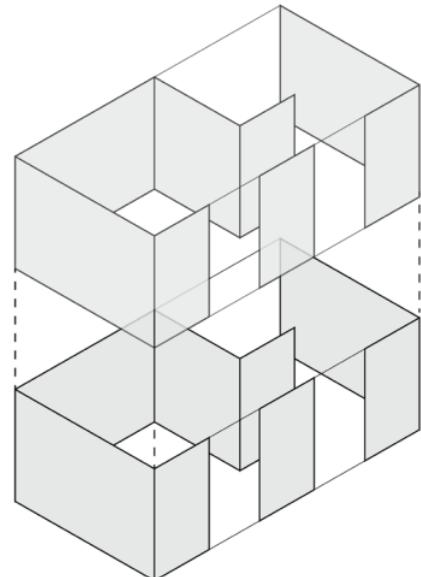
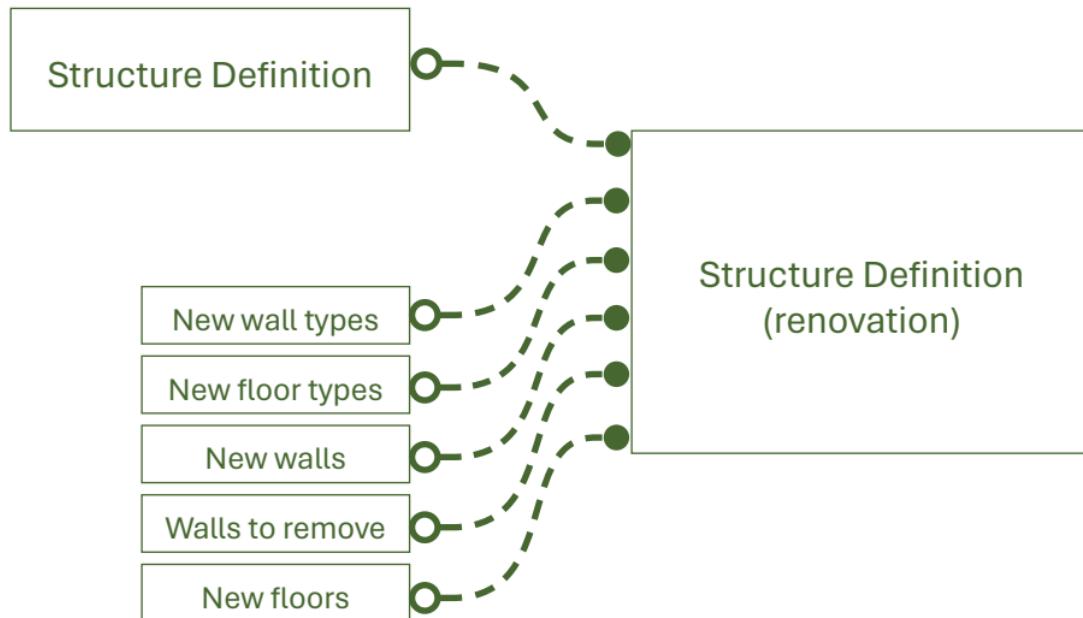
*Renovation*



# Life Time Analysis



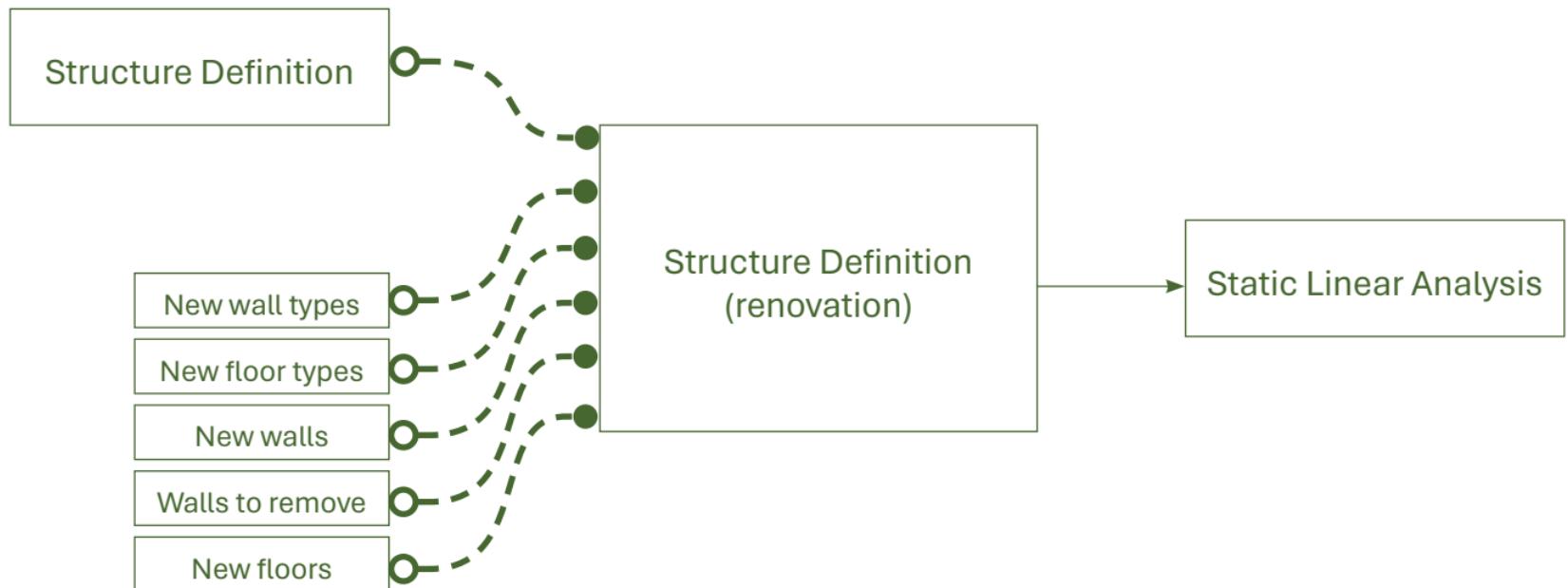
*Renovation*



# Life Time Analysis



*Renovation*



# Life Time Analysis

*Biological Degradation - Walls over time*

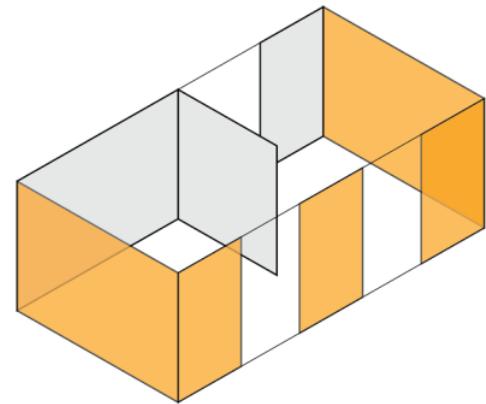


Biological Degradation  
of walls over time

# Life Time Analysis



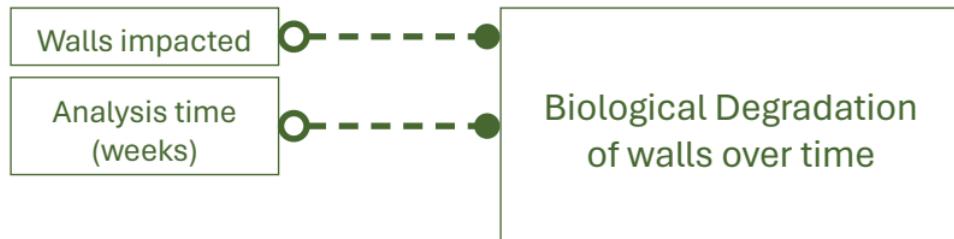
*Biological Degradation - Walls over time*



# Life Time Analysis



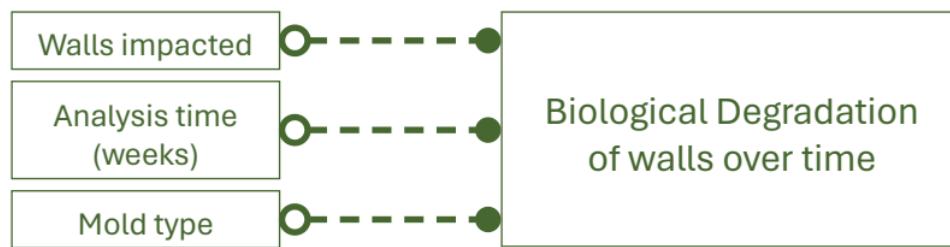
*Biological Degradation - Walls over time*



# Life Time Analysis



*Biological Degradation - Walls over time*



*Gloeophyllum trabeum*

$$F_{dbs_{DF}} = \sqrt{-16(time) + 1891.2}$$

$$F_{dbs_{NS}} = -0.31(time) + 32.3$$

$$F_{dbs_{SPF}} = -0.26(time) + 37.1$$

*Rhodonia placenta*

$$F_{dbs_{DF}} = -0.58(time) + 45.2$$

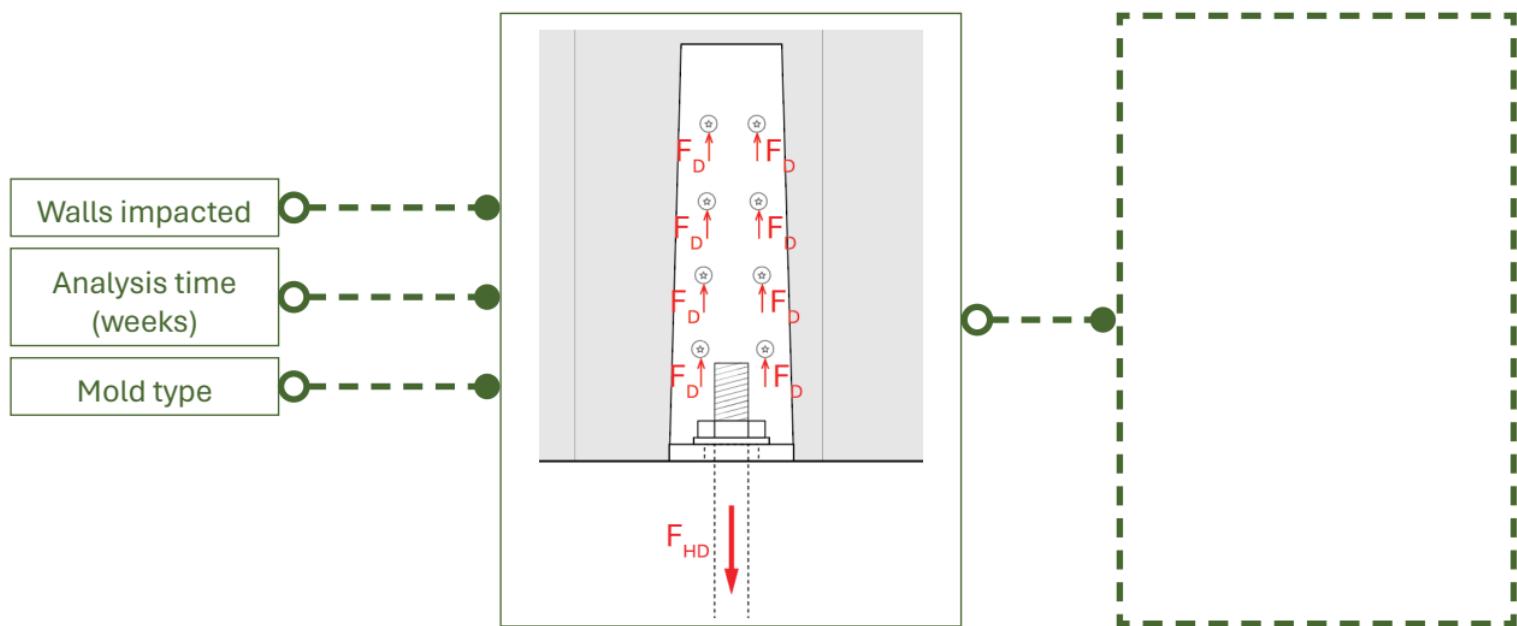
$$F_{dbs_{NS}} = -0.23(time) + 32.7$$

$$F_{dbs_{SPF}} = -0.32(time) + 33.6$$

# Life Time Analysis



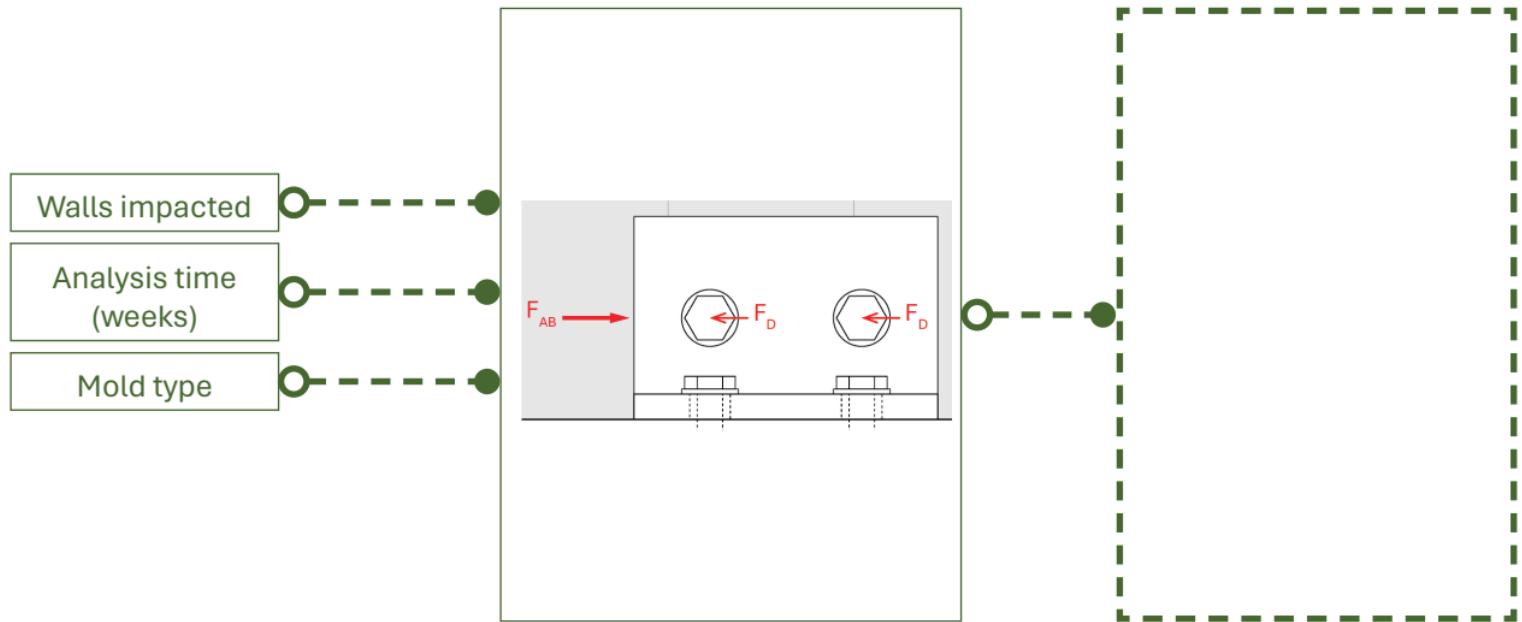
## Biological Degradation - Walls over time



# Life Time Analysis



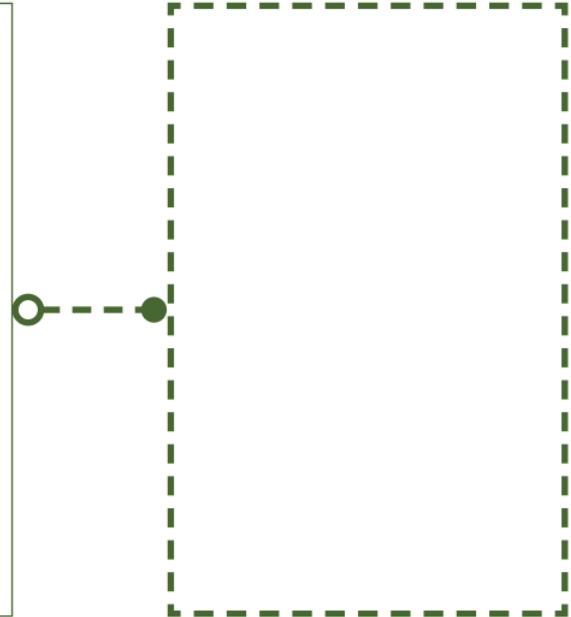
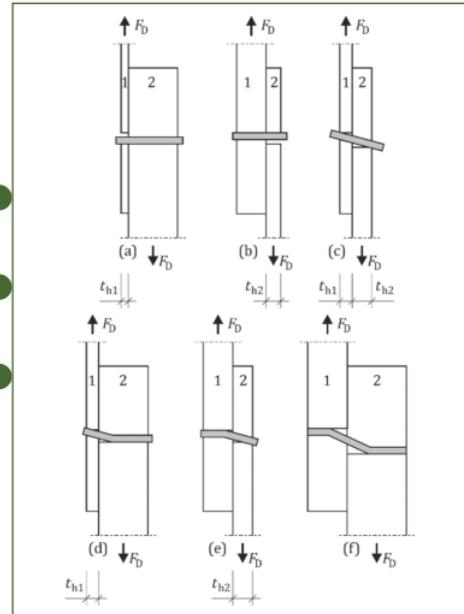
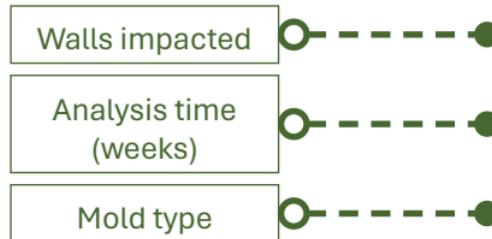
*Biological Degradation - Walls over time*



# Life Time Analysis



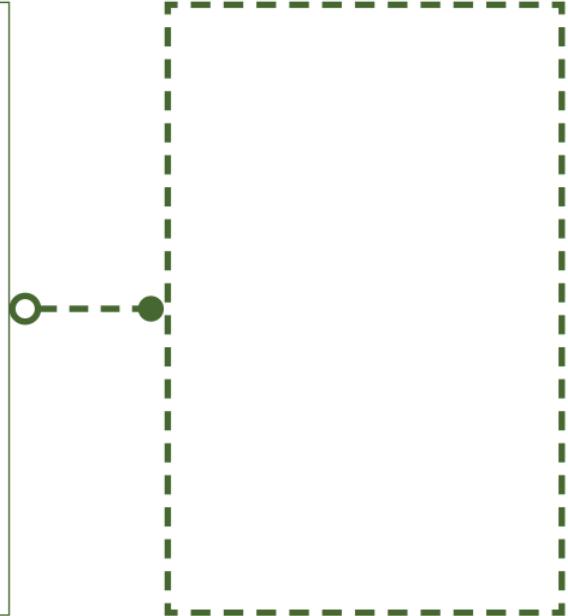
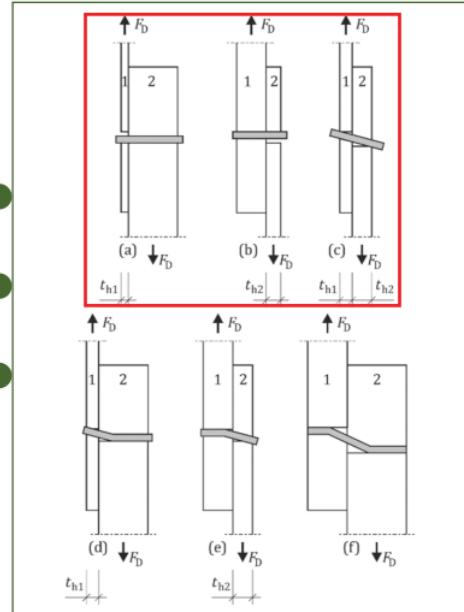
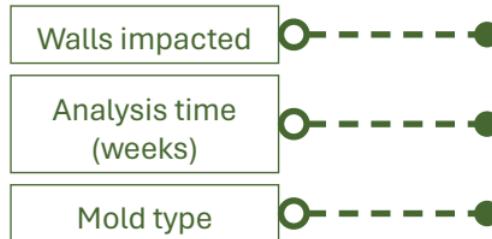
*Biological Degradation - Walls over time*



# Life Time Analysis



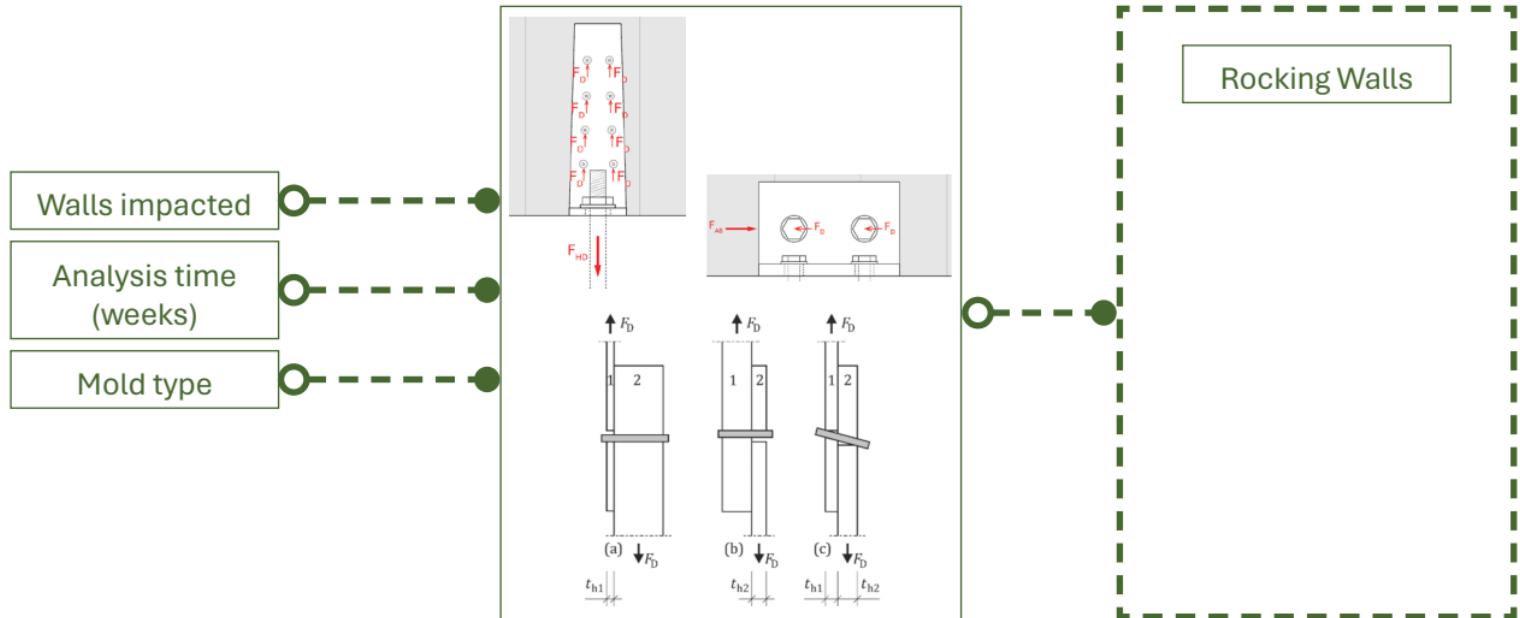
*Biological Degradation - Walls over time*



# Life Time Analysis



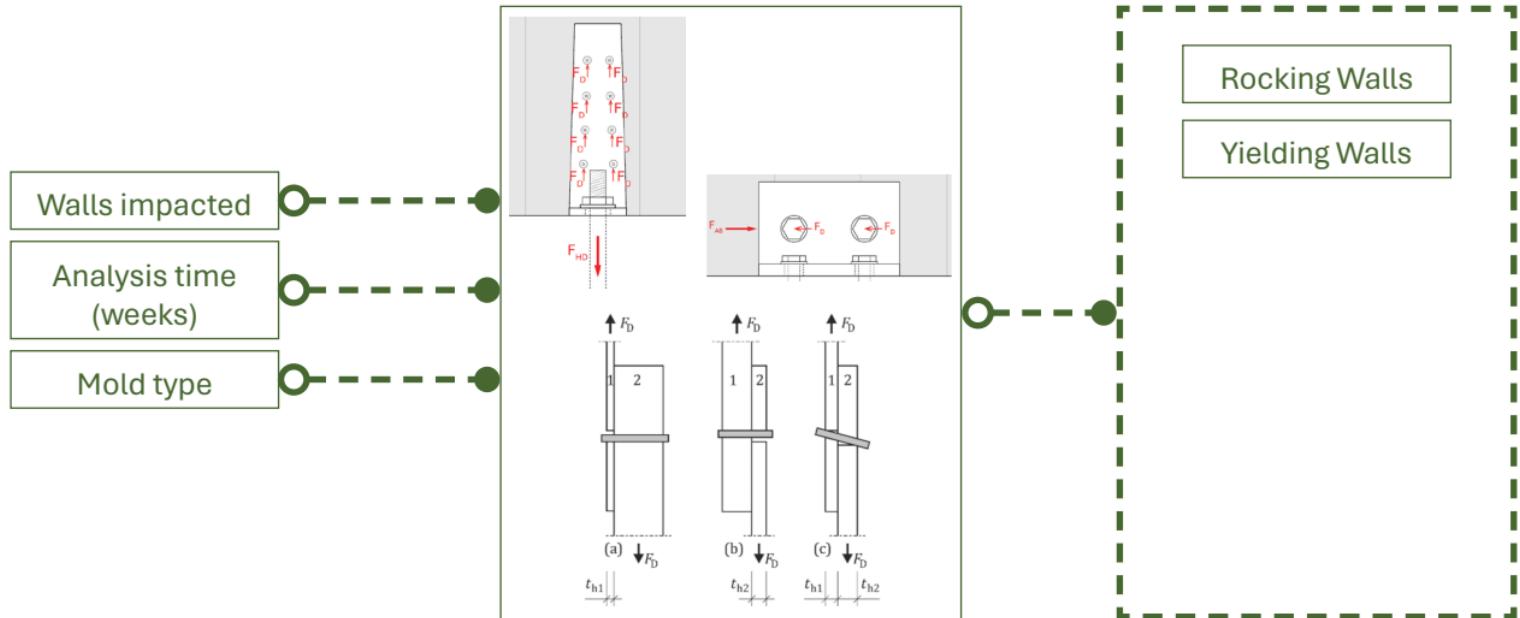
Biological Degradation - Walls over time



# Life Time Analysis



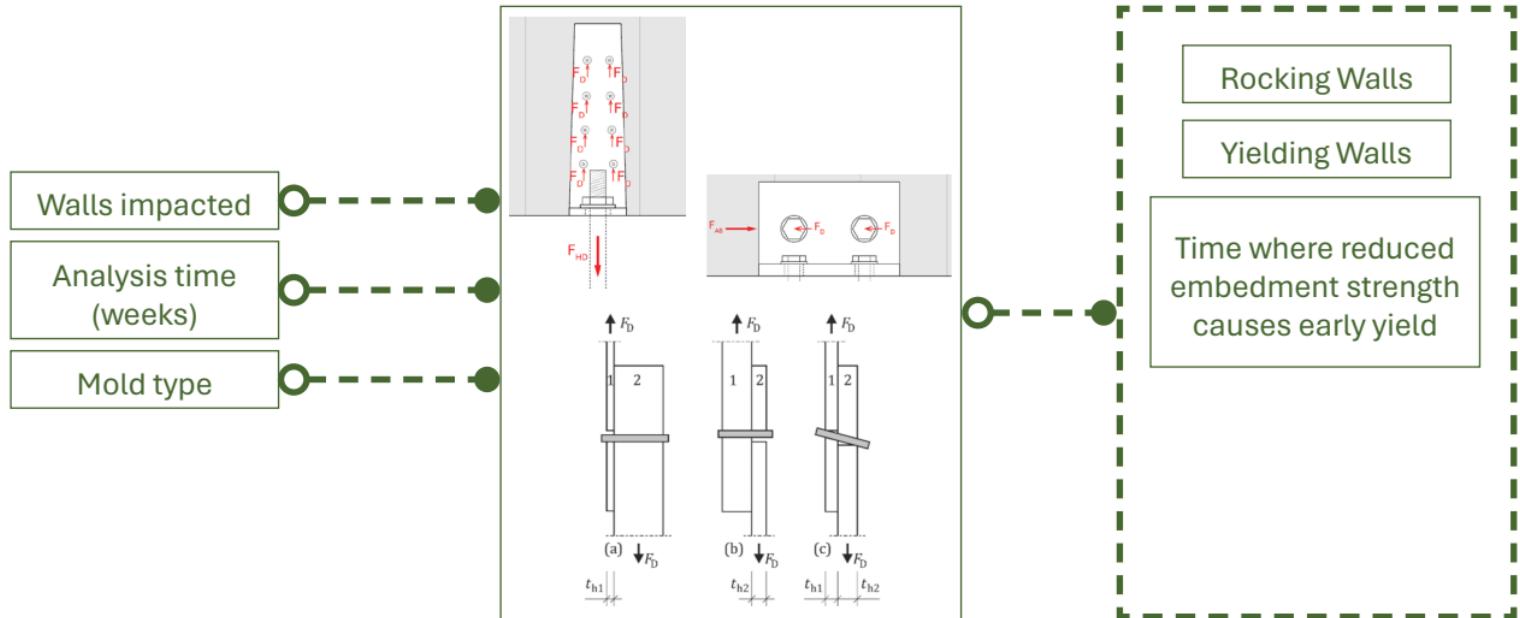
Biological Degradation - Walls over time



# Life Time Analysis



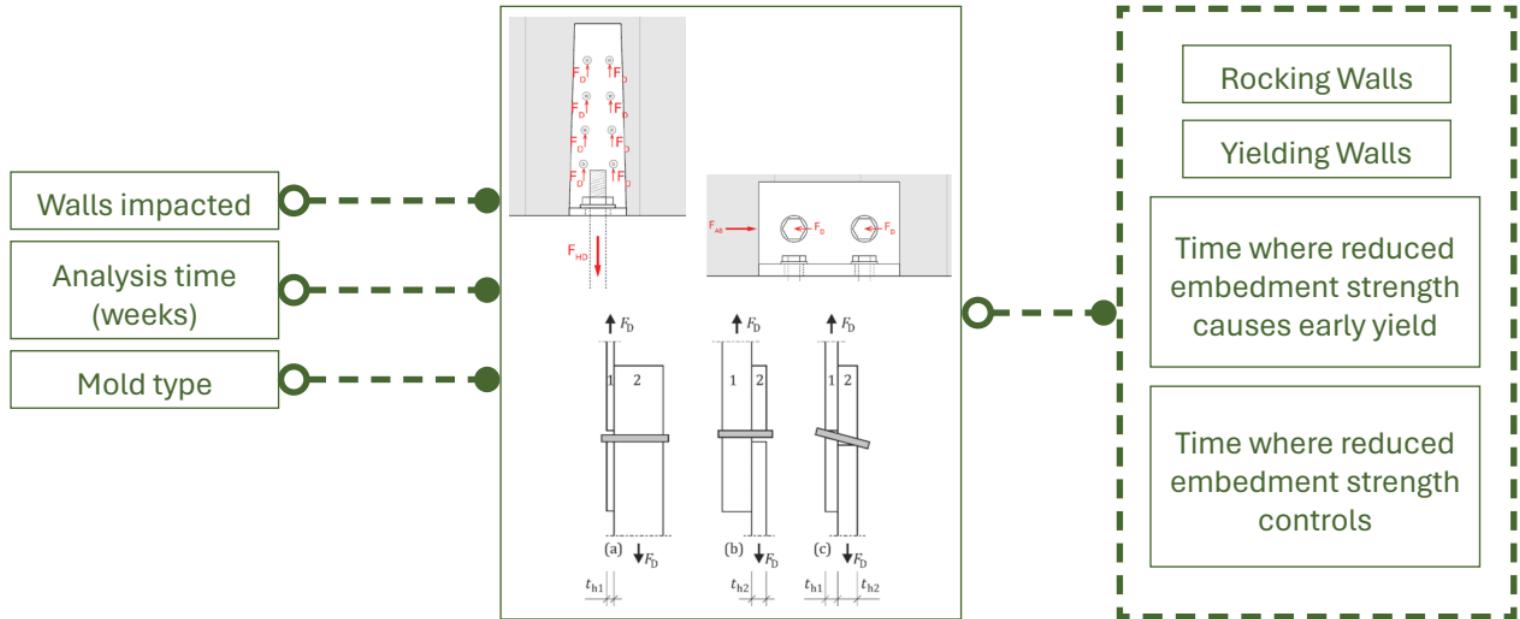
Biological Degradation - Walls over time



# Life Time Analysis



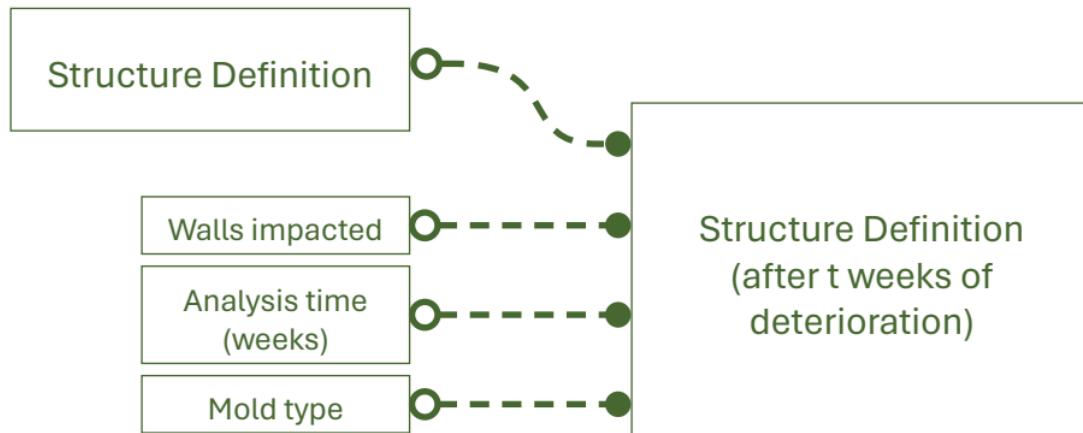
Biological Degradation - Walls over time



# Life Time Analysis



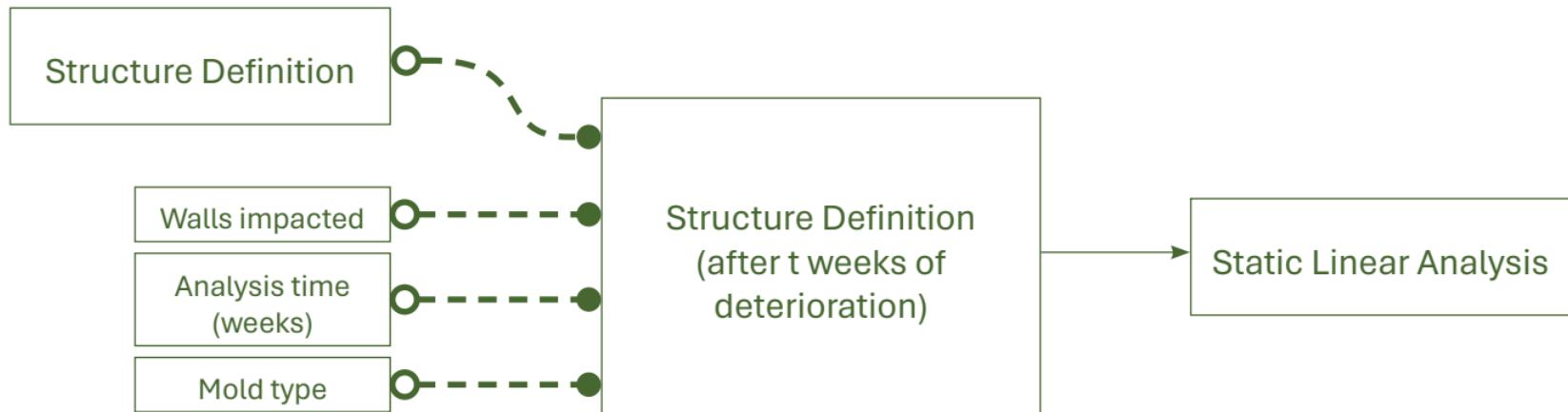
*Biological Degradation - Structure at time t*



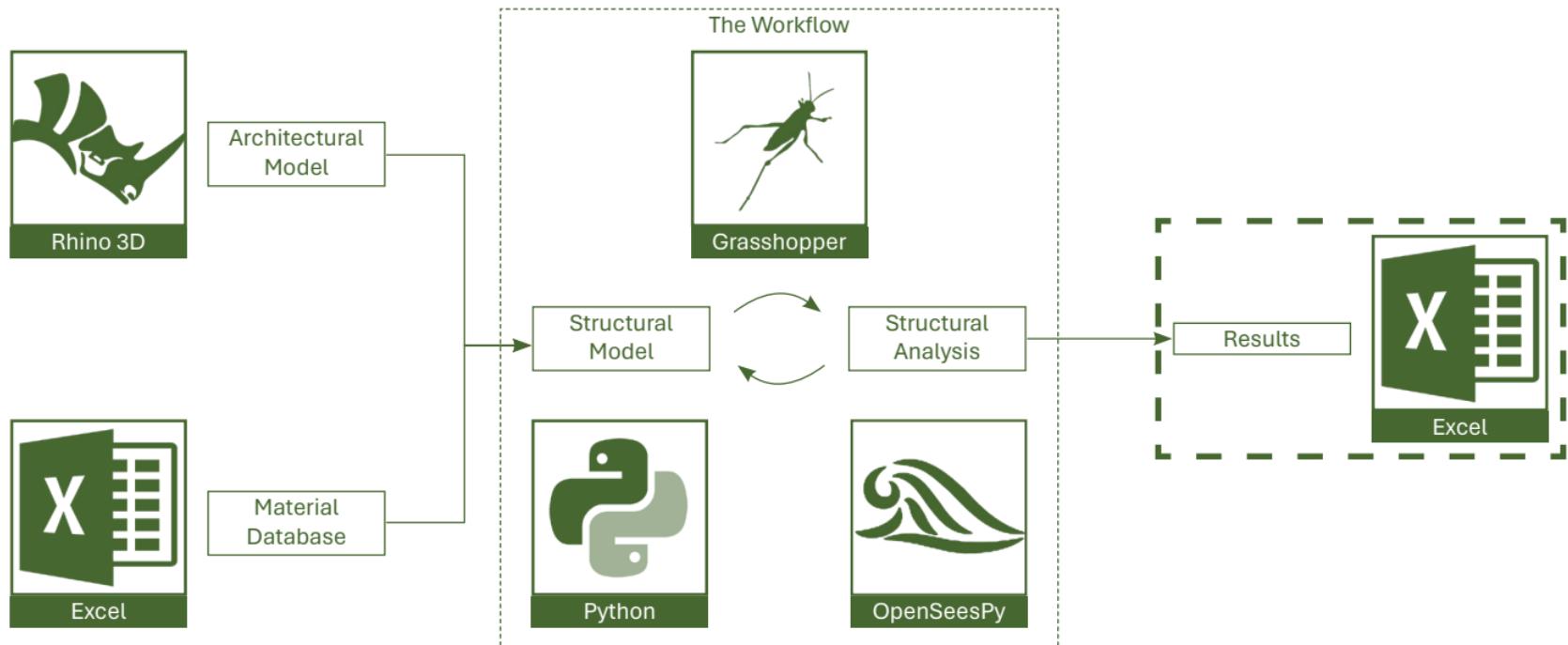
# Life Time Analysis



*Biological Degradation - Structure at time t*



# Programs and Workflow



# Results



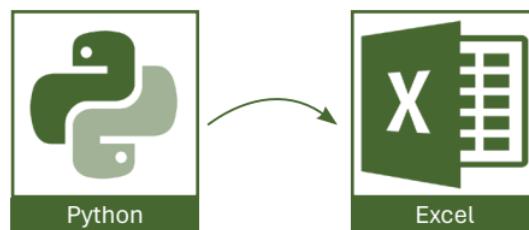
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# Results



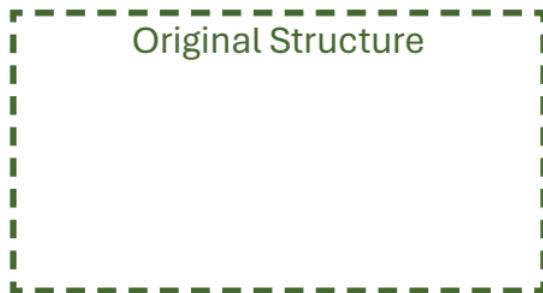
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# Results



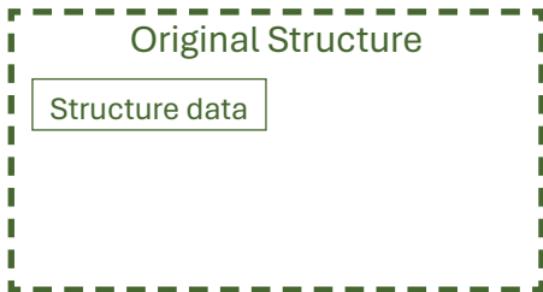
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# Results



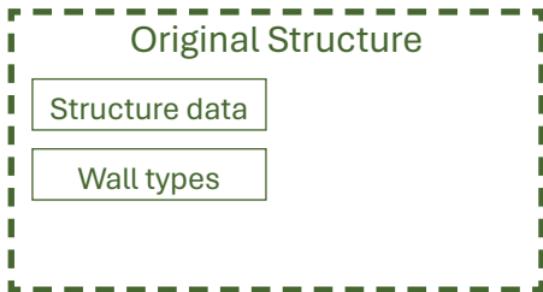
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# Results



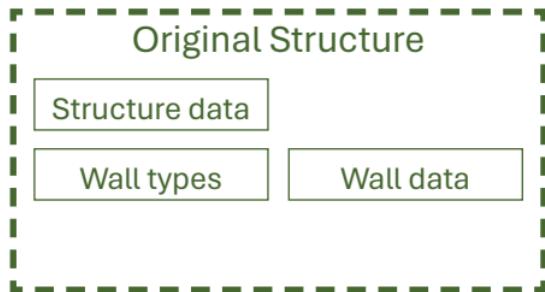
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# Results



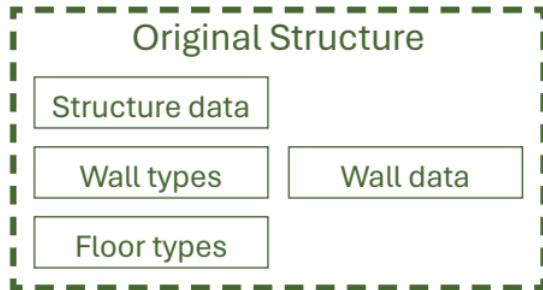
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# Results



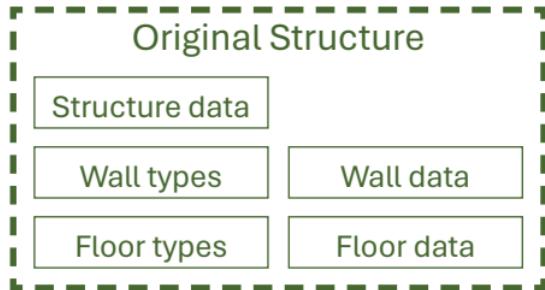
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# Results



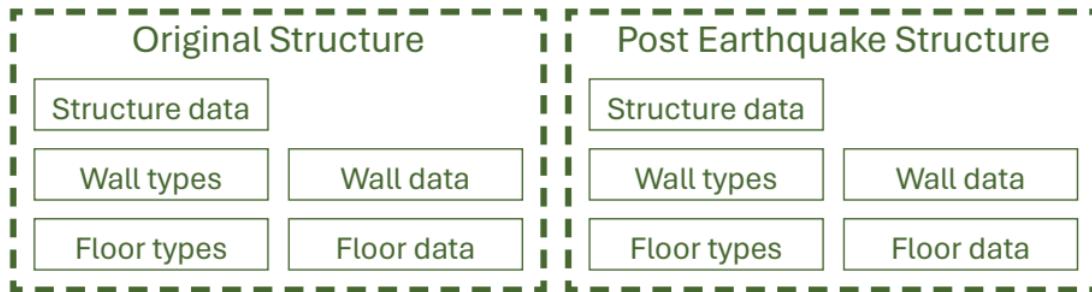
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# Results



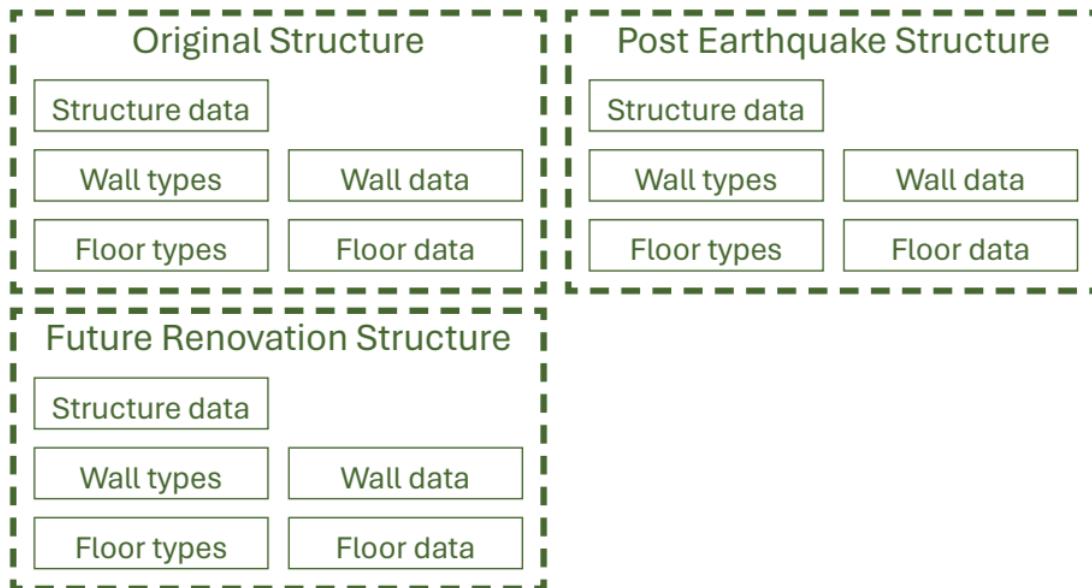
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# Results



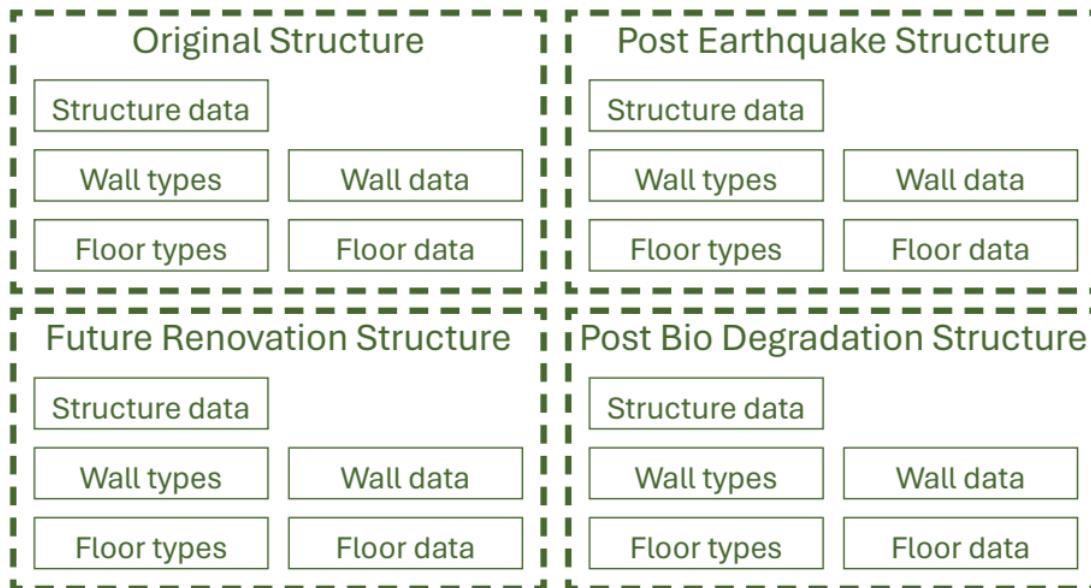
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# Results



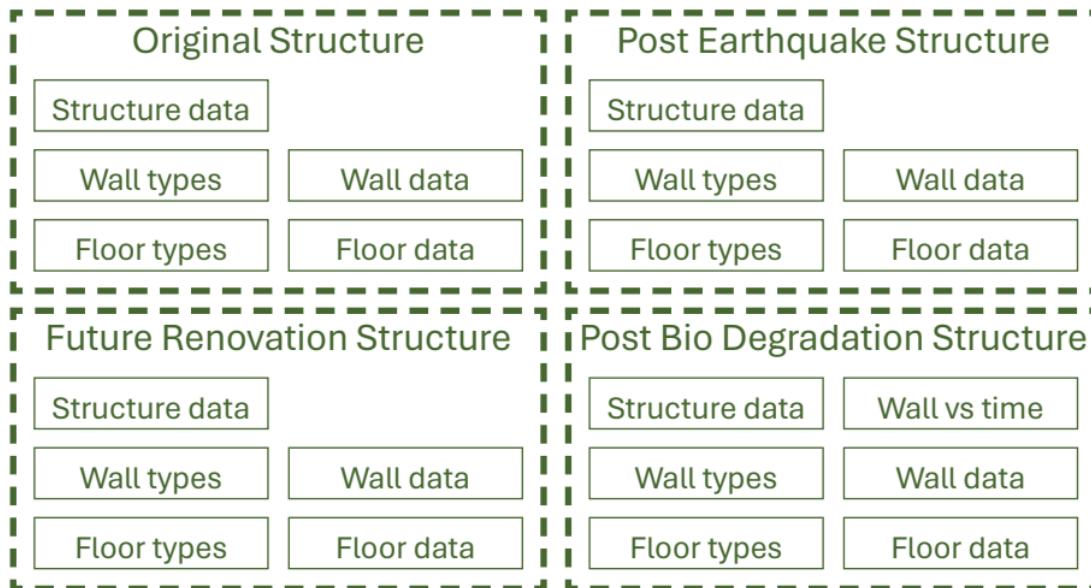
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# Results



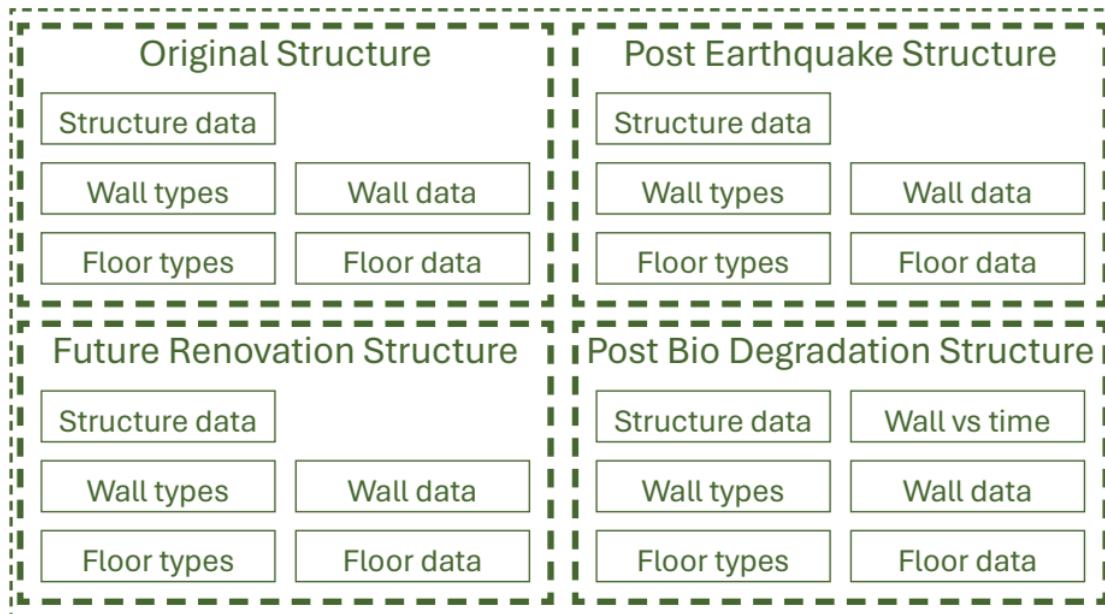
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# Results



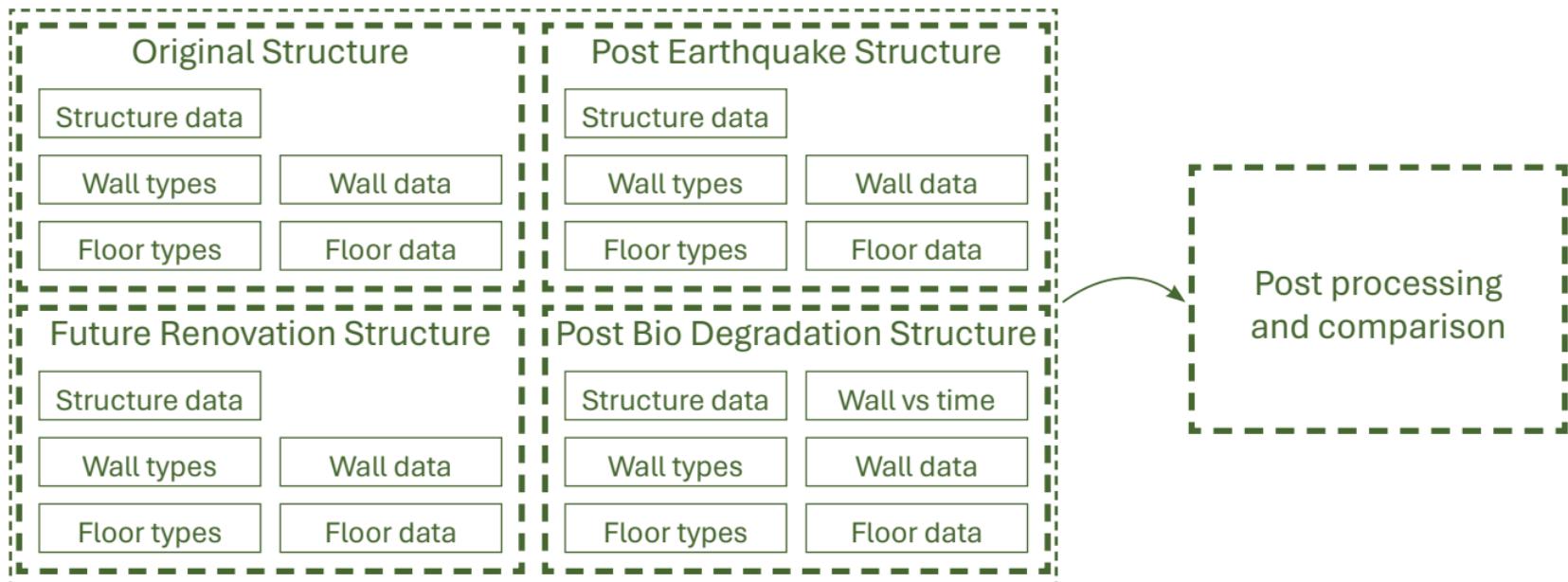
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# Results



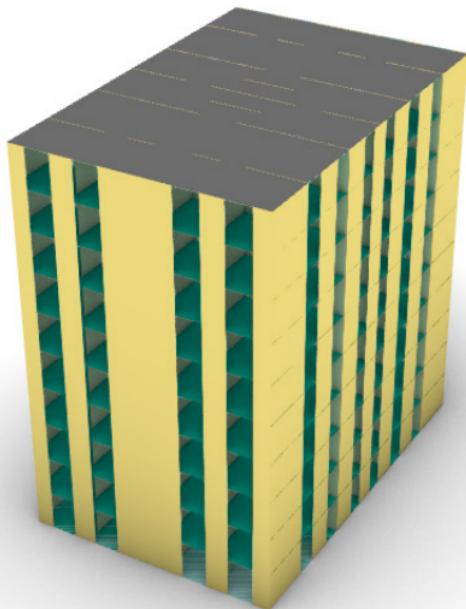
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# Case Study

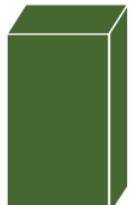


# Case Study Parameters

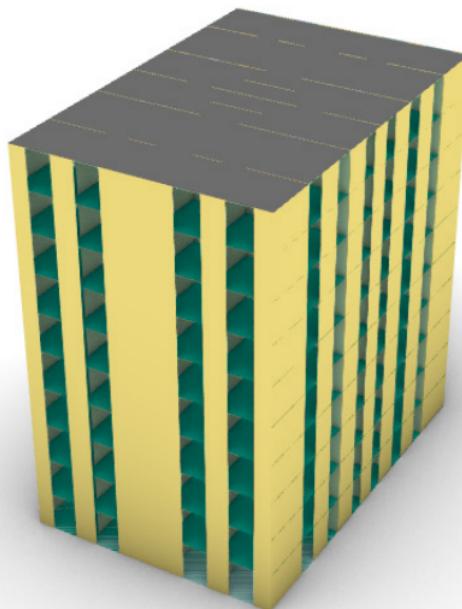




# Case Study Parameters

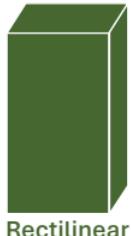


Rectilinear





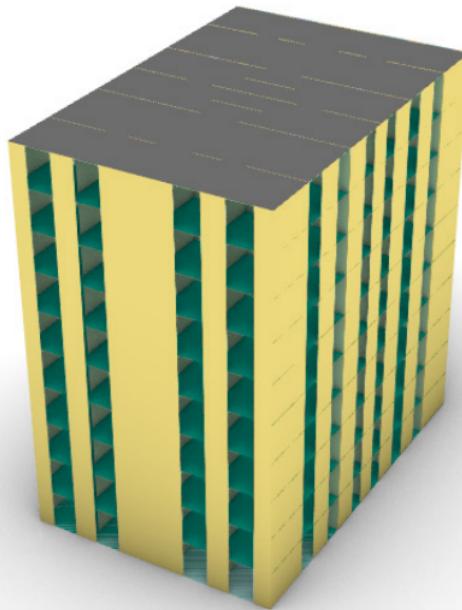
# Case Study Parameters



Rectilinear



Residential





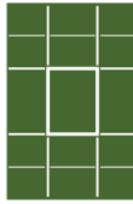
# Case Study Parameters



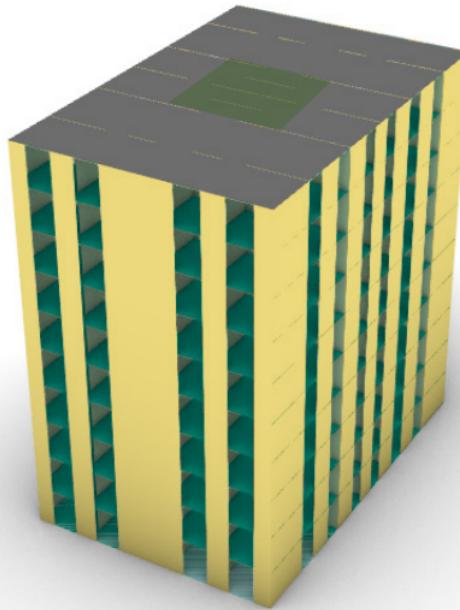
Rectilinear



Residential

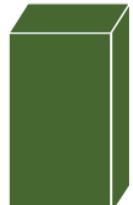


Central Core





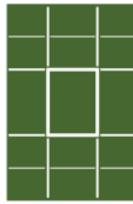
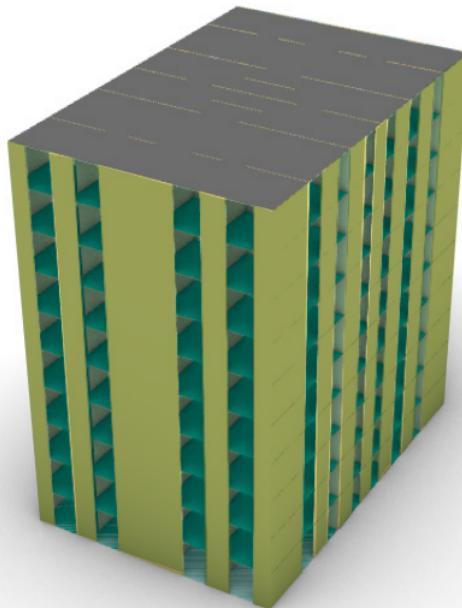
# Case Study Parameters



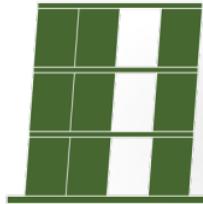
Rectilinear



Residential



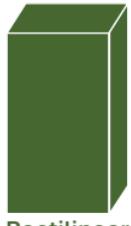
Central Core



Shear Walls



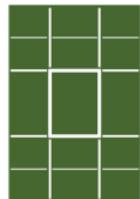
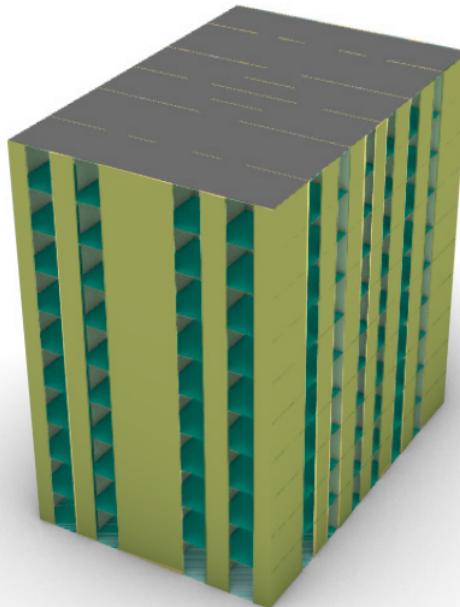
# Case Study Parameters



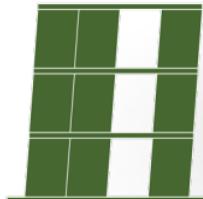
Rectilinear



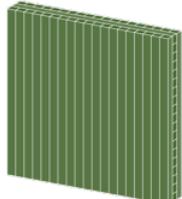
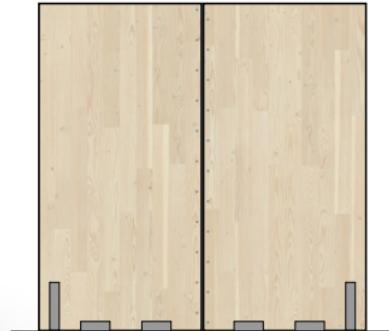
Residential



Central Core



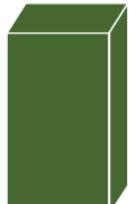
Shear Walls



CLT Walls



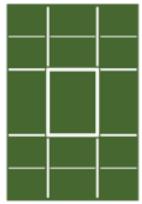
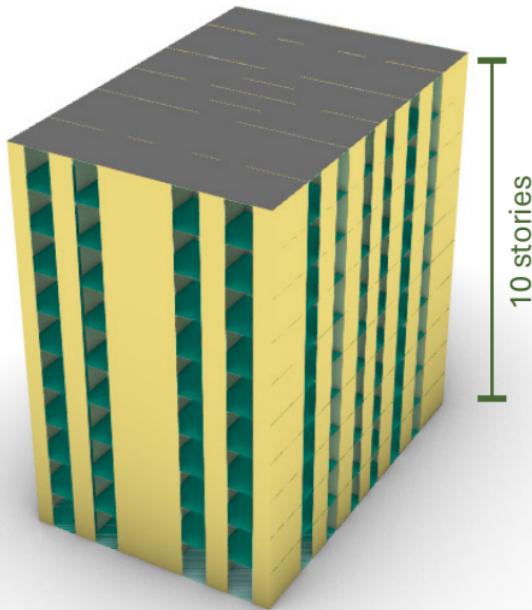
# Case Study Parameters



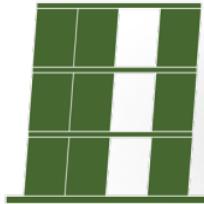
Rectilinear



Residential



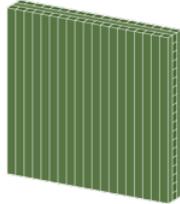
Central Core



Shear Walls



8+ stories



CLT Walls



# Case Study Parameters



Europe Region



# Case Study Parameters



Europe Region

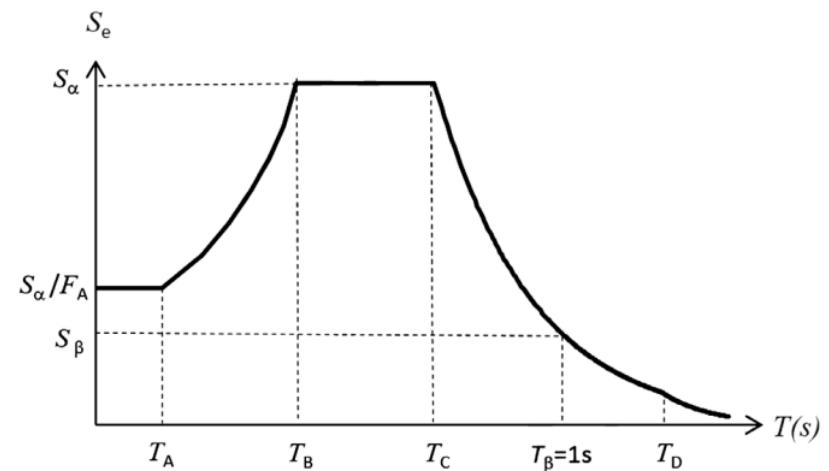


# Case Study Parameters





# Case Study Parameters

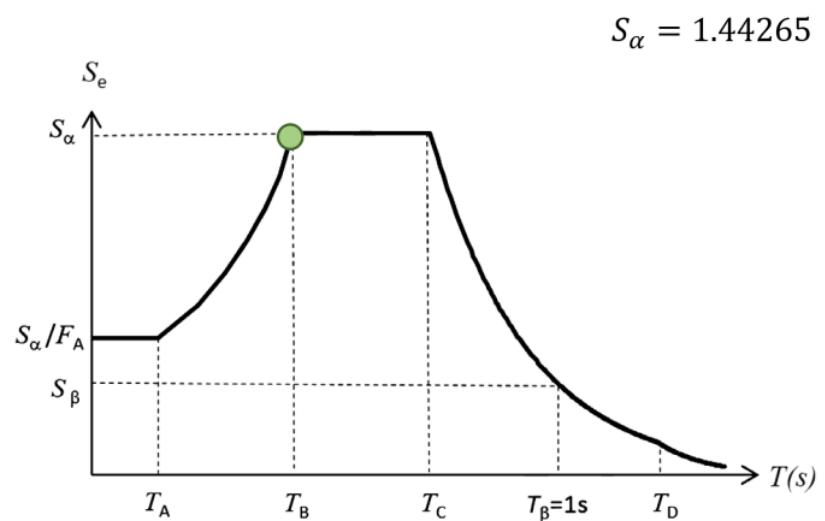




# Case Study Parameters



Europe Region

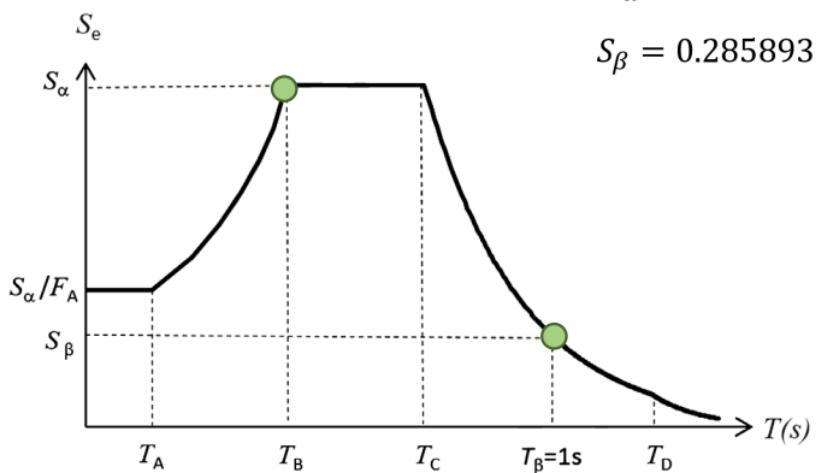




# Case Study Parameters



Europe Region

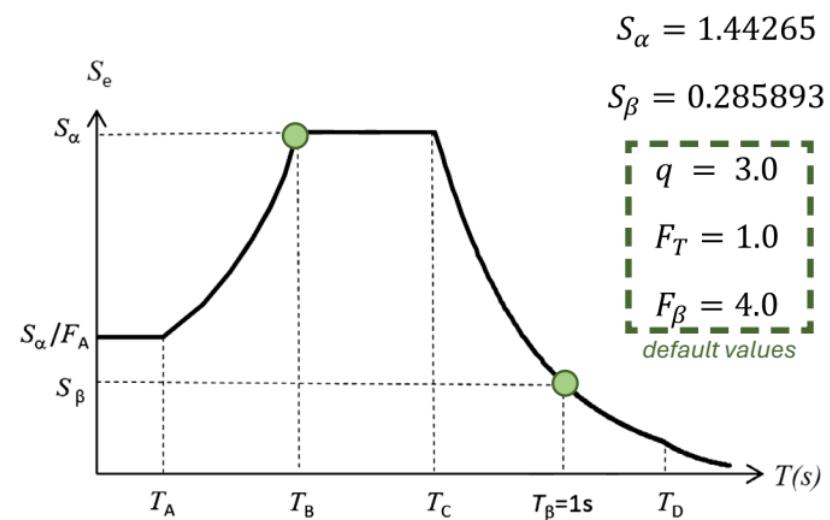




# Case Study Parameters

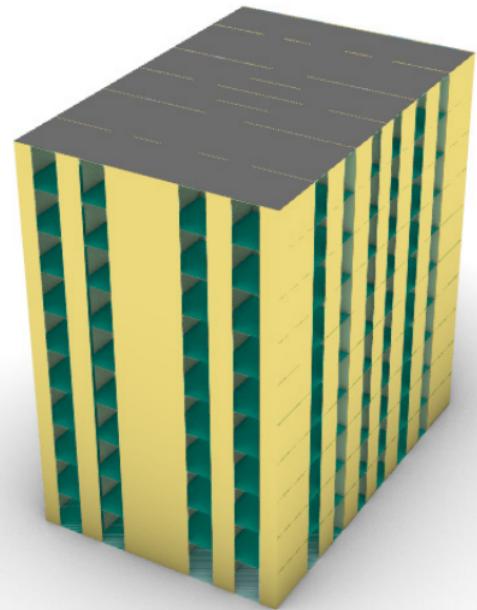


Europe Region



# Original Structure

Structure Parameters

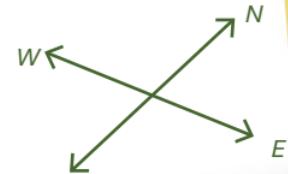


# Original Structure

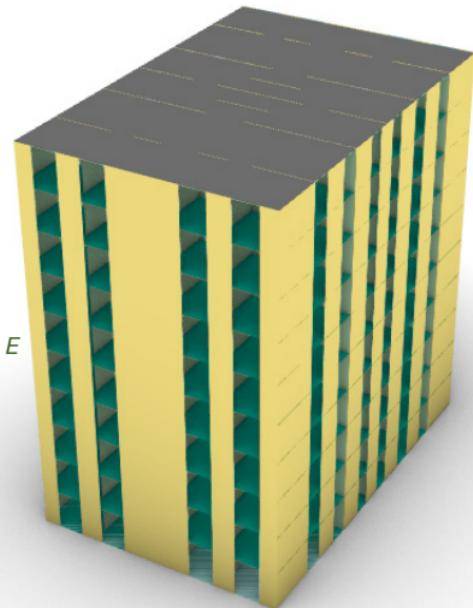
## Structure Parameters

- Building period

$$T_{EW} = 0.0673 \text{ sec}$$



$$T_{NS} = 0.0829 \text{ sec}$$



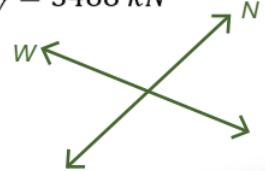
# Original Structure

## Structure Parameters

- Building period
- Base Shear

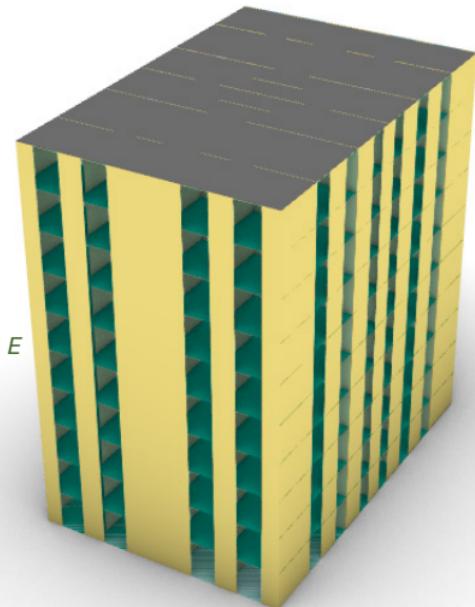
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3488 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$

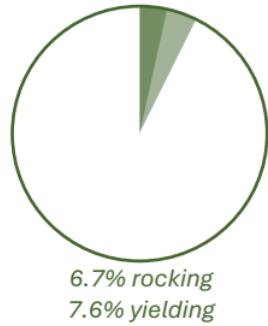


# Original Structure

## Structure Parameters

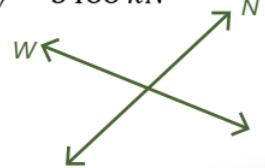
- Building period
- Base Shear

## Rocking and Yielding Walls



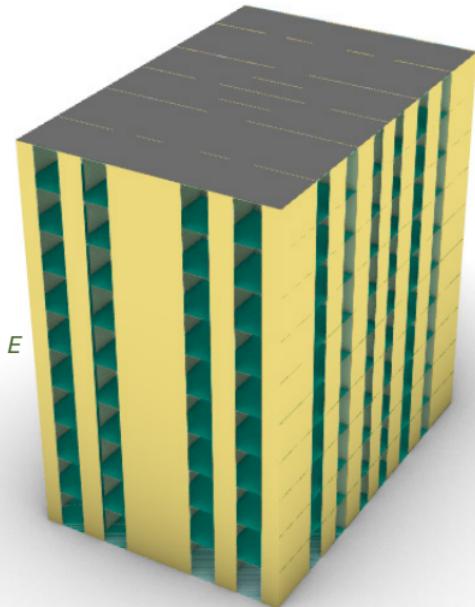
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3488 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



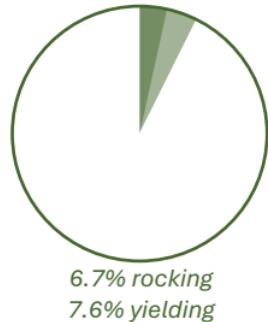
# Original Structure

## Structure Parameters

- Building period
- Base Shear

## Rocking and Yielding Walls

- Only on top two stories
  - Not enough weight to resist overturning moment

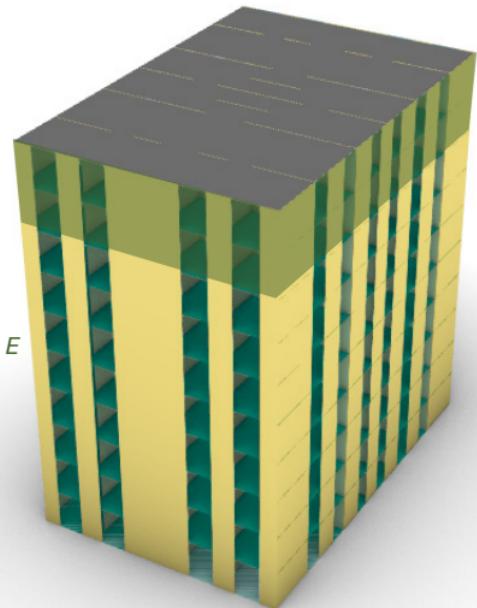
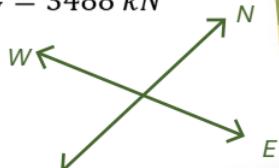


$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3488 \text{ kN}$$

$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



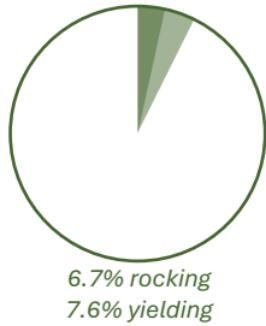
# Original Structure

## Structure Parameters

- Building period
- Base Shear

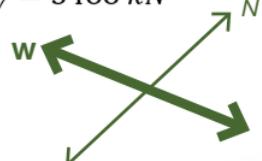
## Rocking and Yielding Walls

- Only on top two stories
  - Not enough weight to resist overturning moment
- East-west direction only



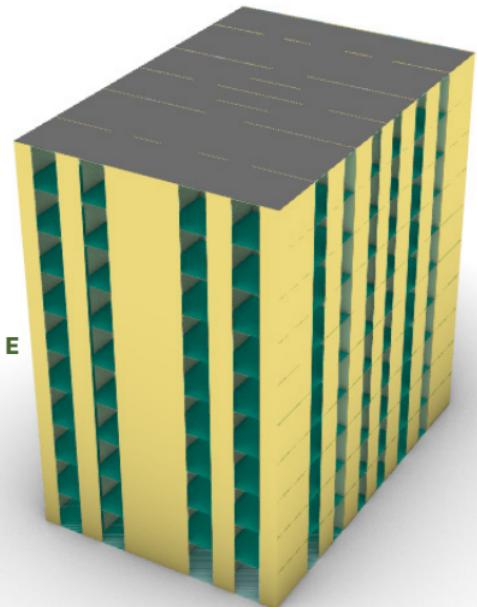
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3488 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



# Original Structure

## Structure Parameters

- Building period
- Base Shear

## Rocking and Yielding Walls

- Only on top two stories
  - Not enough weight to resist overturning moment
- East-west direction only
- Yielding in from shear, not overturning moment



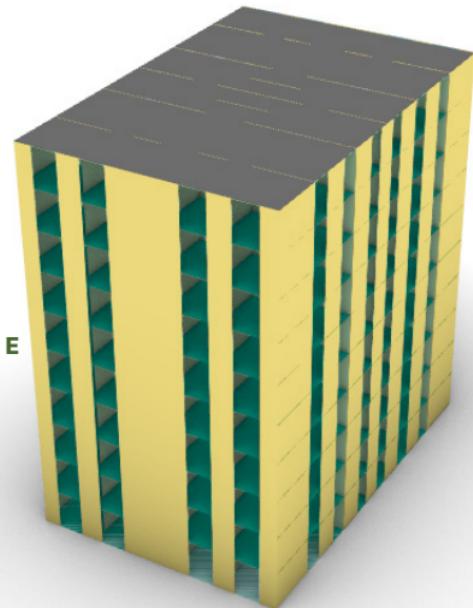
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3488 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

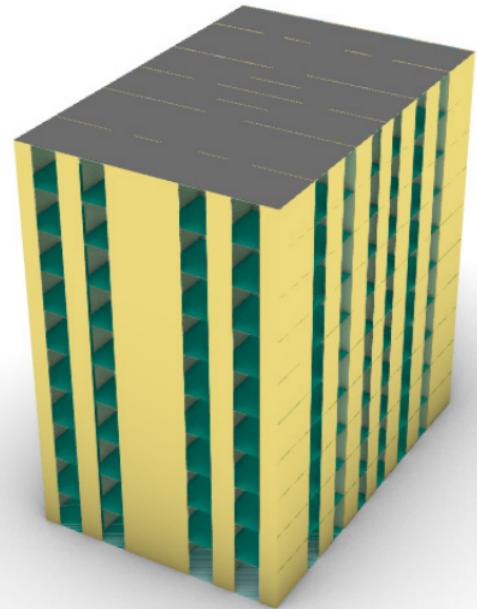
$$V_{NS} = 3513 \text{ kN}$$



# Post Earthquake

## Change

- Stiffness reduction
  - Rocking walls: 80%
  - Yielded walls: 50%



# Post Earthquake

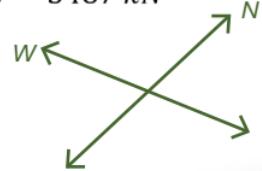
## Change

- Stiffness reduction
  - Rocking walls: 80%
  - Yielded walls: 50%

## Structure Parameters

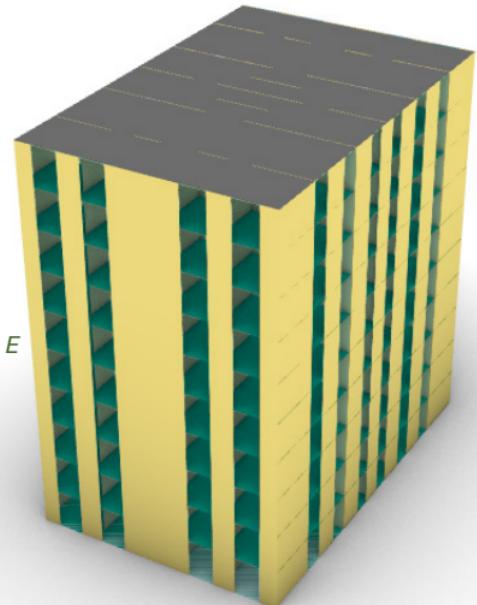
$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



# Post Earthquake

## Change

- Stiffness reduction
  - Rocking walls: 80%
  - Yielded walls: 50%

## Structure Parameters

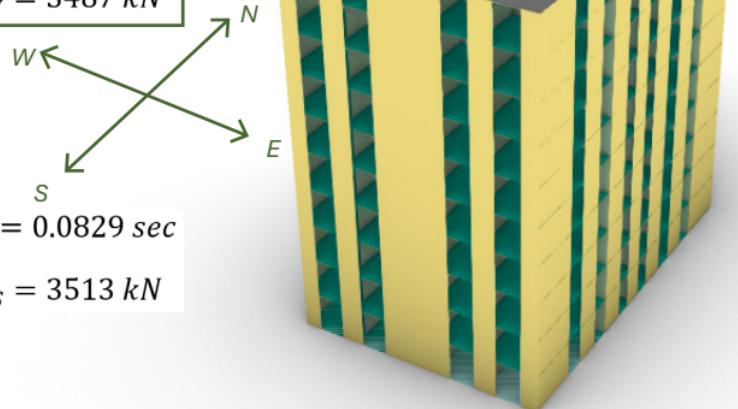
- Slight decrease in EW

$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$

$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



# Post Earthquake

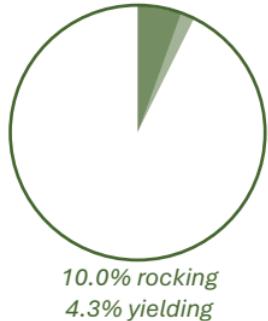
## Change

- Stiffness reduction
  - Rocking walls: 80%
  - Yielded walls: 50%

## Structure Parameters

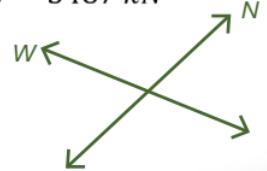
- Slight decrease in EW

## Rocking and Yielding Walls



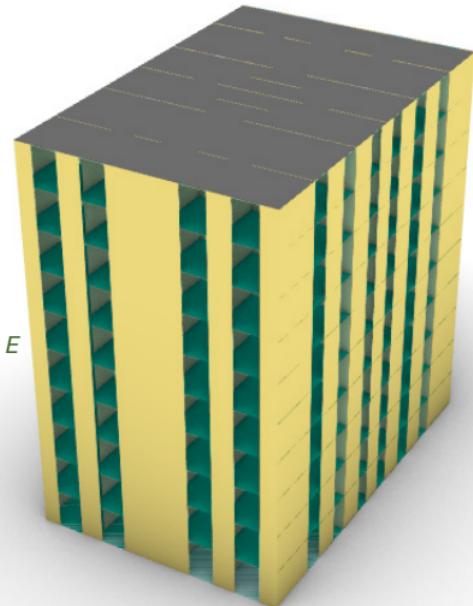
$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



# Post Earthquake

## Change

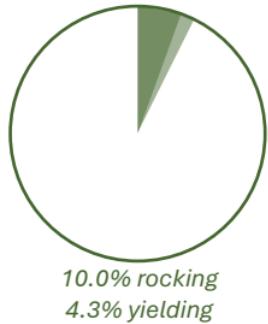
- Stiffness reduction
  - Rocking walls: 80%
  - Yielded walls: 50%

## Structure Parameters

- Slight decrease in EW

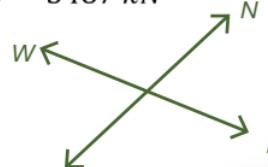
## Rocking and Yielding Walls

- Only on top two stories



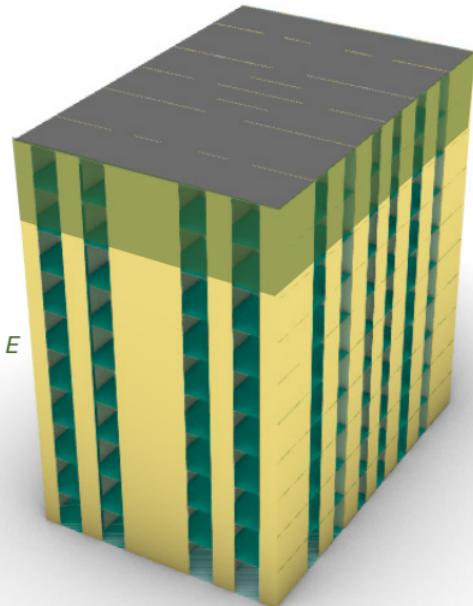
$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



# Post Earthquake

## Change

- Stiffness reduction
  - Rocking walls: 80%
  - Yielded walls: 50%

## Structure Parameters

- Slight decrease in EW

## Rocking and Yielding Walls

- Only on top two stories
  - More in 9<sup>th</sup> story now



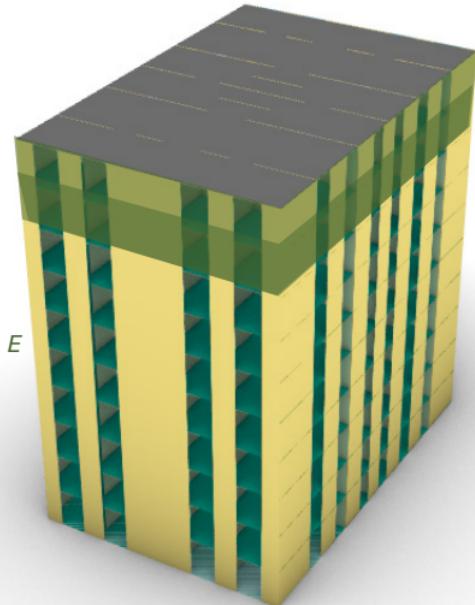
$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



# Post Earthquake

## Change

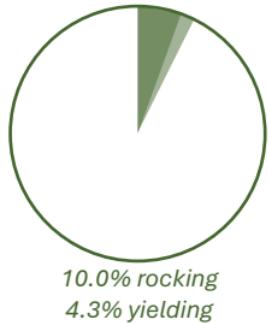
- Stiffness reduction
  - Rocking walls: 80%
  - Yielded walls: 50%

## Structure Parameters

- Slight decrease in EW

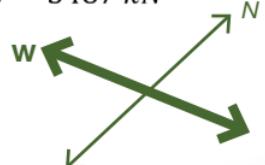
## Rocking and Yielding Walls

- Only on top two stories
  - More in 9<sup>th</sup> story now
- East-west direction only



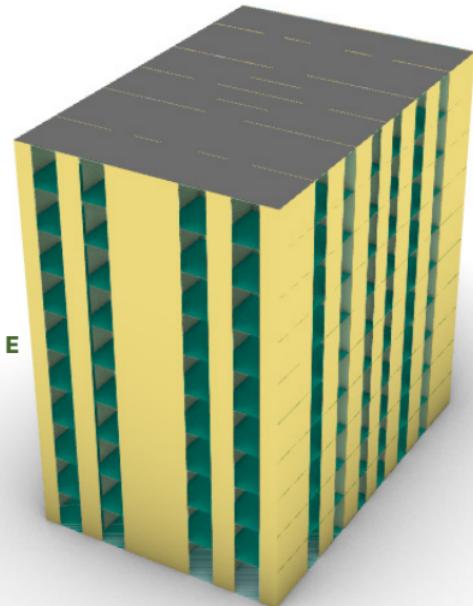
$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



# Post Earthquake

## Change

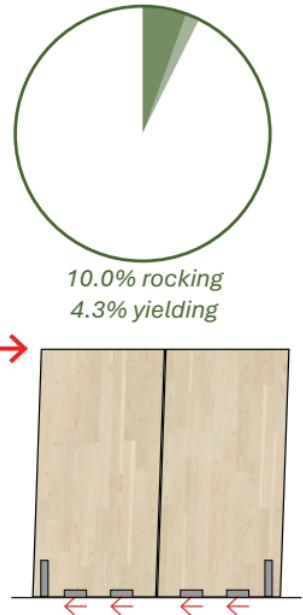
- Stiffness reduction
  - Rocking walls: 80%
  - Yielded walls: 50%

## Structure Parameters

- Slight decrease in EW

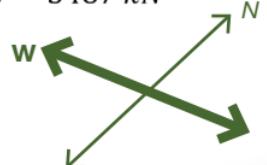
## Rocking and Yielding Walls

- Only on top two stories
  - More in 9<sup>th</sup> story now
- East-west direction only
- Yielding in from shear, not overturning moment



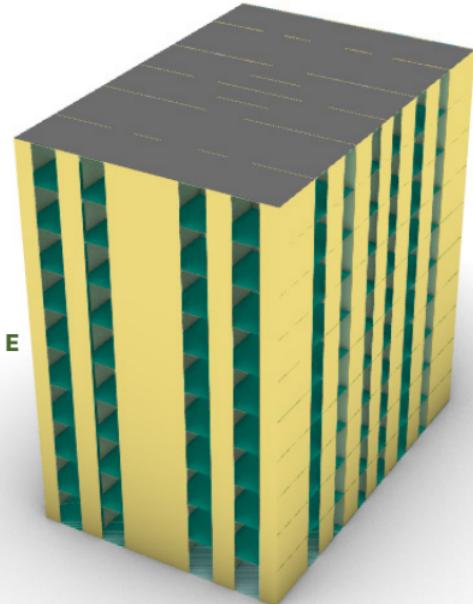
$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



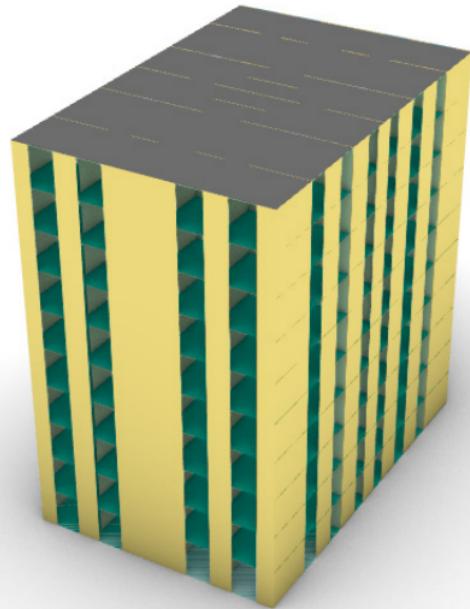
$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



# Renovation

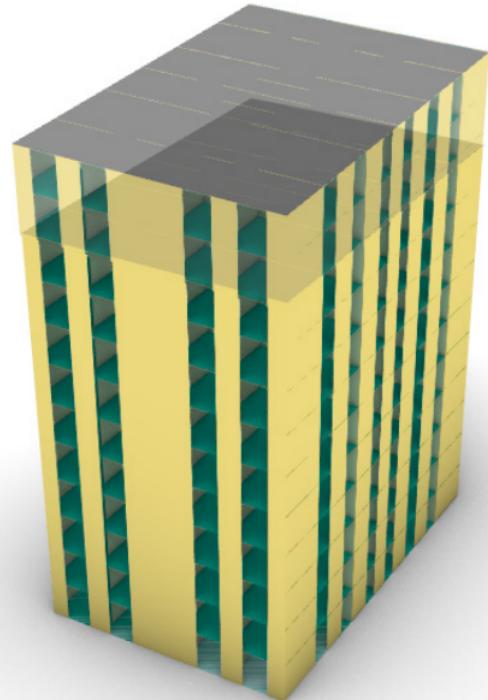
Change



# Renovation

## Change

- Add two new stories

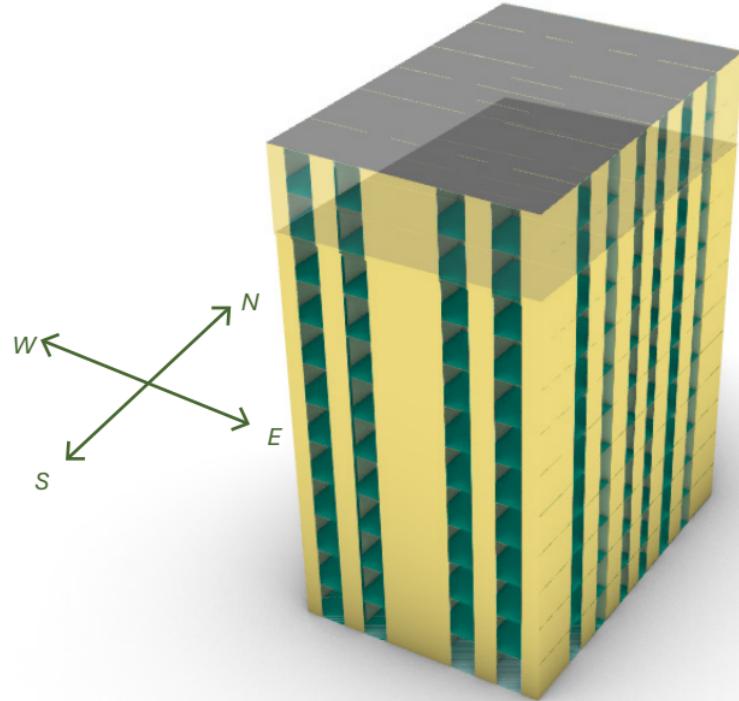


# Renovation

## Change

- Add two new stories

## Structure Parameters



# Renovation

## Change

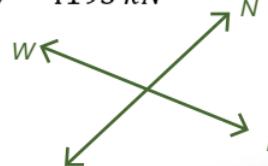
- Add two new stories

## Structure Parameters

- Building period is similar
- Base shear increases

$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 4195 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 4225 \text{ kN}$$

# Renovation

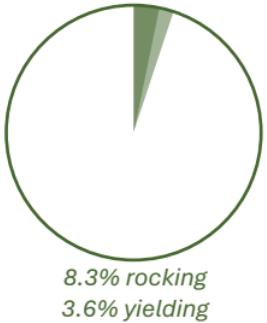
## Change

- Add two new stories

## Structure Parameters

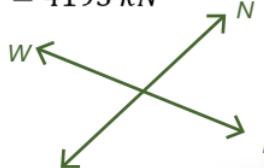
- Building period is similar
- Base shear increases

## Rocking and Yielding Walls



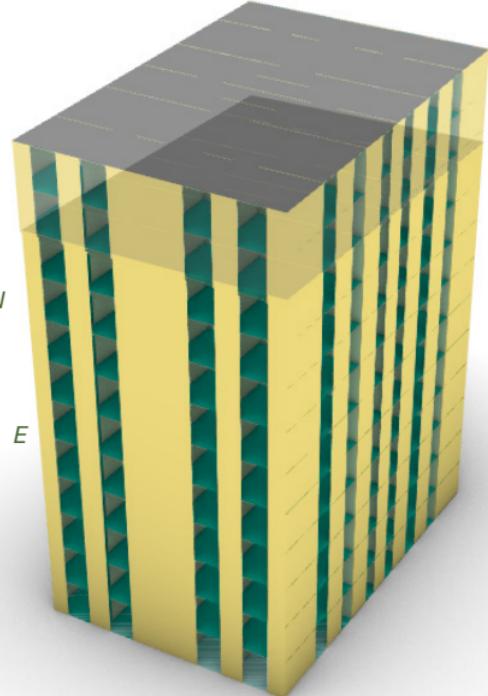
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 4195 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 4225 \text{ kN}$$



# Renovation

## Change

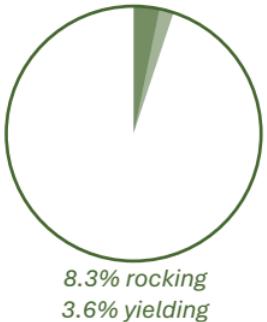
- Add two new stories

## Structure Parameters

- Building period is similar
- Base shear increases

## Rocking and Yielding Walls

- Only in top two stories



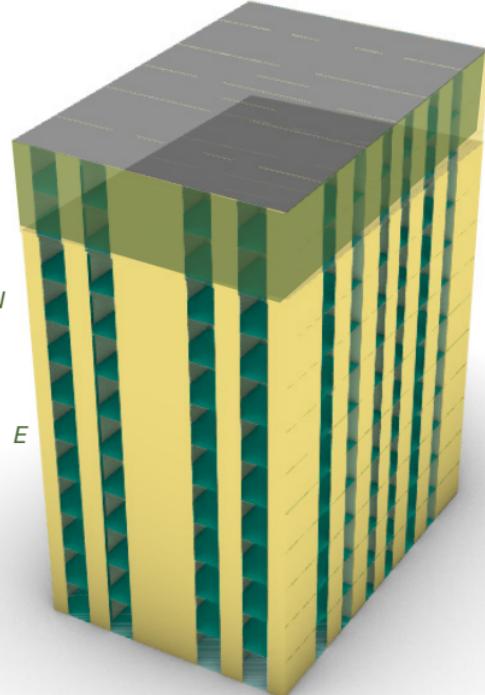
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 4195 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 4225 \text{ kN}$$



# Renovation

## Change

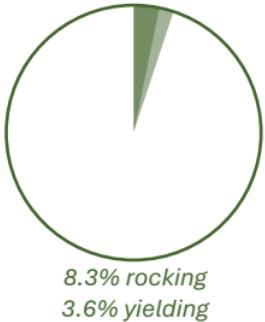
- Add two new stories

## Structure Parameters

- Building period is similar
- Base shear increases

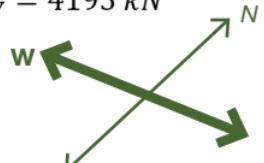
## Rocking and Yielding Walls

- Only in top two stories
- East-west direction only



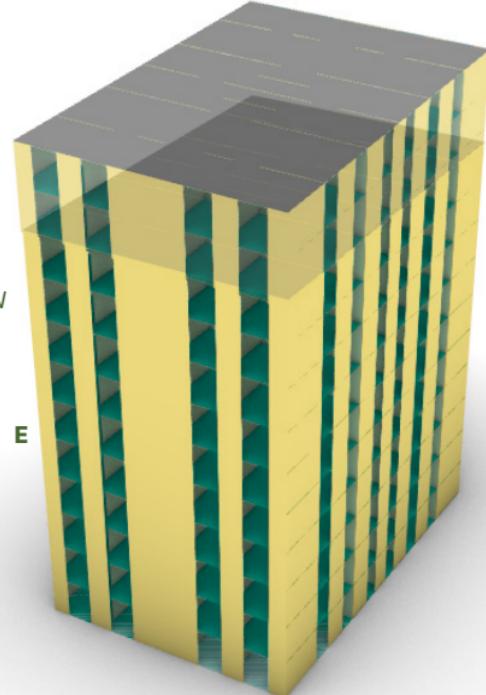
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 4195 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 4225 \text{ kN}$$



# Renovation

## Change

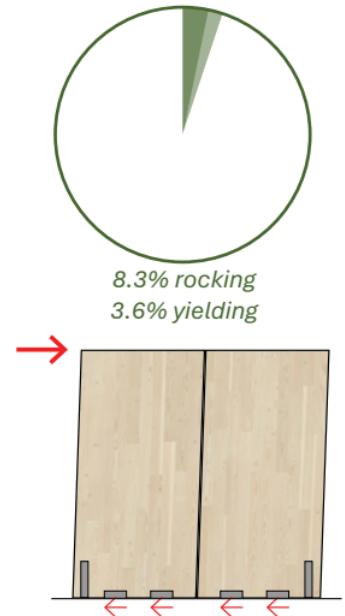
- Add two new stories

## Structure Parameters

- Building period is similar
- Base shear increases

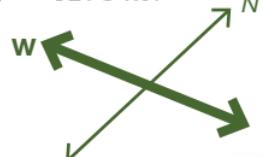
## Rocking and Yielding Walls

- Only in top two stories
- East-west direction only
- Yielding in from shear in angle brackets



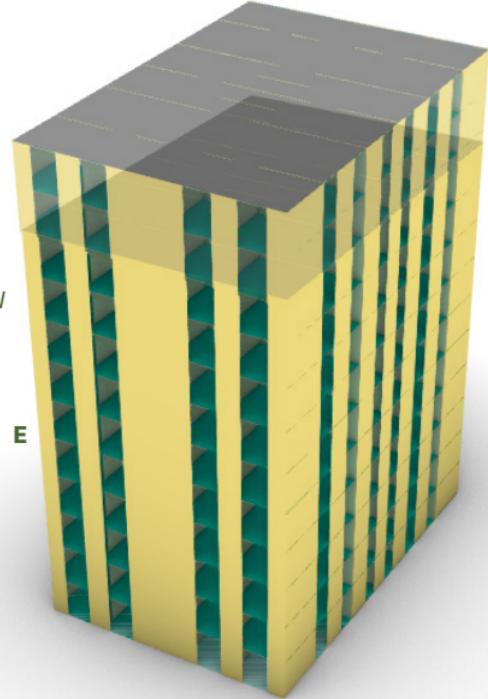
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 4195 \text{ kN}$$



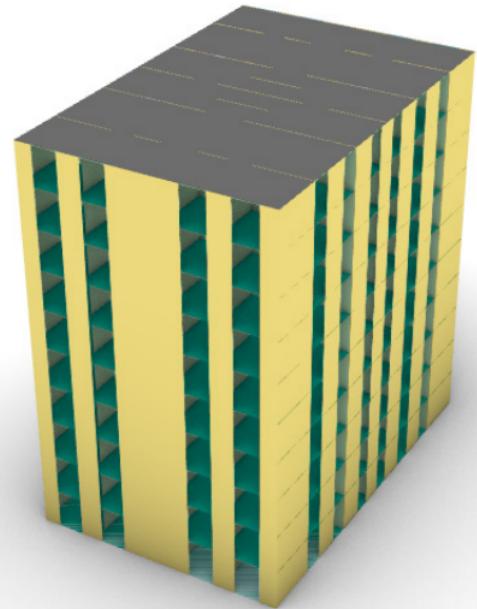
$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 4225 \text{ kN}$$



# Biological Degradation

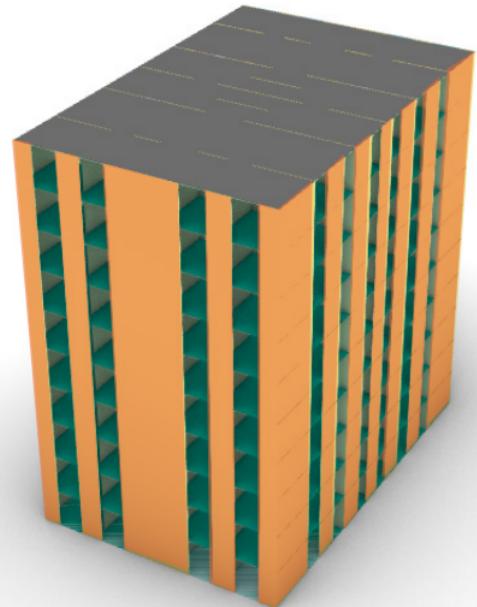
Change



# Biological Degradation

## Change

- 52 weeks of deterioration  
at exterior walls

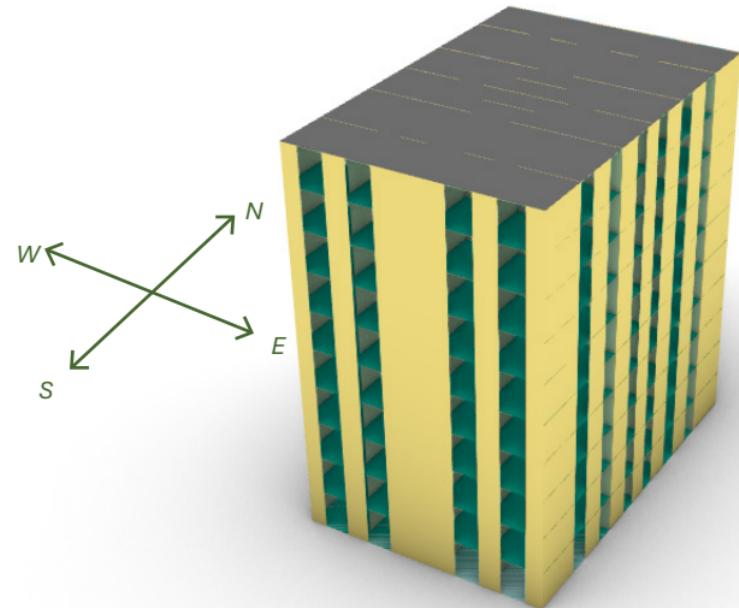


# Biological Degradation

## Change

- 52 weeks of deterioration  
at exterior walls

## Structure Parameters



# Biological Degradation

## Change

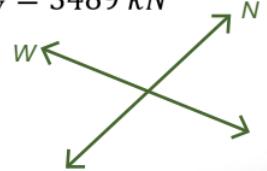
- 52 weeks of deterioration at exterior walls

## Structure Parameters

- Building period and base shear remain the same

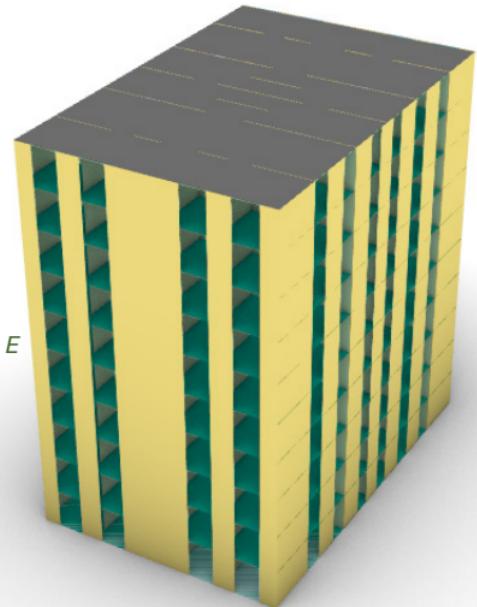
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3489 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3514 \text{ kN}$$



# Biological Degradation

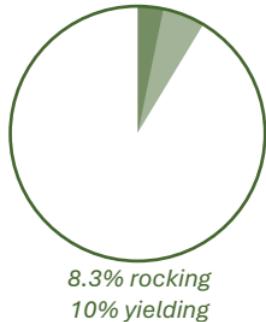
## Change

- 52 weeks of deterioration at exterior walls

## Structure Parameters

- Building period and base shear remain the same

## Rocking and Yielding Walls

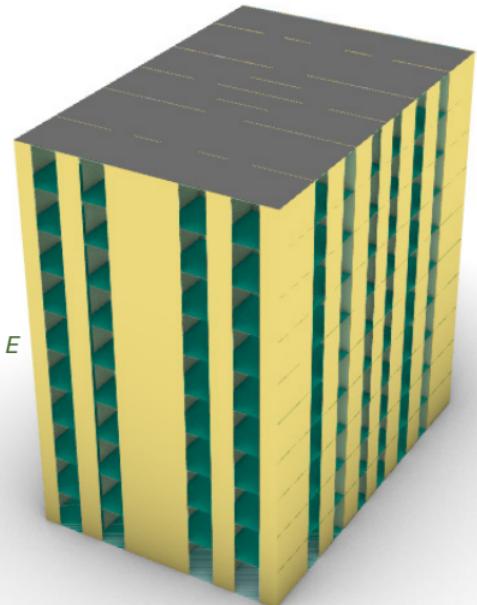
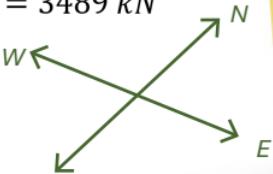


$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3489 \text{ kN}$$

$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3514 \text{ kN}$$



# Biological Degradation

## Change

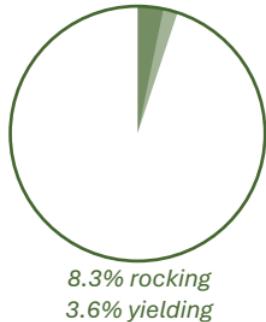
- 52 weeks of deterioration at exterior walls

## Structure Parameters

- Building period and base shear remain the same

## Rocking and Yielding Walls

- Stories 8-10

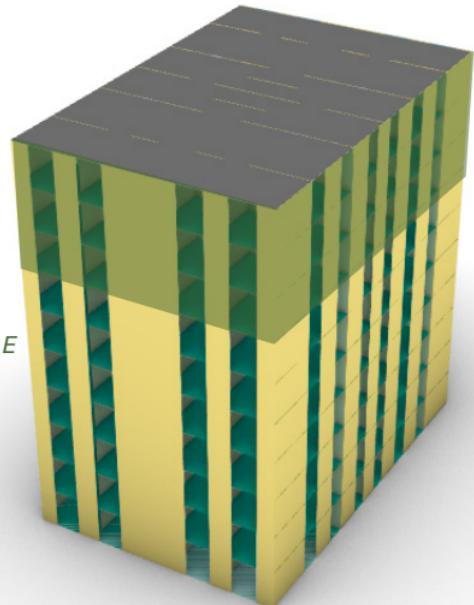


$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3489 \text{ kN}$$

$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3514 \text{ kN}$$



# Biological Degradation

## Change

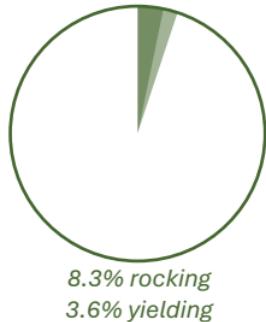
- 52 weeks of deterioration at exterior walls

## Structure Parameters

- Building period and base shear remain the same

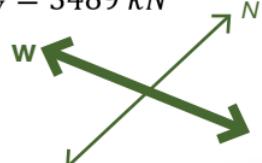
## Rocking and Yielding Walls

- Stories 8-10
- East-west direction only



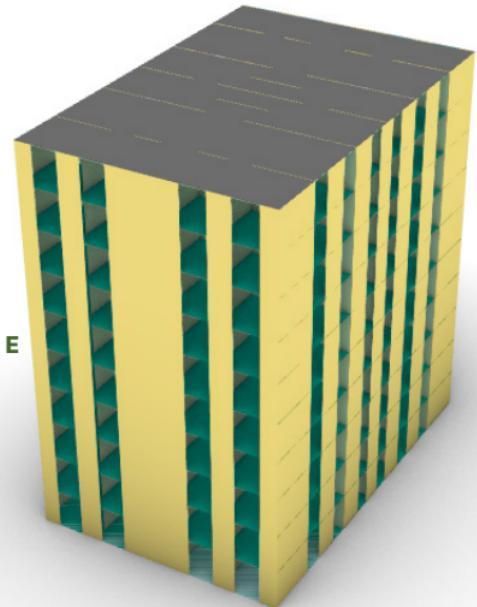
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3489 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3514 \text{ kN}$$



# Biological Degradation

## Change

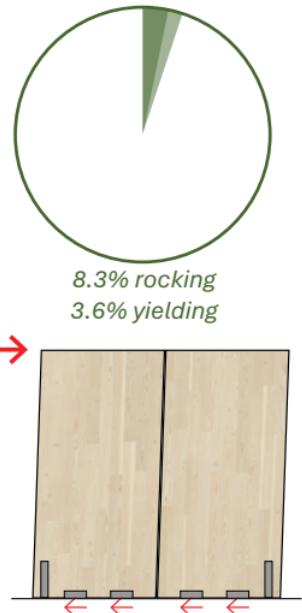
- 52 weeks of deterioration at exterior walls

## Structure Parameters

- Building period and base shear remain the same

## Rocking and Yielding Walls

- Stories 8-10
- East-west direction only
- Yielding from shear in angle brackets, and



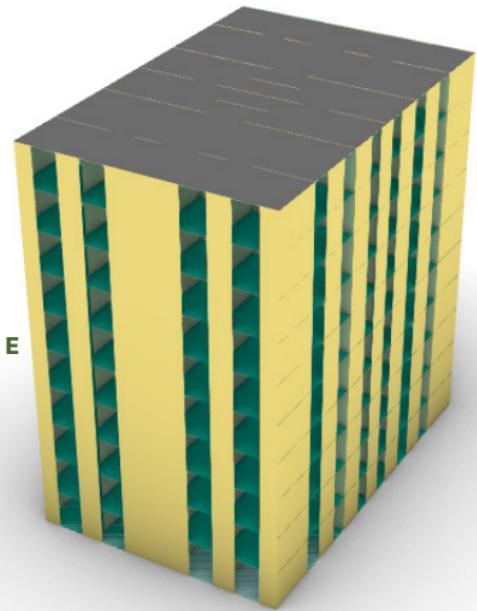
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3489 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3514 \text{ kN}$$



# Biological Degradation

## Change

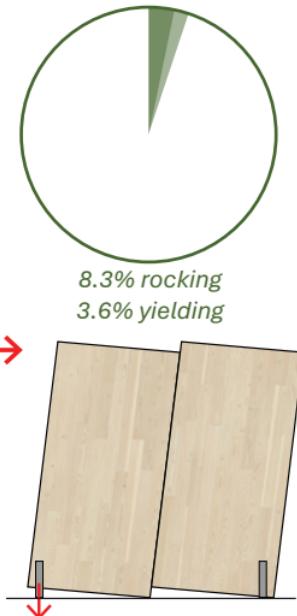
- 52 weeks of deterioration at exterior walls

## Structure Parameters

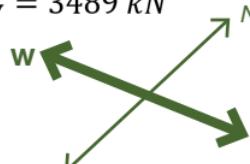
- Building period and base shear remain the same

## Rocking and Yielding Walls

- Stories 8-10
- East-west direction only
- Yielding from shear in angle brackets, and from overturning in hold-downs

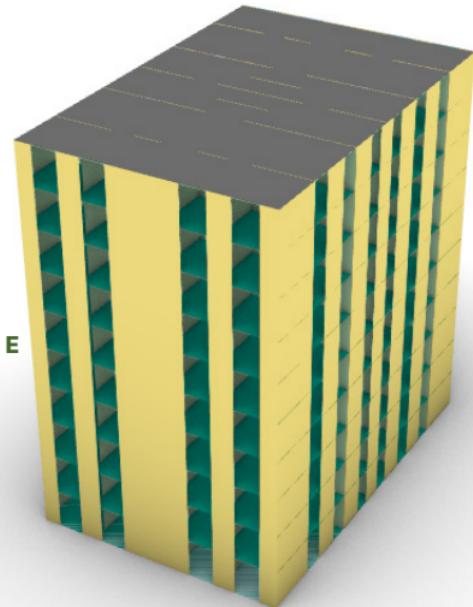


$$T_{EW} = 0.0673 \text{ sec}$$

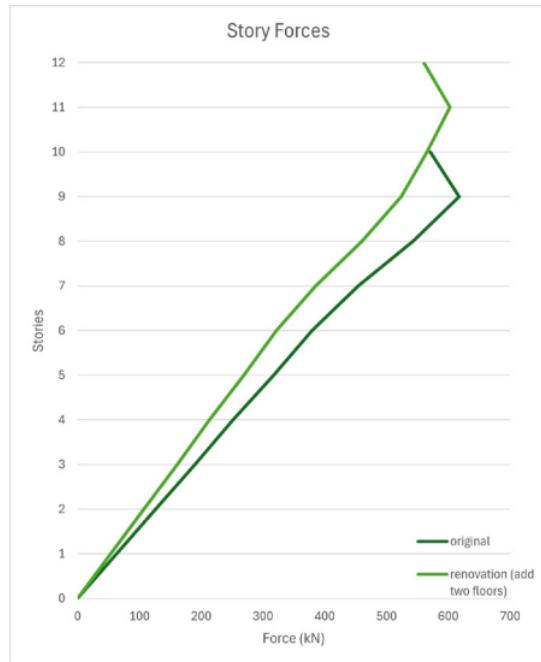
$$V_{EW} = 3489 \text{ kN}$$


$$T_{NS} = 0.0829 \text{ sec}$$

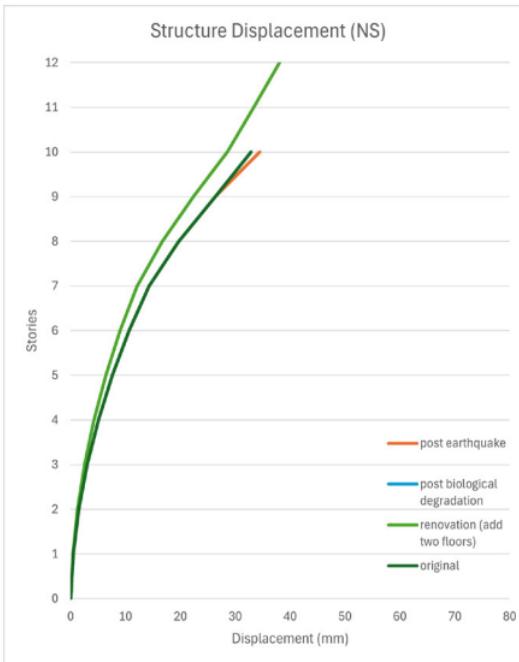
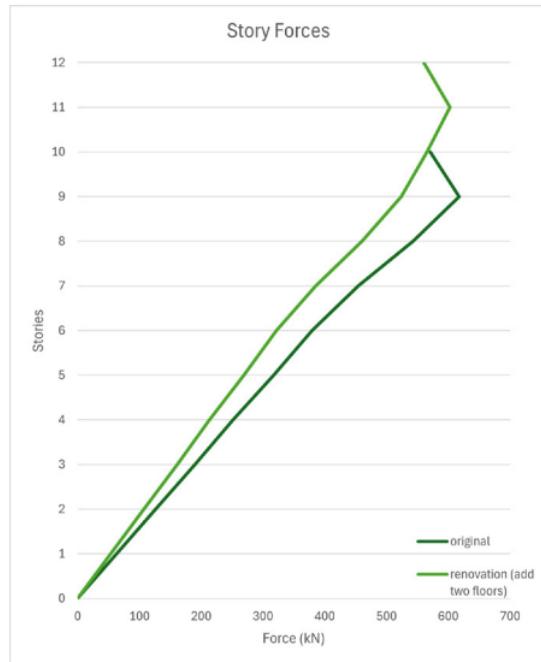
$$V_{NS} = 3514 \text{ kN}$$



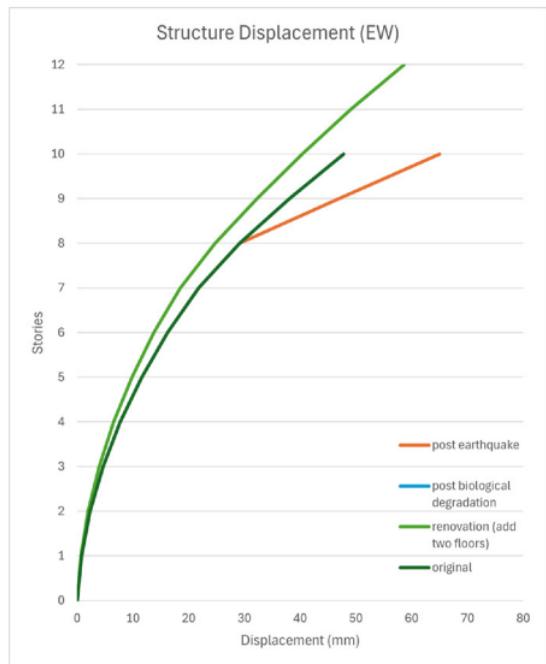
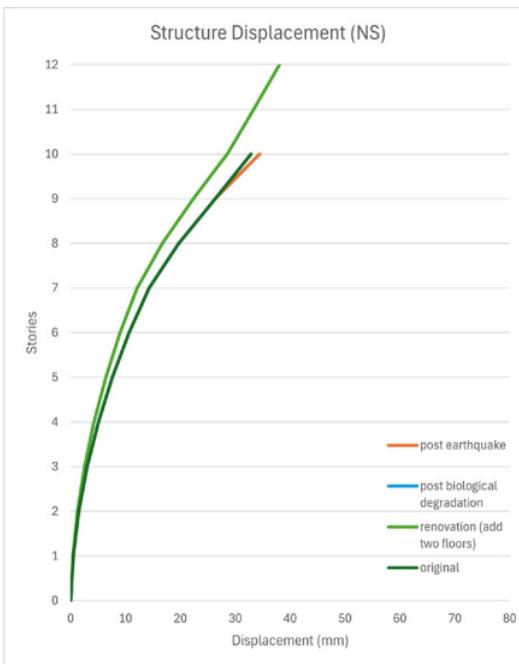
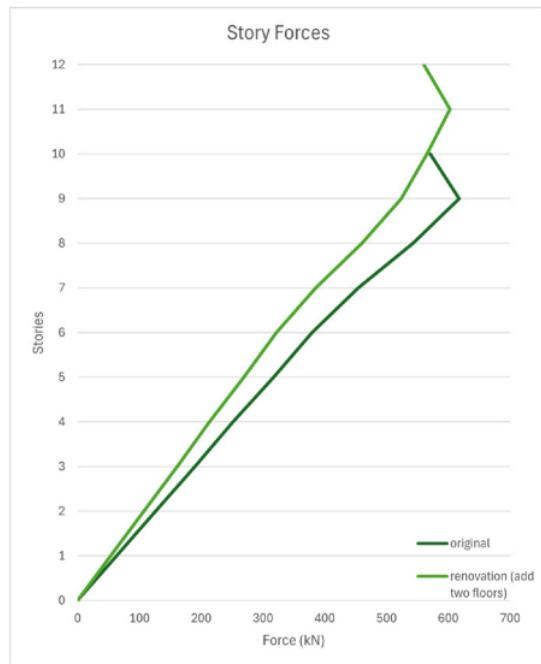
# Results Comparisons



# Results Comparisons



# Results Comparisons

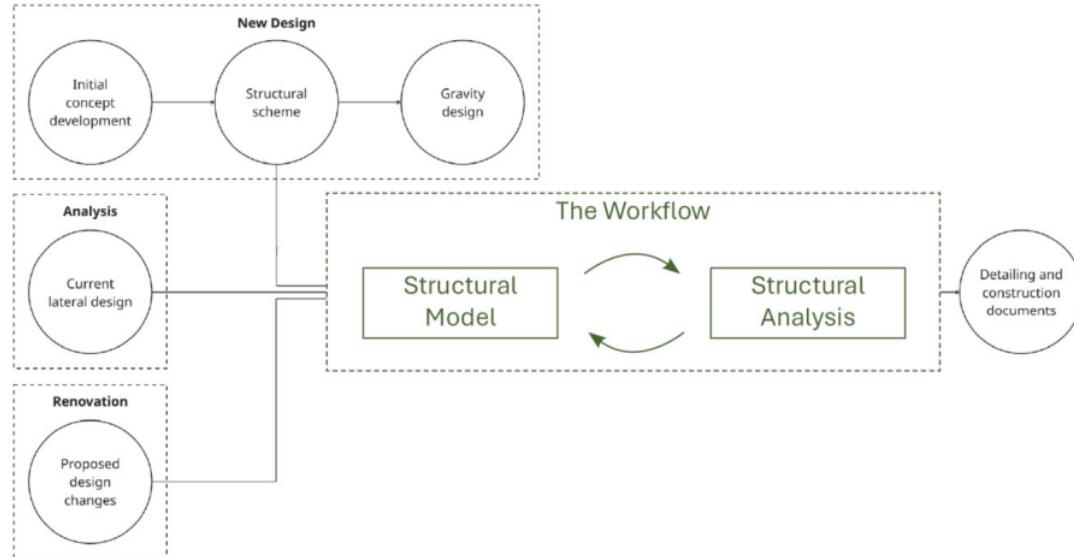


# Conclusions

# Conclusions

## Goals

- Integrated into the main workflow



# Conclusions

## Goals

- Integrated into the main workflow
- Applicable for common tall timber construction



Rectilinear

## *Form and Function*

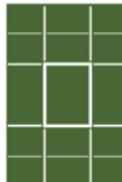


8+ stories

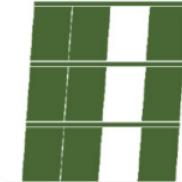


Residential

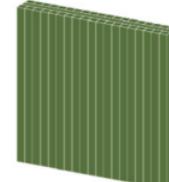
## *Structural System*



Central Core



Shear Walls

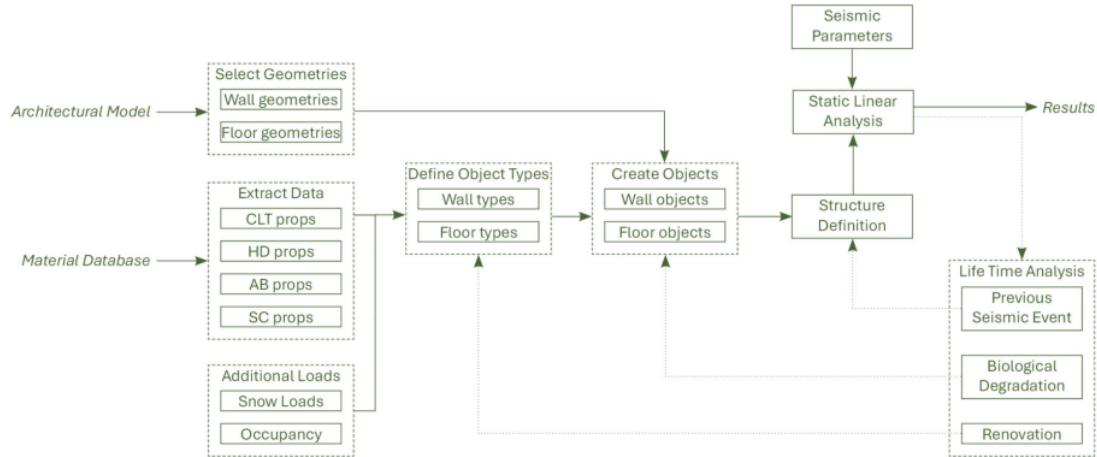


CLT Walls

# Conclusions

## Goals

- Integrated into the main workflow
- Applicable for common tall timber construction
- Simple to implement and adaptable



# Conclusions

## Goals

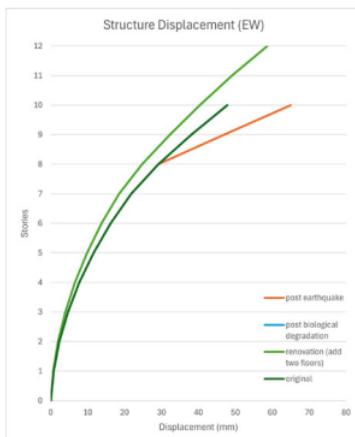
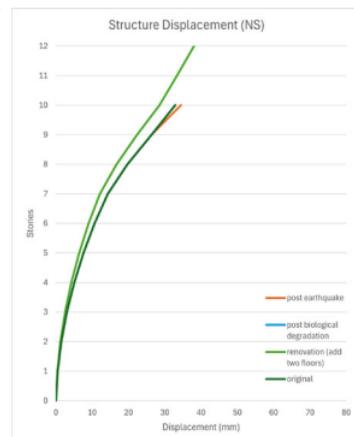
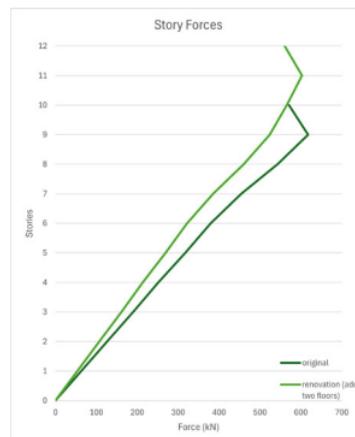
- Integrated into the main workflow
- Applicable for common tall timber construction
- Simple to implement and adaptable
- Includes lifetime analysis



# Conclusions

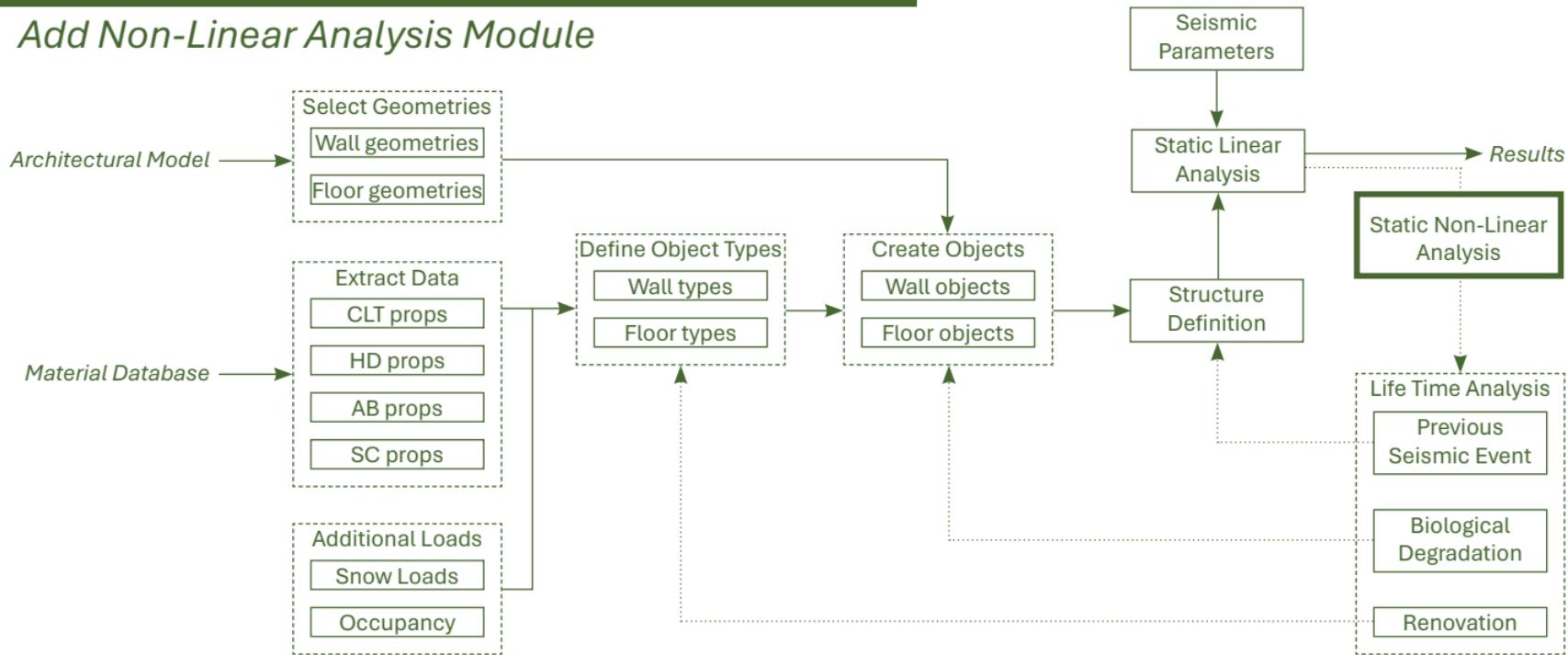
## Goals

- Integrated into the main workflow
- Applicable for common tall timber construction
- Simple to implement and adaptable
- Includes lifetime analysis
- Output format is good for post-processing



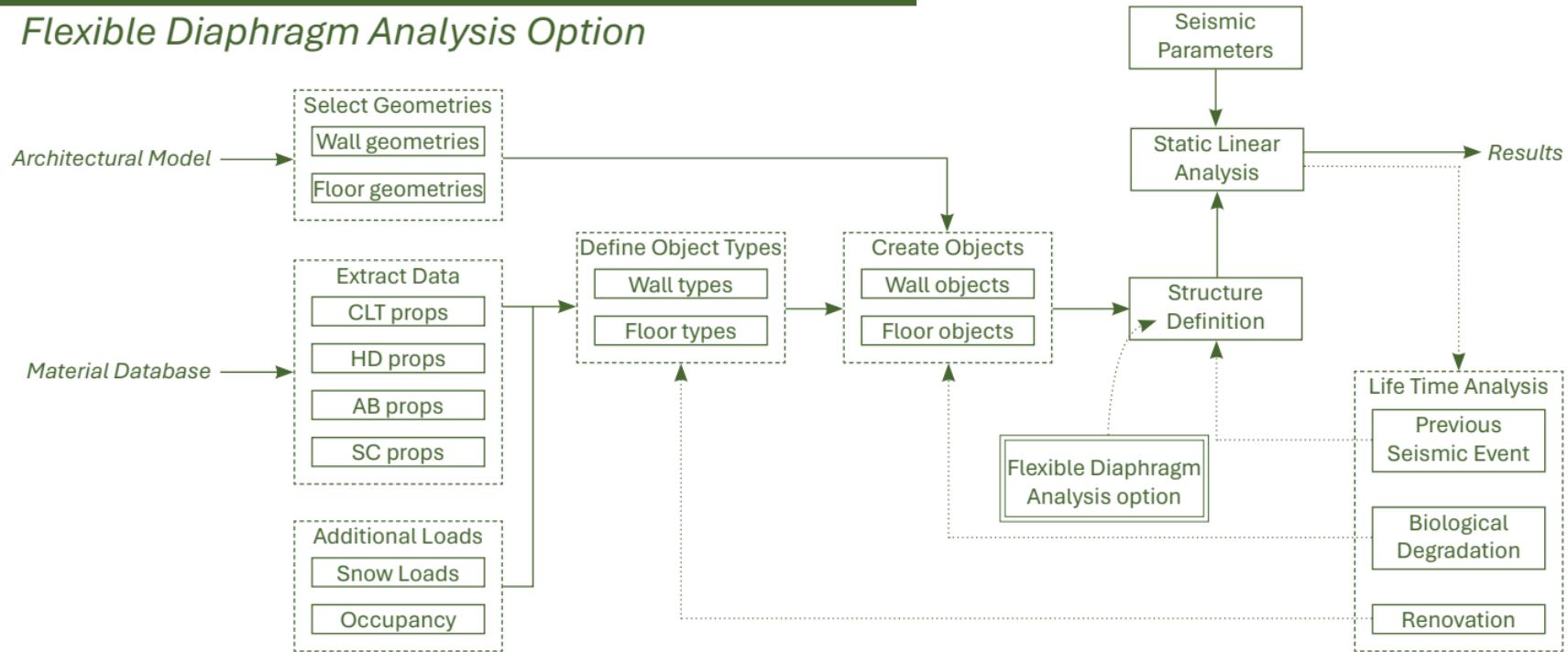
# Future Development

## Add Non-Linear Analysis Module



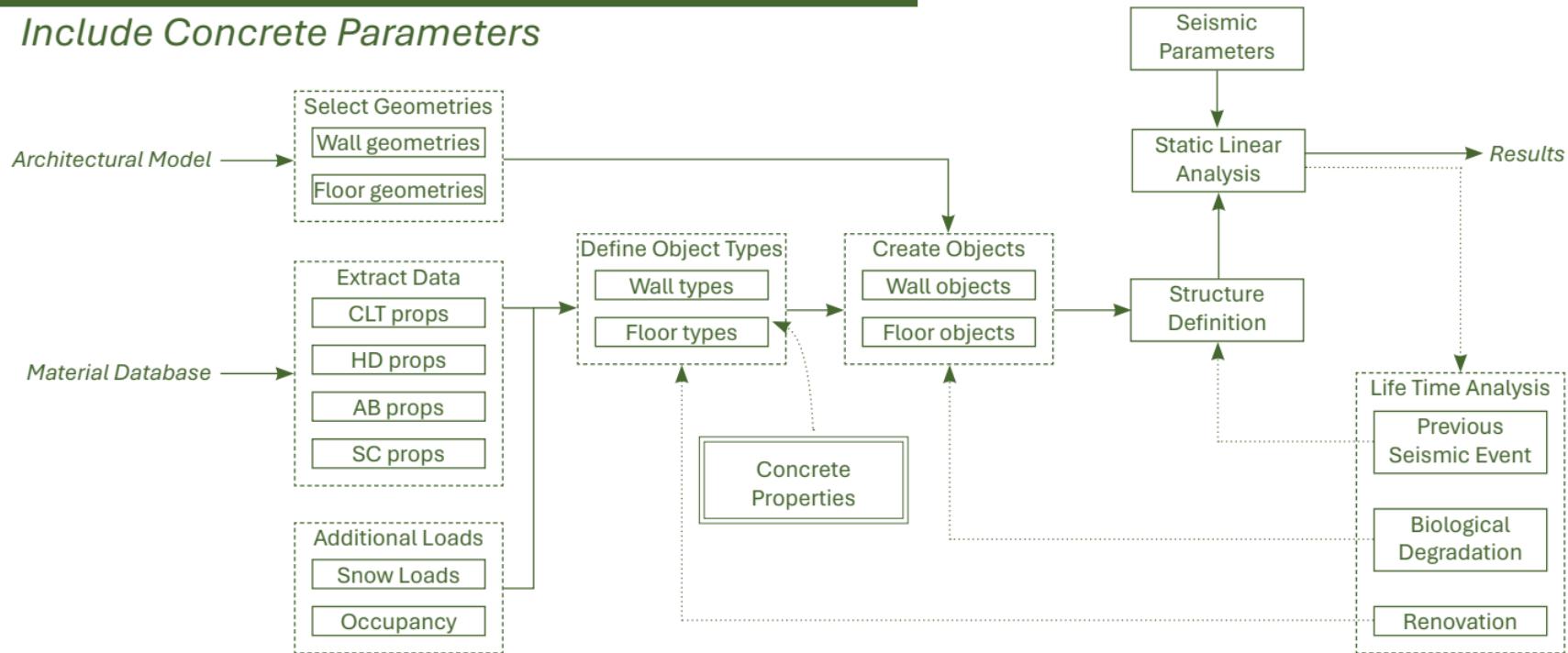
# Future Development

## Flexible Diaphragm Analysis Option



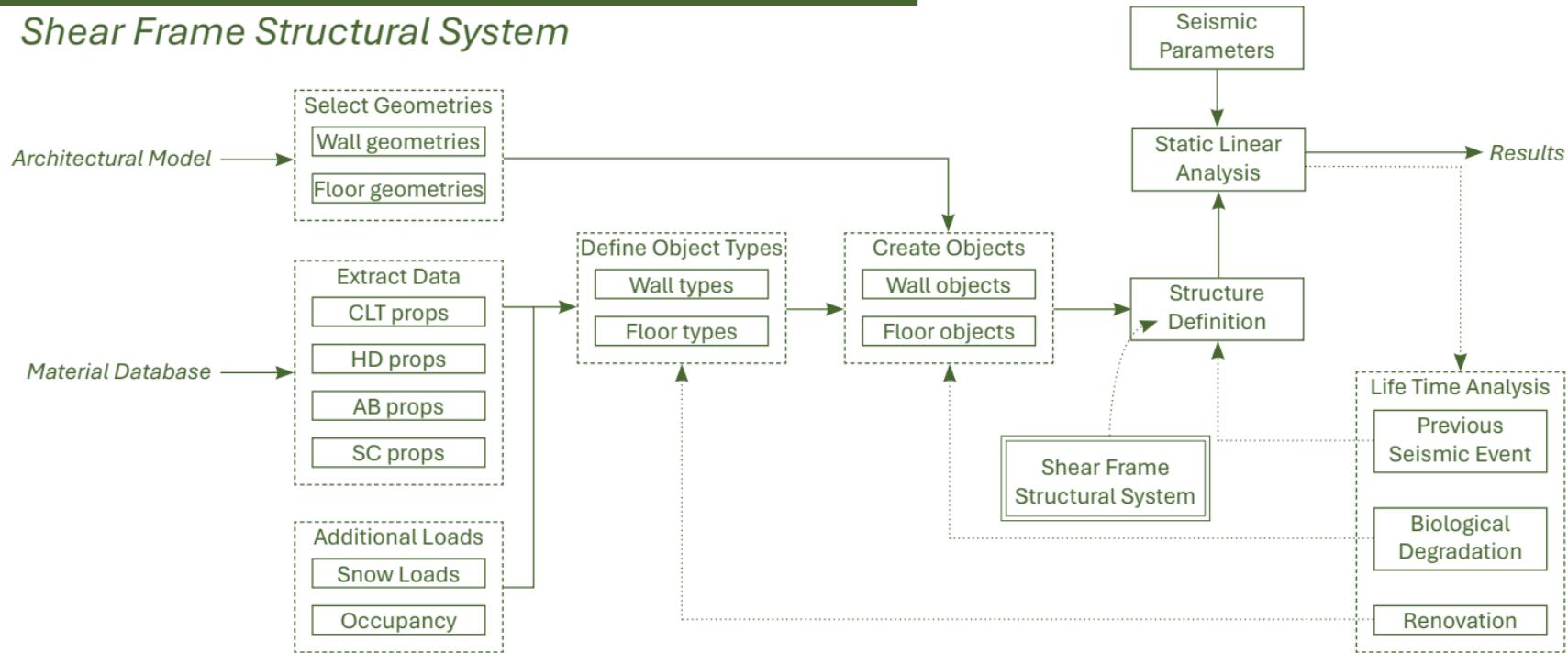
# Future Development

## *Include Concrete Parameters*



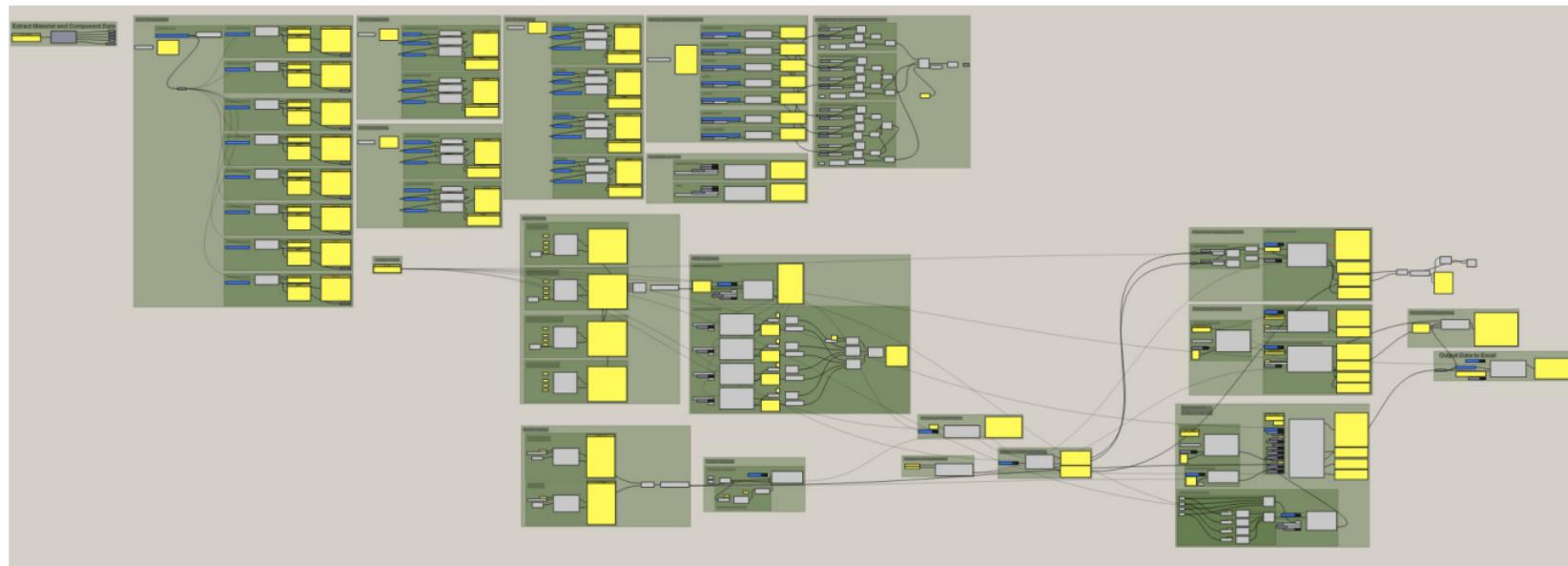
# Future Development

## *Shear Frame Structural System*



# Future Development

## User Interface



Thank you!

# References

Blass, H., and Peter Fellmoser. "Design of Solid Wood Panels with Cross Layers," 2004. <https://www.semanticscholar.org/paper/Design-of-solid-wood-panels-with-cross-layers-Blass-Fellmoser/0ac31e3f0a8923666100baa6e19f6a87c91f7a1a>.

Bogensperger, Thomas, Thomas Moosbrugger, and Gregor Silly. "Verification of CLT-Plates under Loads in Plane," June 2, 2016.

Christovasilis, I. P., L. Riparbelli, G. Rinaldin, and G. Tamagnone. "Methods for Practice-Oriented Linear Analysis in Seismic Design of Cross Laminated Timber Buildings." *Soil Dynamics and Earthquake Engineering* 128 (January 1, 2020): 105869. <https://doi.org/10.1016/j.soildyn.2019.105869>.

Dong, Weiqun, Zhiqiang Wang, Jianhui Zhou, Hao Zhang, Yue Yao, Wei Zheng, Meng Gong, and Xinyi Shi. "Embedment Strength of Smooth Dowel-Type Fasteners in Cross-Laminated Timber." *Construction and Building Materials* 233 (February 2020): 117243. <https://doi.org/10.1016/j.conbuildmat.2019.117243>.

"EN1995-1-1 General - Common Rules and Rules for Buildings - DRAFT 2023." In *Eurocode 5: Design of Timber Structures*., 2023.

Flaig, M, and H J Blaß. "Shear Strength and Shear Stiffness of CLT-Beams Loaded in Plane," 2013.

Foster, R.M., and Michael H. Ramage. "Tall Timber." *Nonconventional and Vernacular Construction Materials*, 2020, pp. 467–490, <https://doi.org/10.1016/b978-0-08-1027042.00017-2>.

Gavric, Igor, Massimo Fragiacomo, and Ario Ceccotti. "Cyclic Behaviour of Typical Metal Connectors for Cross-Laminated (CLT) Structures." *Materials and Structures* 48, no. 6 (June 2015): 1841–57. <https://doi.org/10.1617/s11527-014-0278-7>.

Gavric, Igor, Massimo Fragiacomo, and Ario Ceccotti. "Cyclic Behavior of Typical Screwed Connections for Cross-Laminated (CLT) Structures." *European Journal of Wood and Wood Products* 73, no. 2 (March 2015): 179–91. <https://doi.org/10.1007/s00107-014-0877-6>.

Gavric, Igor, Massimo Fragiacomo, and Ario Ceccotti. "Cyclic Behavior of CLT Wall Systems: Experimental Tests and Analytical Prediction Models." *Journal of Structural Engineering* 141, no. 11 (November 2015): 04015034. [https://doi.org/10.1061/\(ASCE\)ST.1943-541X.0001246](https://doi.org/10.1061/(ASCE)ST.1943-541X.0001246).

González-Retamal, Marcelo, Eric Forcael, Gerardo Saelzer-Fuica, and Mauricio Vargas-Mosqueda. "From Trees to Skyscrapers: Holistic Review of the Advances and Limitations of Multi-Storey Timber Buildings." *Buildings* 12, no. 8 (August 2022): 1263. <https://doi.org/10.3390/buildings12081263>.

# References

Ilgin, Hüseyin Emre. "Analysis of the Main Architectural and Structural Design Considerations in Tall Timber Buildings." *Buildings* 14, no. 1 (December 22, 2023): 43. <https://doi.org/10.3390/buildings14010043>.

Izzi, Matteo, Daniele Casagrande, Stefano Bezzi, Dag Pasca, Maurizio Follesa, and Roberto Tomasi. "Seismic Behaviour of Cross-Laminated Timber Structures: A State-of-the-Art Review." *Engineering Structures* 170 (September 1, 2018): 42–52. <https://doi.org/10.1016/j.engstruct.2018.05.060>.

Laguarda Mallo, M. F., & Espinoza, O. (2015). Awareness, perceptions and willingness to adopt cross-laminated timber by the architecture community in the United States. *Journal of Cleaner Production*, 94, 198–210. <https://doi.org/10.1016/j.jclepro.2015.01.090>.

Kennedy, S., A. Salenikovich, W. Munoz, and M. Mohammad. "Design Equations for Dowel Embedment Strength and Withdrawal Resistance for Threaded Fasteners in CLT." *Proceedings of the World Conference on Timber Engineering*, 2014, Quebec, Canada.

Mitchell, H., Kotsovinos, P., Richter, F., Thomson, D., Barber, D., & Rein, G. (2022). Review of fire experiments in mass timber compartments: Current understanding, limitations, and research gaps. *Fire and Materials*, 47(4), 415–432. <https://doi.org/10.1002/fam.3121>.

Philion, Ethan, et al. "Structural fire modeling strategies for exposed mass timber compartments and experimental gaps for model validation." *Journal of Performance of Constructed Facilities*, vol. 36, no. 6, 2022, [https://doi.org/10.1061/\(ASCE\)CF.19435509.0001761](https://doi.org/10.1061/(ASCE)CF.19435509.0001761).

Pozza, Luca, Marco Savoia, Luca Franco, Anna Saetta, and Diego Talledo. "Effect Of Different Modelling Approaches On The Prediction Of The Seismic Response Of Multi-Storey CLT Buildings," 2017. <https://www.witpress.com/elibrary/cmem-volumes/5/6/1930>.

Pozza, Luca, Roberto Scotta, Davide Trutalli, and Andrea Polastri. "Behaviour Factor for Innovative Massive Timber Shear Walls." *Bulletin of Earthquake Engineering* 13, no. 11 (November 1, 2015): 3449–69. <https://doi.org/10.1007/s10518-015-9765-7>.

Puettmann, M. E., & Wilson, J. (2005). Life-cycle analysis of wood products: cradle-to-gate LCI of residential wood building materials. *Wood and Fiber Science*, 37, 18–29.

Rinaldi, Vincenzo, Daniele Casagrande, Catia Cimini, Maurizio Follesa, and Massimo Fragiacomo. "An Upgrade of Existing Practice-Oriented FE Design Models for the Seismic Analysis of CLT Buildings." *Soil Dynamics and Earthquake Engineering* 149 (October 1, 2021): 106802. <https://doi.org/10.1016/j.soildyn.2021.106802>.

Schmidt, Evan, and Mariapaola Riggio. "Monitoring Moisture Performance of Cross-Laminated Timber Building Elements during Construction." *Buildings* 9, no. 6 (June 2019): 144. <https://doi.org/10.3390/buildings9060144>.

# References

The Efficient Engineer. (2021, April 27). Understanding the Finite Element Method [Video]. YouTube. <https://www.youtube.com/watch?v=GHjopp47vvQ>.

Udele, Kenneth Emamoke, Jeffrey J. Morrell, Anthony Newton, and Arijit Sinha. "Evaluation of Dowel Bearing Strength of Fungal-Decayed Cross-Laminated Timber." *Wood Material Science & Engineering* 19, no. 3 (May 3, 2024): 564–72. <https://doi.org/10.1080/17480272.2023.2269392>.

Udele, Kenneth Emamoke, Jeffrey J. Morrell, Jed Cappellazzi, and Arijit Sinha. "Characterizing Properties of Fungal-Decayed Cross Laminated Timber (CLT) Connection Assemblies." *Construction and Building Materials* 409 (December 2023): 134080. <https://doi.org/10.1016/j.conbuildmat.2023.134080>.

Uibel, T., and H.J. Blab. "Joints with Dowel Type Fasteners in CLT Structures." Focus Solid Timber Solutions–European Conference on Cross Laminated Timber (CLT), COST Action FP1004, 2013, pp. 119–134. Graz, Austria.

Viitanen, Hannu, and Tuomo Ojanen. "Improved Model to Predict Mold Growth in Building Materials." Thermal Performance of the Exterior Envelopes of Whole Buildings X–Proceedings CD, 2007.

Viitanen, Hannu, Juha Vinha, Kati Salminen, Tuomo Ojanen, Ruut Peuhkuri, Leena Paajanen, and Kimmo Lähdesmäki. "Moisture and Bio-Deterioration Risk of Building Materials and Structures." *Journal of Building Physics* 33, no. 3 (January 1, 2010): 201–24. <https://doi.org/10.1177/1744259109343511>.

Wang, Wei. "Research on Seismic Design of High-Rise Buildings Based on Framed-Shear Structural System." *Frontiers Research of Architecture and Engineering* 3, no. 3 (December 14, 2020): 87–90. <https://doi.org/10.30564/frae.v3i3.2670>.

Wang, X., Q. Xu, X. Wang, J. Guo, W. Cao, and C. Xiao. "Strength Degradation of Wood Members Based on the Correlation of Natural and Accelerated Decay Experiments." *Journal of Renewable Materials* 8, no. 5 (2020): 565–577. <https://doi.org/10.32604/jrm.2020.09020>.

Wieringa, R. J., and J. M. G. Heerkens. "The Methodological Soundness of Requirements Engineering Papers: A Conceptual Framework and Two Case Studies." *Requirements Engineering* 11, no. 4 (September 2006): 295–307. <https://doi.org/10.1007/s00766-006-0037-6>.

Yan, Luyue, Yi Li, Wen-Shao Chang, and Haoyu Huang. "Seismic Control of Cross Laminated Timber (CLT) Structure with Shape Memory Alloy-Based Semi-Active Tuned Mass Damper (SMA-STMD)." *Structures* 57 (November 1, 2023): 105093. <https://doi.org/10.1016/j.istruc.2023.105093>.