



Dynamic Timber

*A Seismic Analysis Workflow for Tall Timber Structures
with Variable Parameters*

Presentation Contents

Background

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Background
Research Goals

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Project Scope

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Background
Research Goals
Project Scope
The Workflow

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The Workflow
Case Study

Presentation Contents

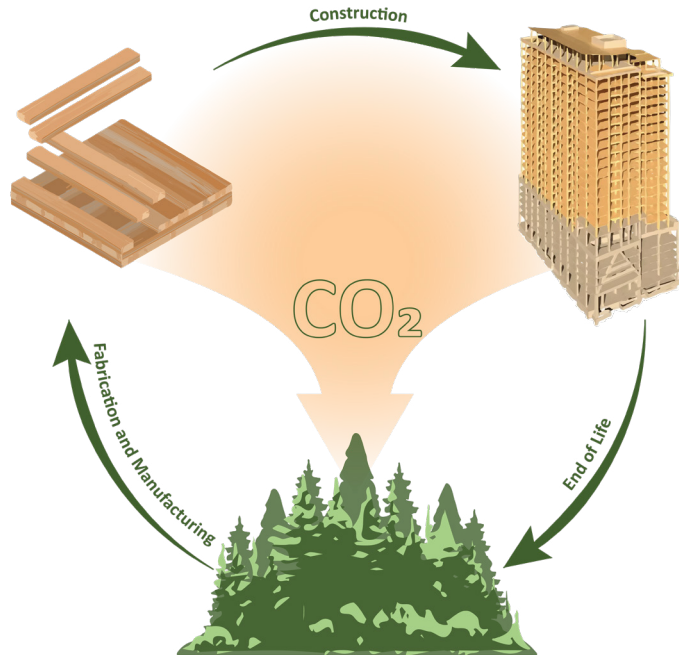
Background
Research Goals
Project Scope
The Workflow
Case Study
Conclusion

Background

Why Timber?

Carbon Neutral

- Carbon sequestration
- 1m^3 of wood = 1.1 tons of CO_2



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Occupant Health

- Natural materials make for healthier spaces



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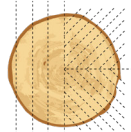
Occupant Health

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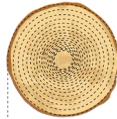
New Engineered Products



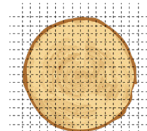
Heartwood is sourced from center of tree trunk



Planing or Quartering



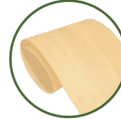
Slicing



Ripping



Solid Sawn Lumber



Timber Veneer



Timber Strands



Cross-Laminated Timber



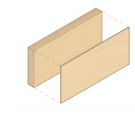
Mass Plywood Panels



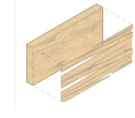
Laminated Strand Lumber



Glue-Laminated Timber



Laminated Veneer Lumber



Parallel Strand Lumber

Why Timber?

Carbon Neutral

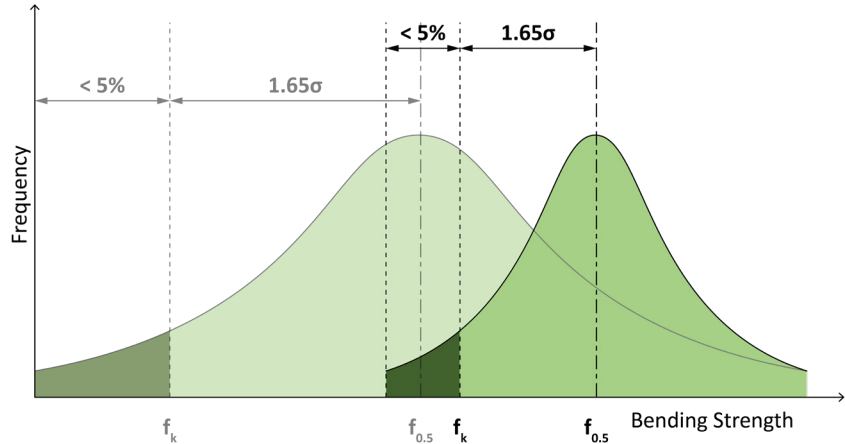
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New Engineered Products

- Elements are more reliable



f_k = Characteristic Strength

$f_{0.5}$ = Mean Strength

Why Timber?

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Occupant Health

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New Engineered Products

- Elements are more reliable
- New structure types possible

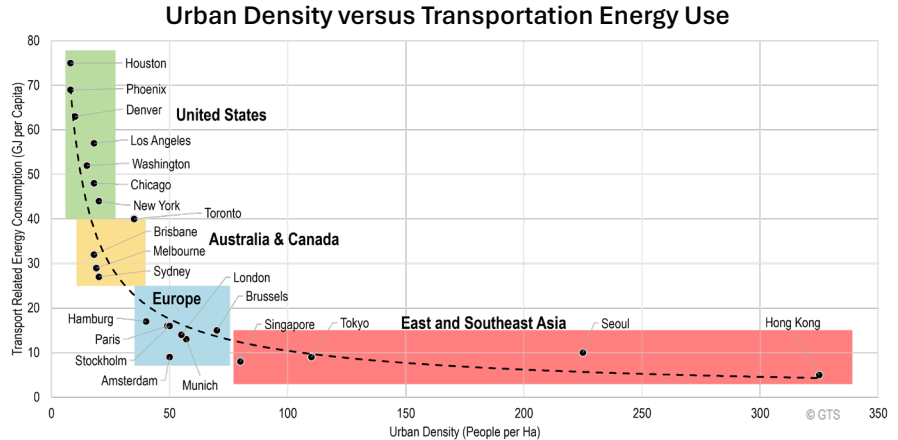


Mjøstårnet - Brumunddal, Norway - Voll Arkitekter

Why Tall Timber?

Increased Density

- Higher density cities use less transportation energy



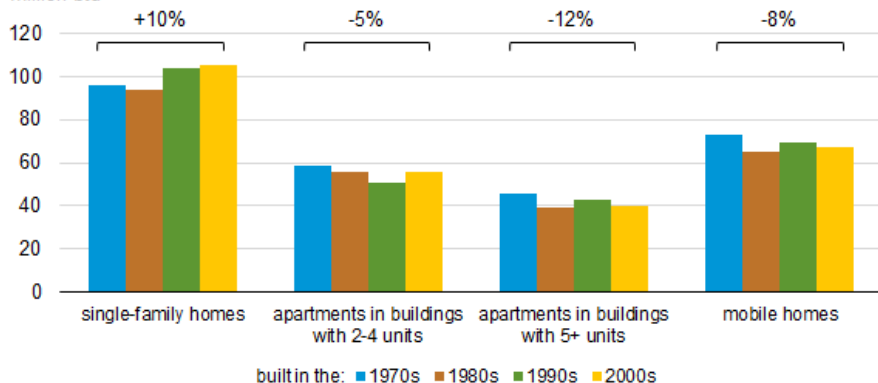
Source: *The Geography of Transportation Systems*

Why Tall Timber?

Increased Density

- Higher density cities use less transportation energy
- High density living has lower emissions per floor area

Site energy use per household in 2009 by year of construction
million btu



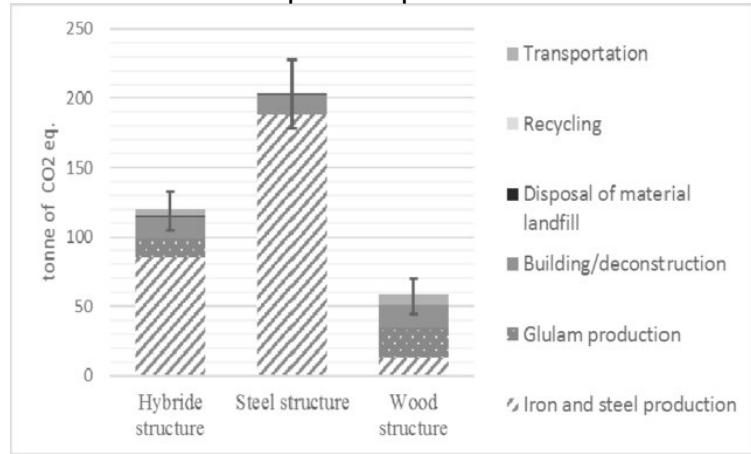
Source: US Energy Information Administration

Why Tall Timber?

Increased Density

- Higher density cities use less transportation energy
- High density living has lower emissions per floor area
- Need low carbon solutions for such structures

Material carbon footprint comparison for like structures



Source: Laurent et al. 2019

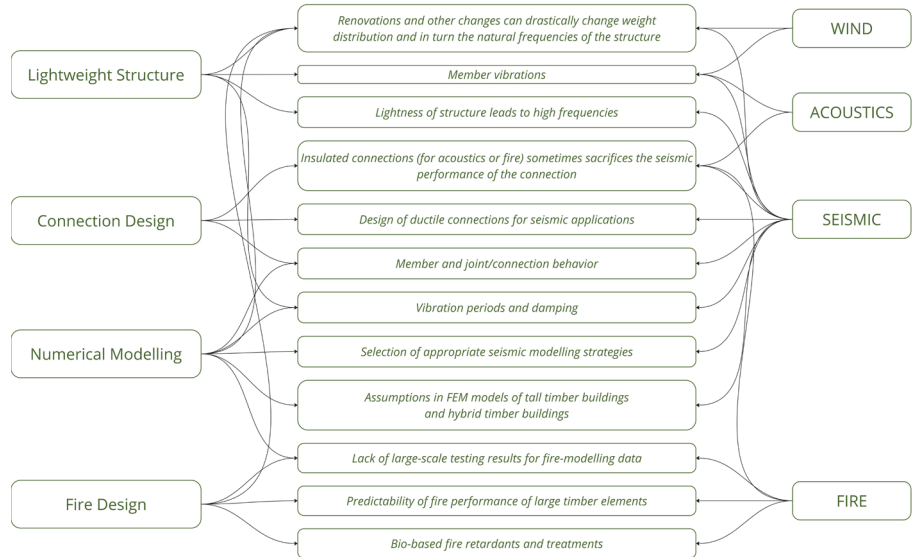
Why Tall Timber?

Increased Density

- Higher density cities use less transportation energy
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Current Challenges

- Lightweight structure
- Connection design
- Computational modelling
- Fire design



Why Seismic Analysis?

(of tall timber structures)

Material Challenges

- Anisotropic material
 - behaves differently in each direction



Why Seismic Analysis?

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Material Challenges

- Anisotropic material
 - behaves differently in each direction
- Natural material
 - subject to deterioration



Why Seismic Analysis?

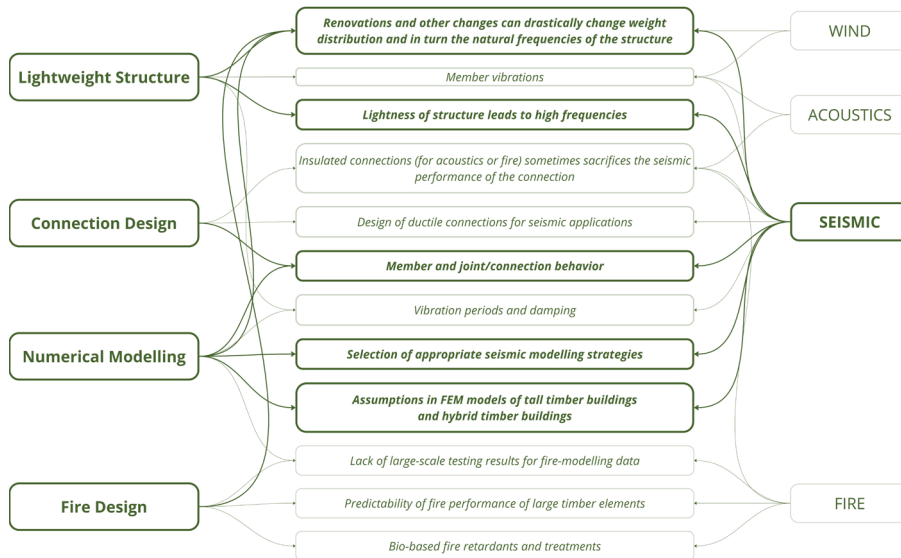
(of tall timber structures)

Material Challenges

- Anisotropic material
 - behaves differently in each direction
- Natural material
 - subject to deterioration

Tall Structure Challenges

- Relatively lightweight
- Properties can change through the structure's lifetime



Why Seismic Analysis Workflow?

(of tall timber structures)

Current Modelling Strategies

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- Complex to implement
 - More accurate models are time-consuming for practice



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- Disconnected from the main design workflow



Why Seismic Analysis Workflow?

(of tall timber structures)

Current Modelling Strategies

- Complex to implement
 - More accurate models are time-consuming for practice
- Disconnected from the main design workflow
- Does not consider changing parameters overtime



Research Goals

Problem Statement

Current **practice-oriented seismic analysis** methods for *tall timber structures* are **isolated** from the main design process, **complex to implement**, and do not consider **variability of parameters** over the structure's lifetime.

Objective

How will this problem be addressed by the research project?

Develop a **practice-oriented seismic analysis** workflow for *tall timber structures* which is **integrated** into the main design process, is **quick** and **adaptable** to implement, and includes **lifetime analysis** options.

Questions

What is the existing practice and knowledge for the seismic analysis of tall timber structures?

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How can this knowledge be applied in a practice-oriented workflow for seismic analysis?

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**What is the existing practice and knowledge for structure?
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How can a lifetime analysis be incorporated into the workflow?

What is the accuracy, applicability, *What is the impact of the lifetime analysis?*

and advantage of this workflow? *How does the workflow impact the decision making process for the engineer?*

Project Scope

Project Scope

Form and Function



Rectilinear



8+ stories



Residential

Project Scope

Form and Function



Rectilinear

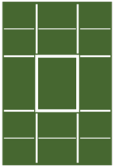


8+ stories

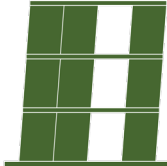


Residential

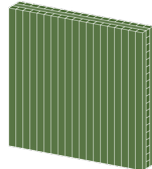
Structural System



Central Core



Shear Walls



CLT Walls

Project Scope

Form and Function



Rectilinear



8+ stories



Residential



Europe Region

Building Standard

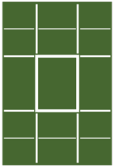


EN 1995

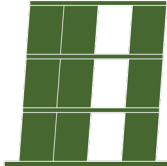


EN 1998

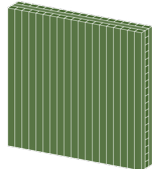
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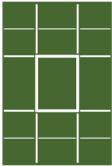


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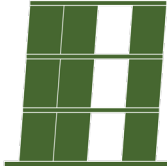


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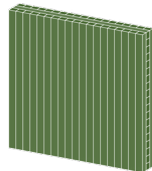
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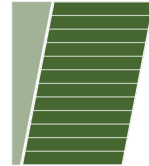
Central Core



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CLT Walls



Static Linear Analysis

Analysis

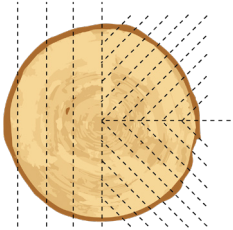


Post Event



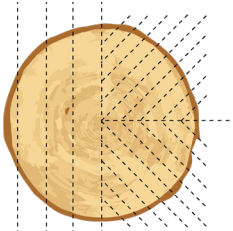
Over Time

Cross Laminated Timber (CLT)



Planing or Quartering

Cross Laminated Timber (CLT)

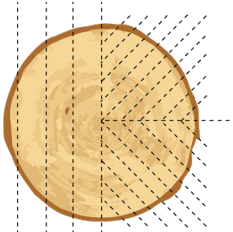


Planing or Quartering



Solid Sawn Lumber

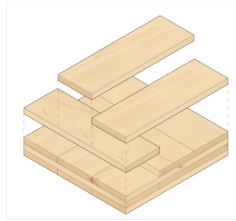
Cross Laminated Timber (CLT)



Planing or Quartering

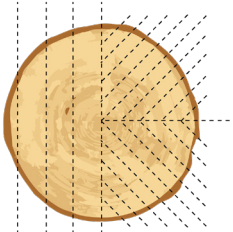


Solid Sawn Lumber



Cross-Laminated Timber

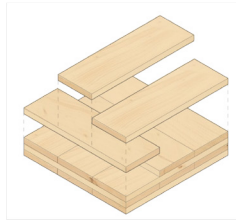
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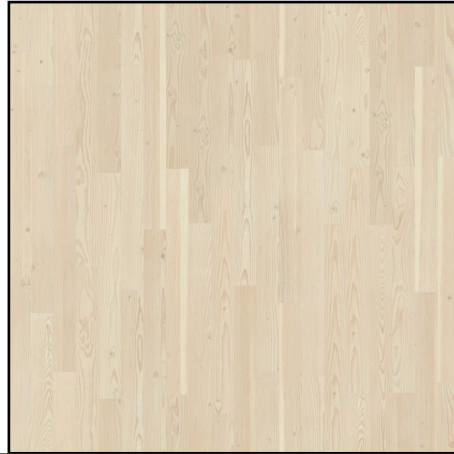
Cross-Laminated Timber



CLT Wall Construction

CLT Panels

- Single- or



CLT Wall Construction

CLT Panels

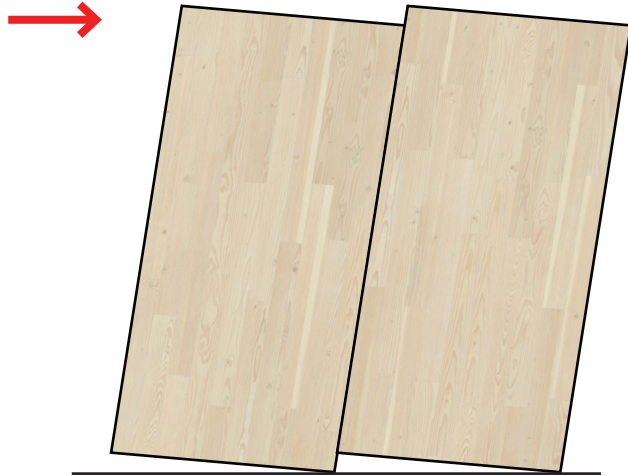
- Single- or multi-panel



CLT Wall Construction

CLT Panels

- Single- or multi-panel
- Work together to resist lateral loads

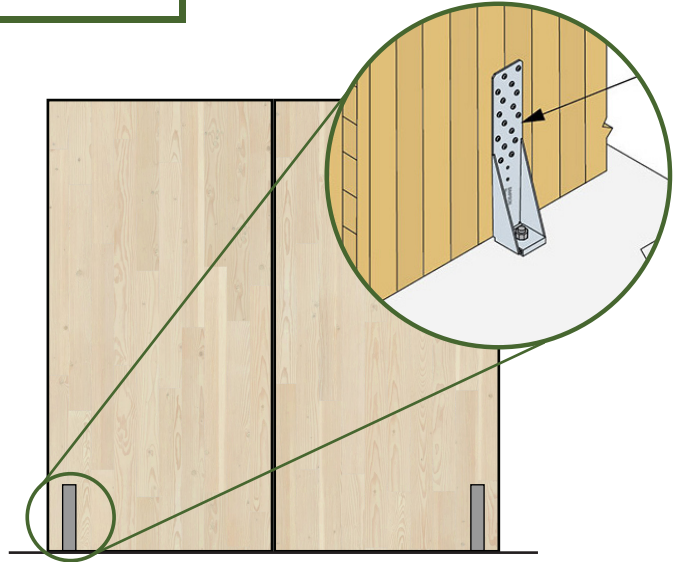


CLT Wall Construction

CLT Panels

- Single- or multi-panel
 - Work together to resist lateral loads

Hold downs



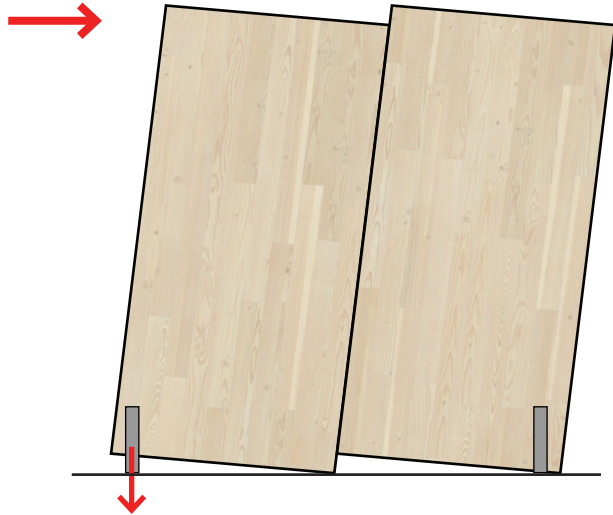
CLT Wall Construction

CLT Panels

- Single- or multi-panel
 - Work together to resist lateral loads

Hold downs

- Resist tension from overturning



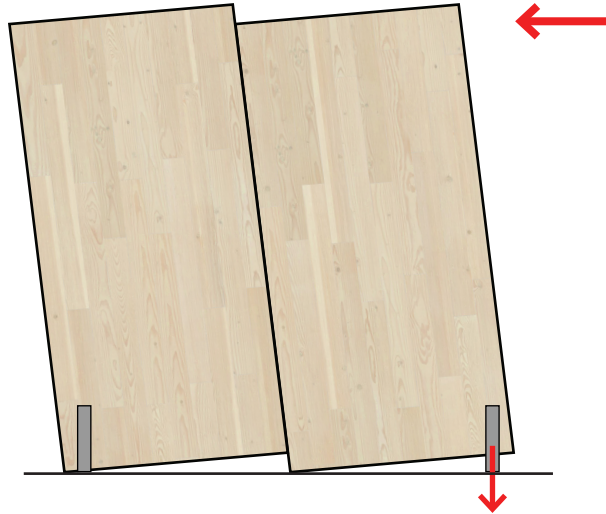
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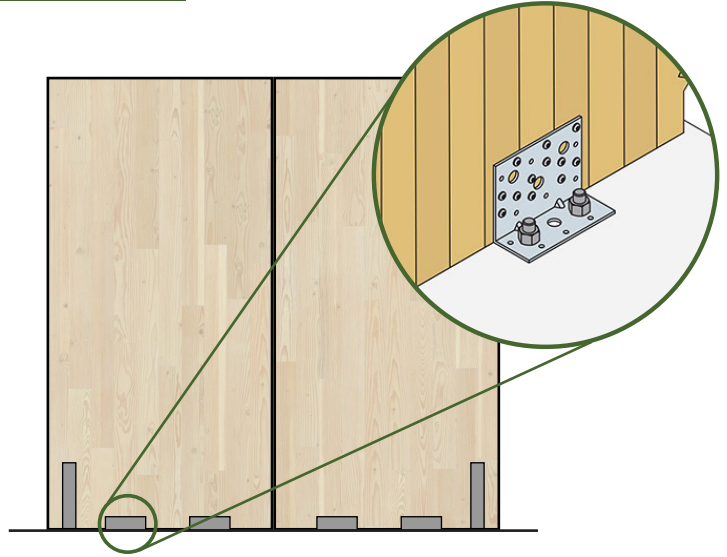
CLT Panels

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- Resist tension from overturning

Angle brackets



CLT Wall Construction

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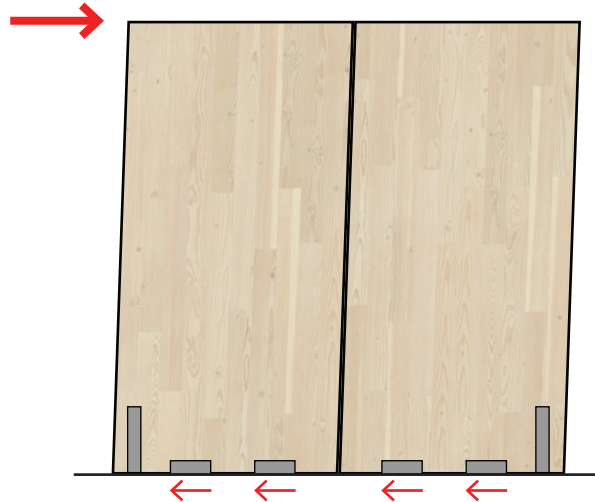
- Single- or multi-panel
 - Work together to resist lateral loads

Hold downs

- Resist tension from overturning

Angle brackets

- Resist shear forces at base



CLT Wall Construction

CLT Panels

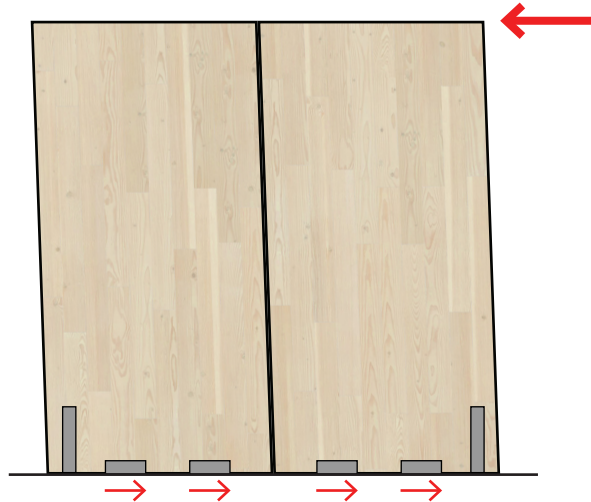
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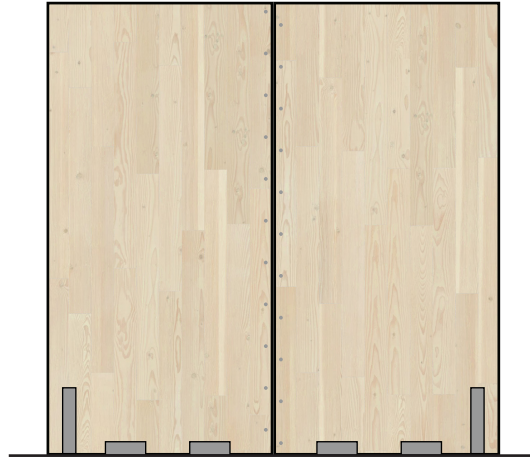
Hold downs

- Resist tension from overturning

Angle brackets

- Resist shear forces at base

Screws in joint



CLT Wall Construction

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 - Work together to resist lateral loads

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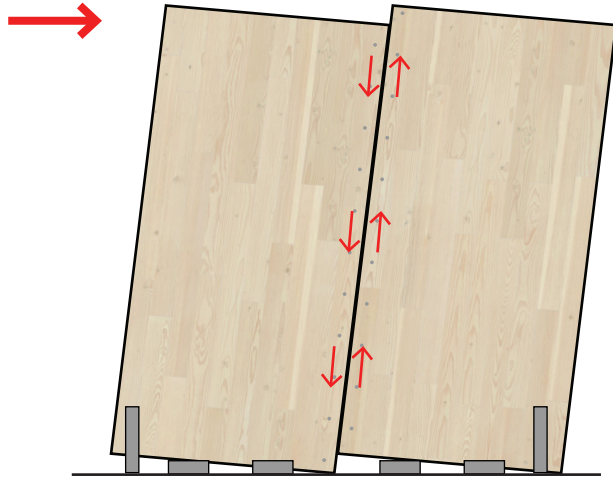
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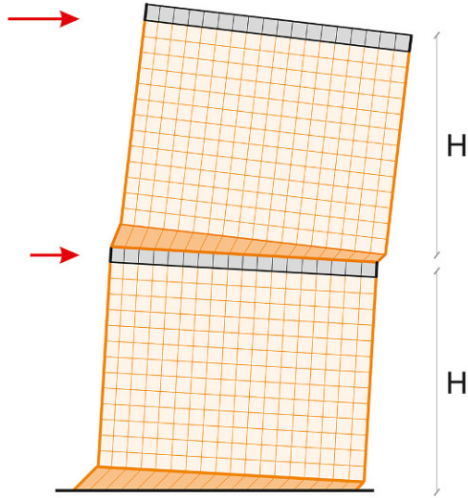
- Resist shear forces at base

Screws in joint

- Resist sliding forces between panels

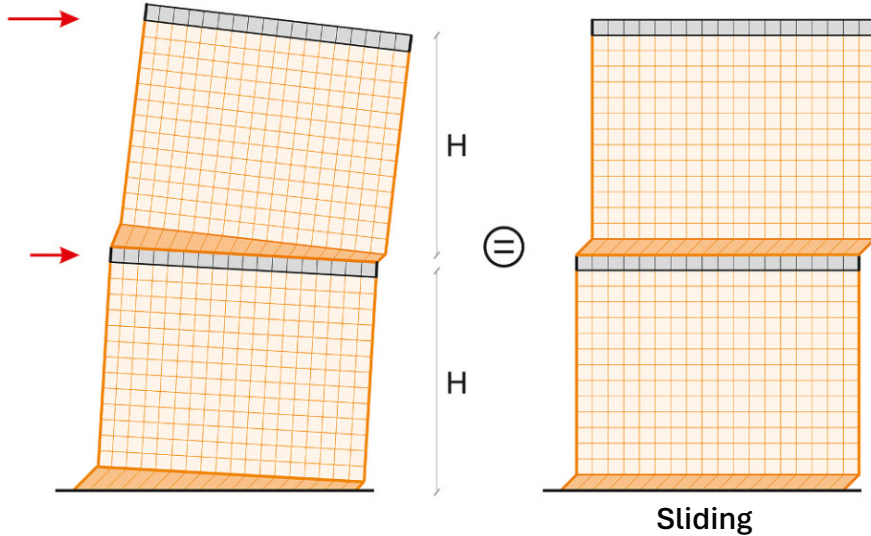


Deformation Behavior



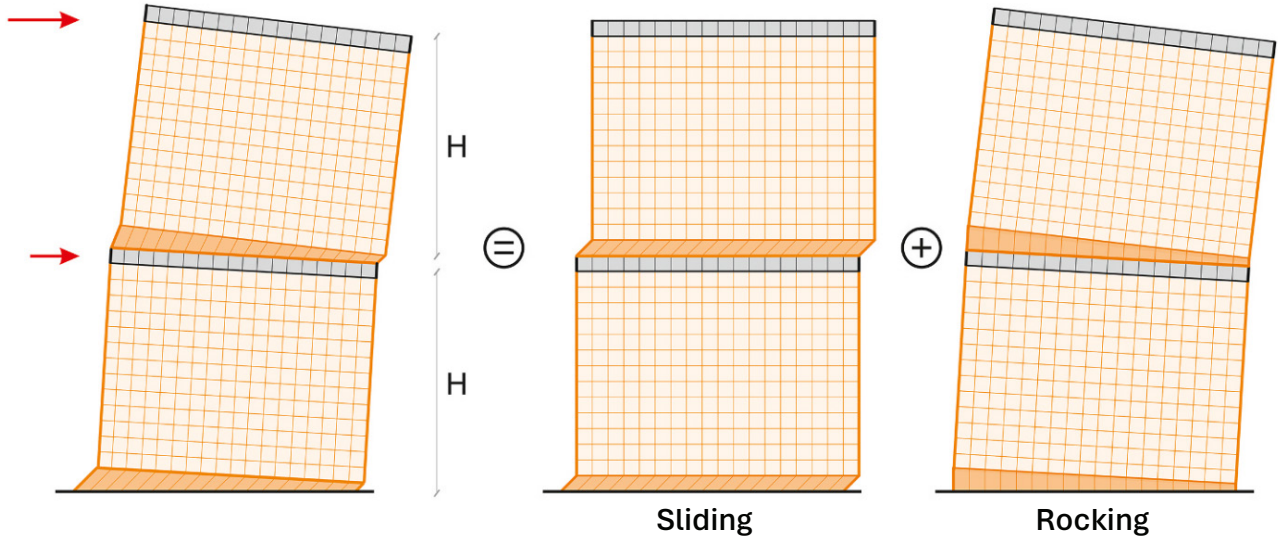
Source: Upgraded Model, Rinaldi et al. 2021

Deformation Behavior



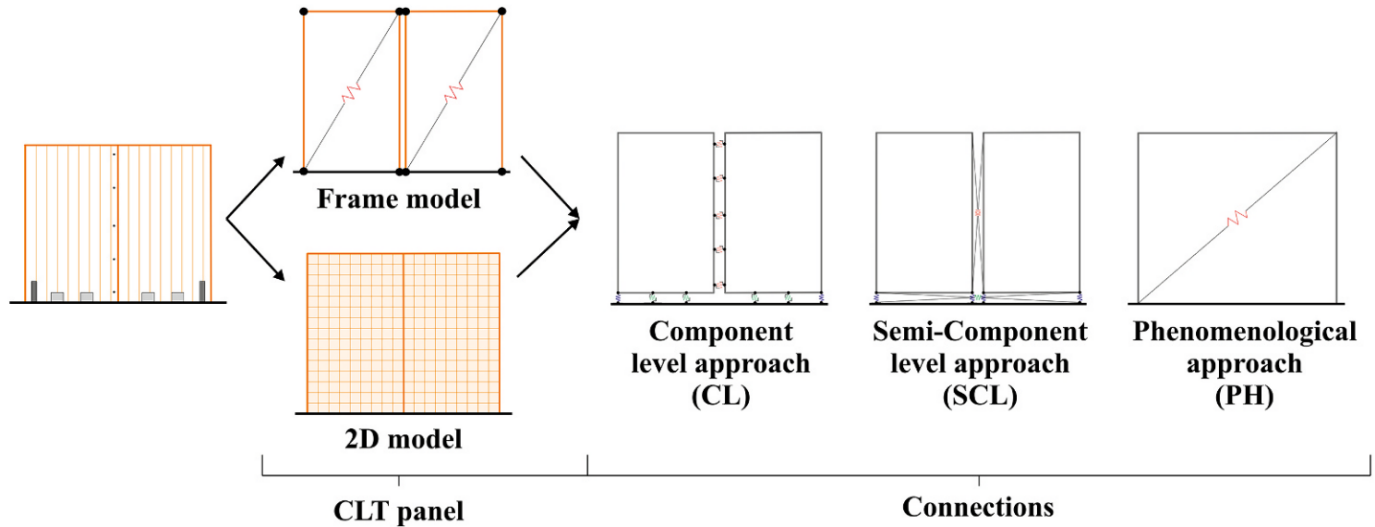
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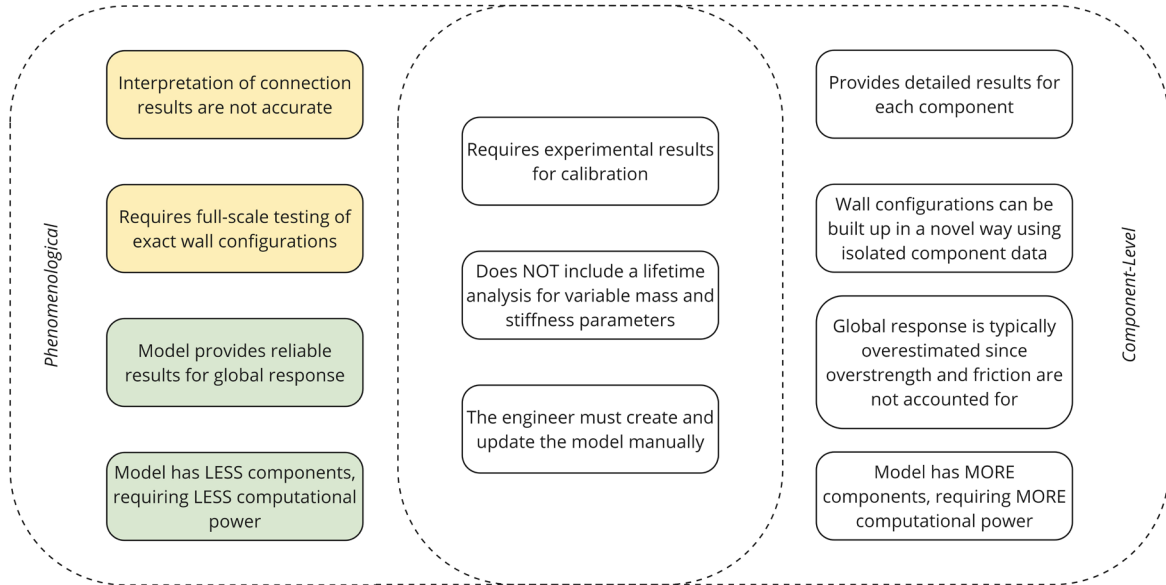
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Modelling Strategies

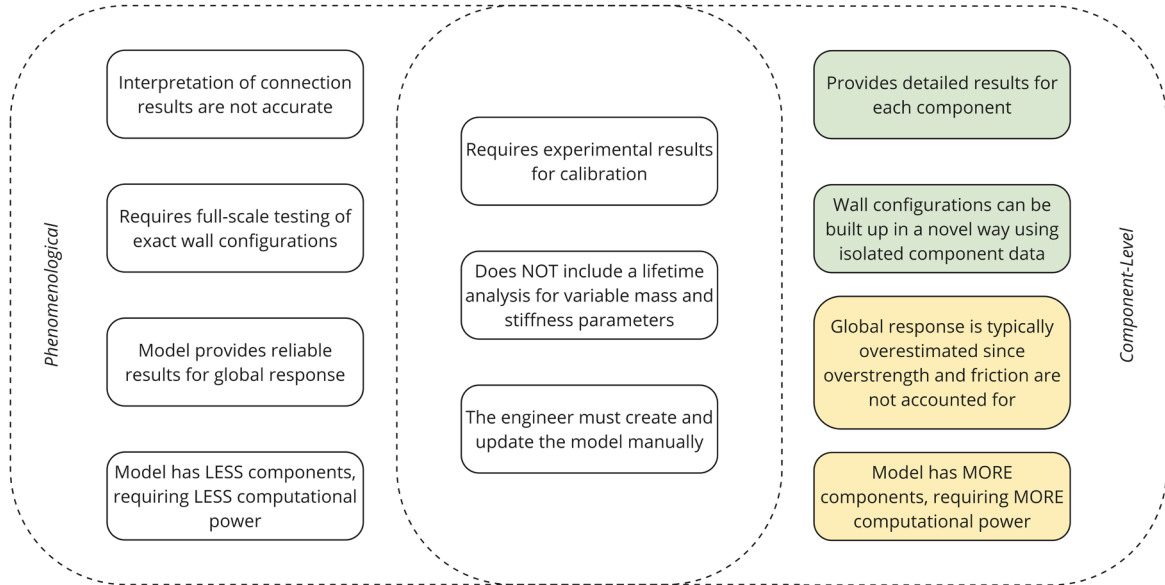


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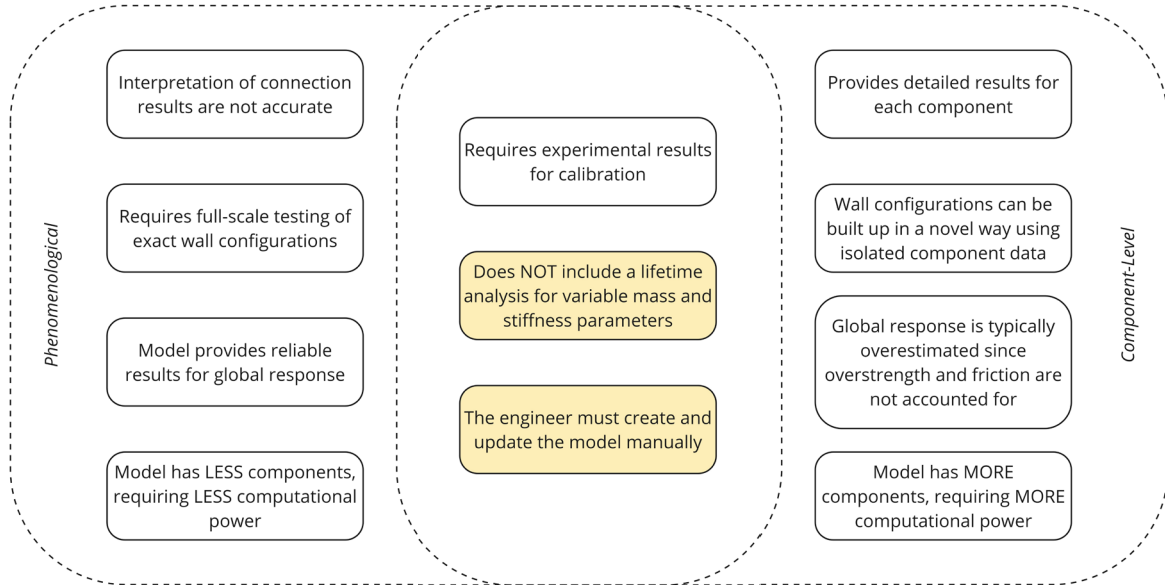
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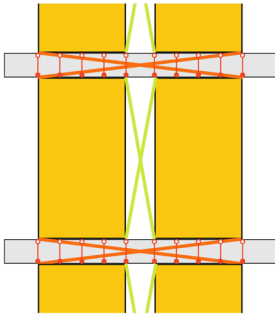


Modelling Strategies



Modelling Strategies

Follesa et al. 2013



Truss and links for wall-to-floor joints

Truss for wall-to-wall joints

Floor stiffness modelled with truss

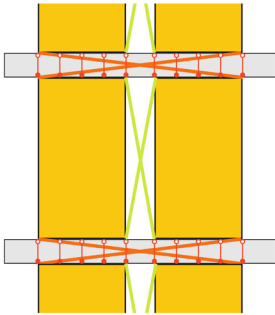
Rigid CLT panels

Includes friction in angle brackets

Neglects rocking

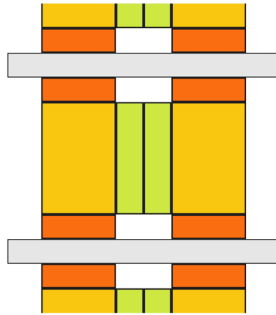
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Follesa et al. 2013



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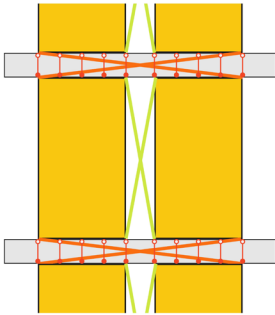
Christovasilis et al. 2020



Horizontal strips for wall-floor joints
Vertical strips for wall-wall joints
Floor stiffness modelled as 2D strip
Rigid CLT panels
Neglects friction
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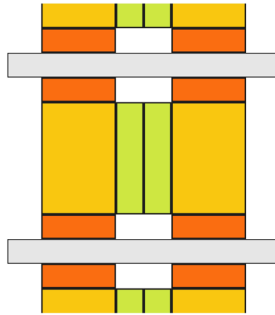
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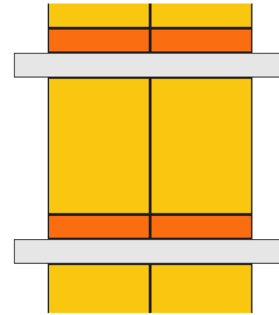
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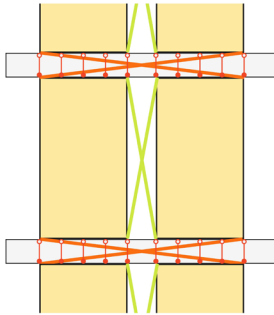
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Wall-wall joints in equivalent stiffness
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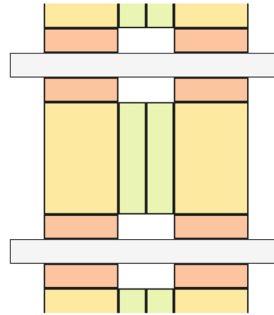
Modelling Strategies

Follesa et al. 2013



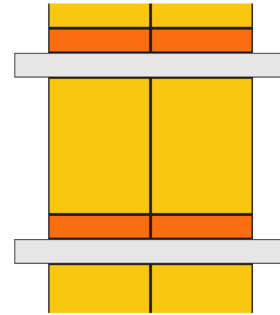
Truss and links for wall-to-floor joints
Truss for wall-to-wall joints
Floor stiffness modelled with truss
Rigid CLT panels
Includes friction in angle brackets
Neglects rocking

Christovasilis et al. 2020



Horizontal strips for wall-floor joints
Vertical strips for wall-wall joints
Floor stiffness modelled as 2D strip
Rigid CLT panels
Neglects friction
Neglects rocking

Rinaldi et al. 2021



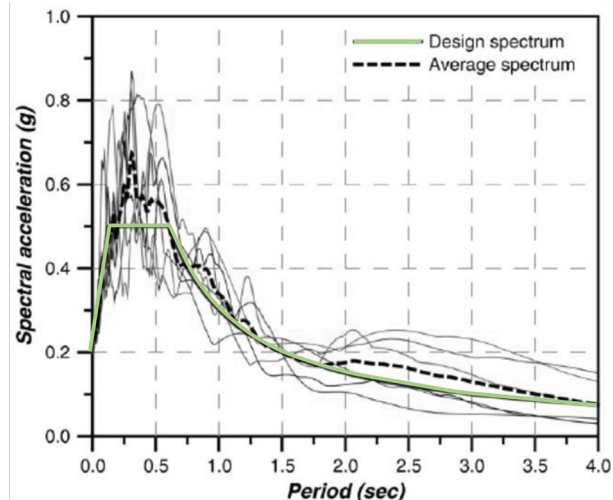
Horizontal strips for wall-floor joints
Wall-wall joints in equivalent stiffness
Floor stiffness modelled as 2D strip
Rigid CLT panels
Neglects friction
Includes rocking in equivalent stiffness

Full Structure Analysis

What is required for full structure analysis?

Seismic Parameters

- Spectral acceleration

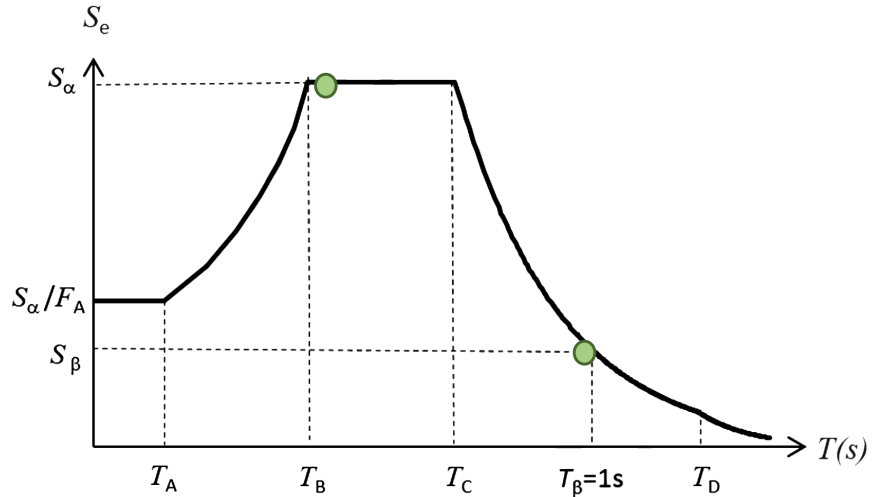


Full Structure Analysis

What is required for full structure analysis?

Seismic Parameters

- Spectral acceleration



Full Structure Analysis

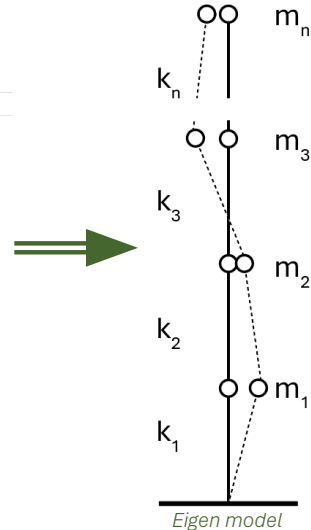
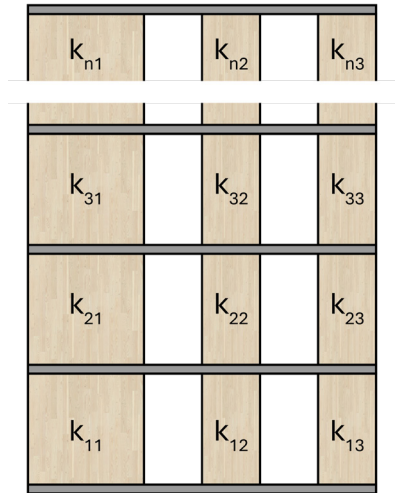
What is required for full structure analysis?

Seismic Parameters

- Spectral acceleration

Structural Parameters

- Mass and stiffness distribution



Full Structure Analysis

What is required for full structure analysis?

Seismic Parameters

- Spectral acceleration

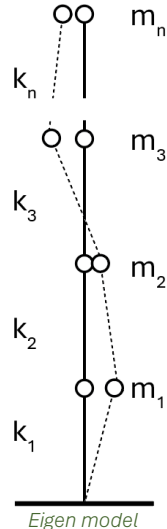
Structural Parameters

- Mass and stiffness distribution
- Building period

$$\omega = \sqrt{\frac{m}{k}} = \text{eigen vector}$$

$$T = 2\pi\omega$$

Relationship b/t eigen analysis and building period



Full Structure Analysis

What is required for full structure analysis?

Seismic Parameters

- Spectral acceleration

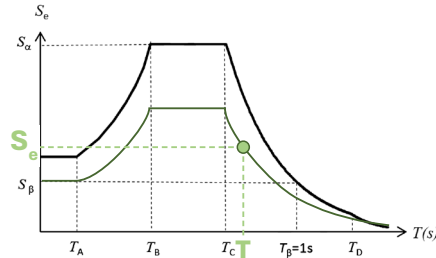
Structural Parameters

- Mass and stiffness distribution
- Building period
- Seismic coefficient

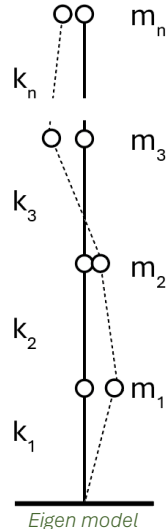
$$\omega = \sqrt{\frac{m}{k}} = \text{eigen vector}$$

$$T = 2\pi\omega$$

Relationship b/t eigen analysis and building period



Determining the seismic coefficient from the design spectra and the building period



Full Structure Analysis

What is required for full structure analysis?

Seismic Parameters

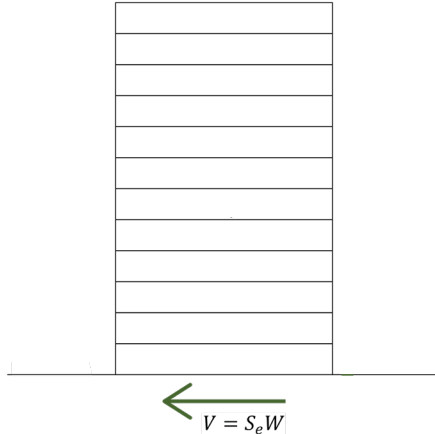
- Spectral acceleration

Structural Parameters

- Mass and stiffness distribution
- Building period
- Seismic coefficient

Force Distribution

- Base shear



Full Structure Analysis

What is required for full structure analysis?

Seismic Parameters

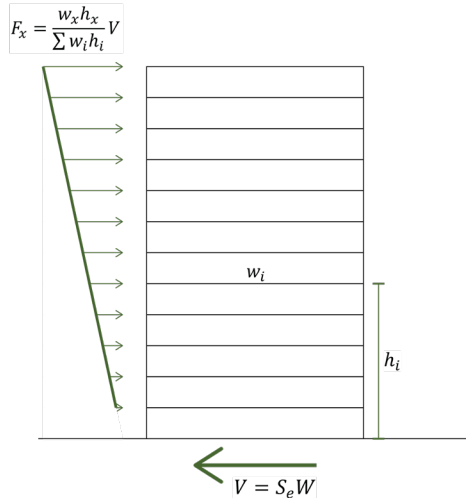
- Spectral acceleration

Structural Parameters

- Mass and stiffness distribution
- Building period
- Seismic coefficient

Force Distribution

- Base shear
- Distribute to floors based on story height and weight



Full Structure Analysis

What is required for full structure analysis?

Seismic Parameters

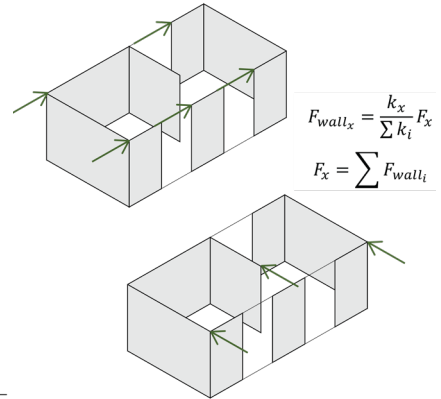
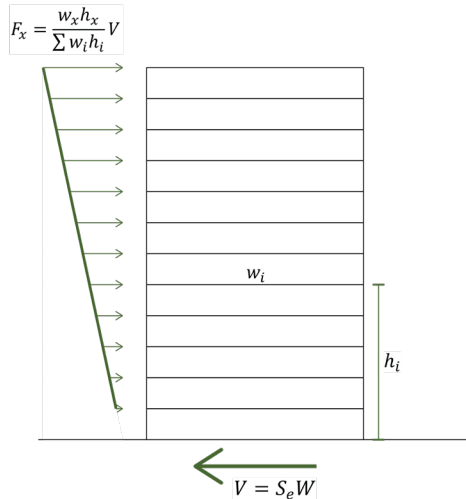
- Spectral acceleration

Structural Parameters

- Mass and stiffness distribution
- Building period
- Seismic coefficient

Force Distribution

- Base shear
- Distribute to floors based on story height and weight
- Distribute to walls based on effective stiffness



Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event



Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:



Source: SOFIE project, Ceccotti et al. 2013

Seven story test structure from the SOFIE project

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures



Seven story test structure from the SOFIE project



Nail failures at hold-downs

Source: SOFIE project, Ceccotti et al. 2013

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals



Seven story test structure from the SOFIE project



Nail failures at hold-downs



Nail withdrawal at angle brackets

Source: SOFIE project, Ceccotti et al. 2013

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing



Seven story test structure from the SOFIE project

Source: SOFIE project, Ceccotti et al. 2013



Nail failures at hold-downs



Nail withdrawal at angle brackets



Wood crushing under hold-downs

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing

Future Renovation



Lifetime Analysis

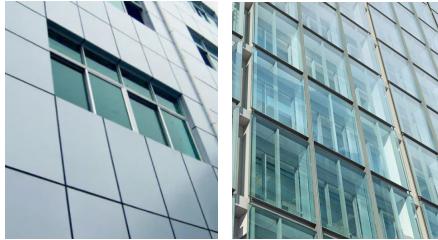
How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing

Future Renovation

- Mass distribution



Facade renovation

Lifetime Analysis

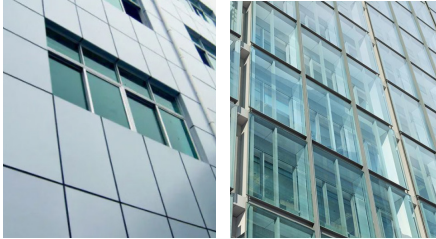
How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing

Future Renovation

- Mass distribution



Facade renovation



Change in occupancy

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing

Future Renovation

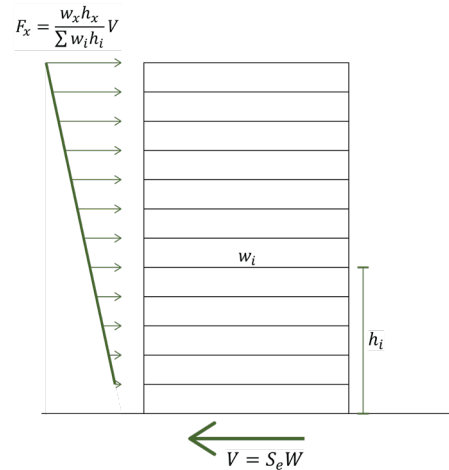
- Mass distribution



Facade renovation



Change in occupancy



Change in mass and forces

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing

Future Renovation

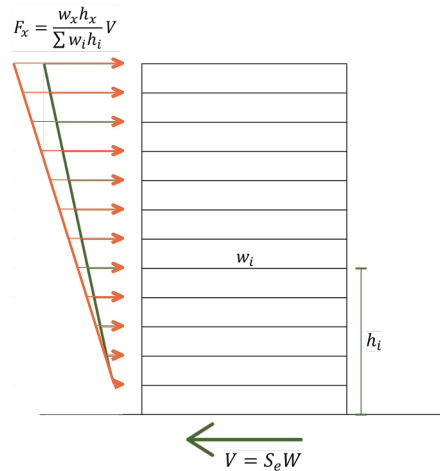
- Mass distribution



Facade renovation



Change in occupancy



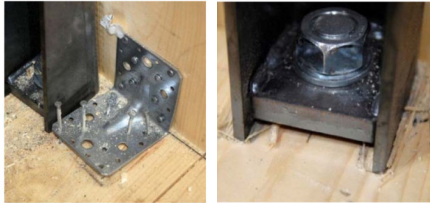
Change in mass and forces

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing



Replace old connections

Future Renovation

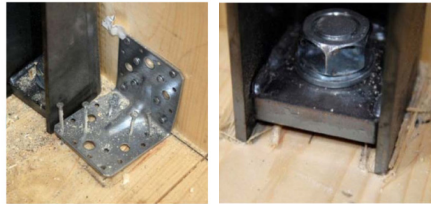
- Mass distribution
- Stiffness change

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

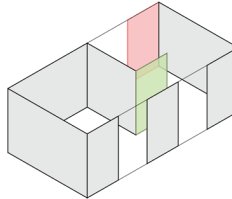
- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing



Replace old connections

Future Renovation

- Mass distribution
- Stiffness change



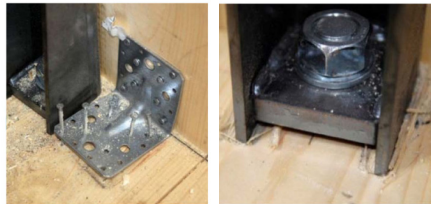
Add, remove, or replace walls

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

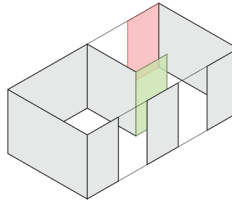
- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing



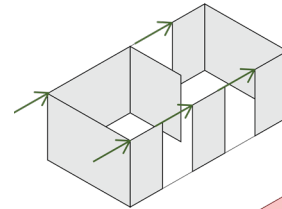
Replace old connections

Future Renovation

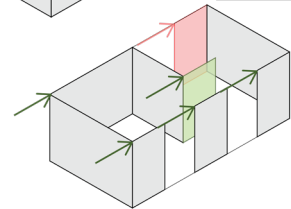
- Mass distribution
- Stiffness change



Add, remove, or replace walls



$$F_{wall_x} = \frac{k_x}{\sum k_i} F_x$$
$$F_x = \sum F_{wall_i}$$



Change in stiffness distribution

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing

Future Renovation

- Mass distribution
- Stiffness change

Biological Degradation



Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing



Deterioration causes loss in mass and stiffness

Future Renovation

- Mass distribution
- Stiffness change

Biological Degradation

- Stiffness and mass loss in CLT

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing



Deterioration causes loss in mass and stiffness

Future Renovation

- Mass distribution
- Stiffness change

Biological Degradation

- Stiffness and mass loss in CLT



Related to moisture exposure and time

Lifetime Analysis

How can these events or time variables impact seismic performance?

Previous Seismic Event

- Stiffness reduction from:
 - Fastener failures
 - Fastener withdrawals
 - Wood crushing

Future Renovation

- Mass distribution
- Stiffness change

Biological Degradation

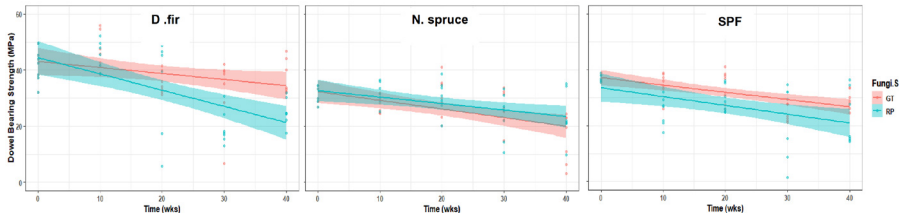
- Stiffness and mass loss in CLT
- Embedment strength of fasteners in connections



Deterioration causes loss in mass and stiffness



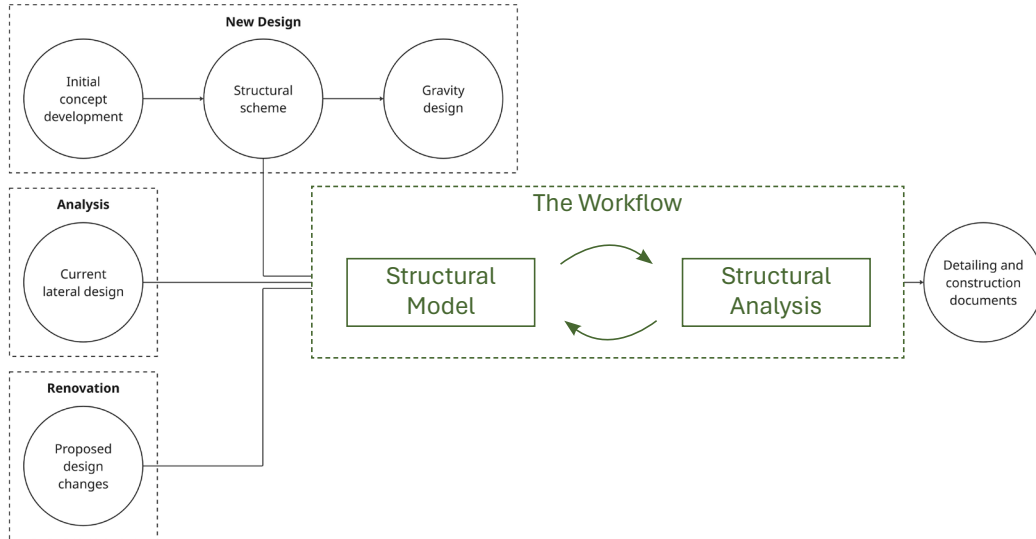
Related to moisture exposure and time



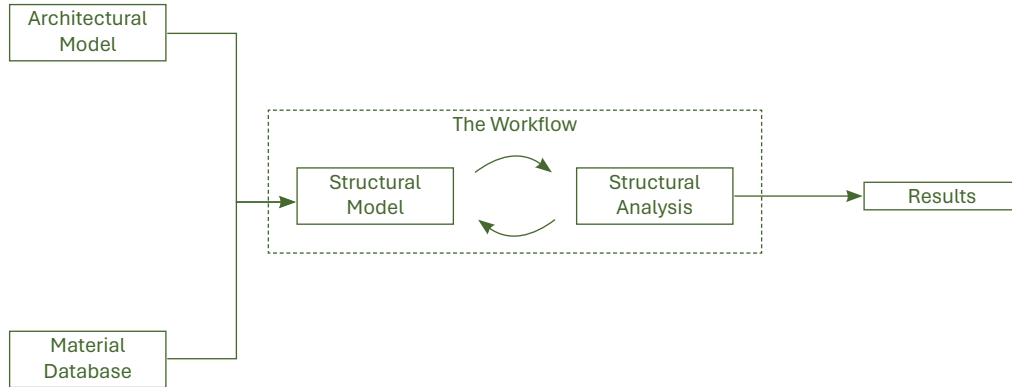
Linear regression for dowel bearing strength over time of fungal exposure for three species of wood: Douglas Fir, Norway Spruce, and Spruce Pine Fir (Source: Udele et al. 2024)

The Workflow

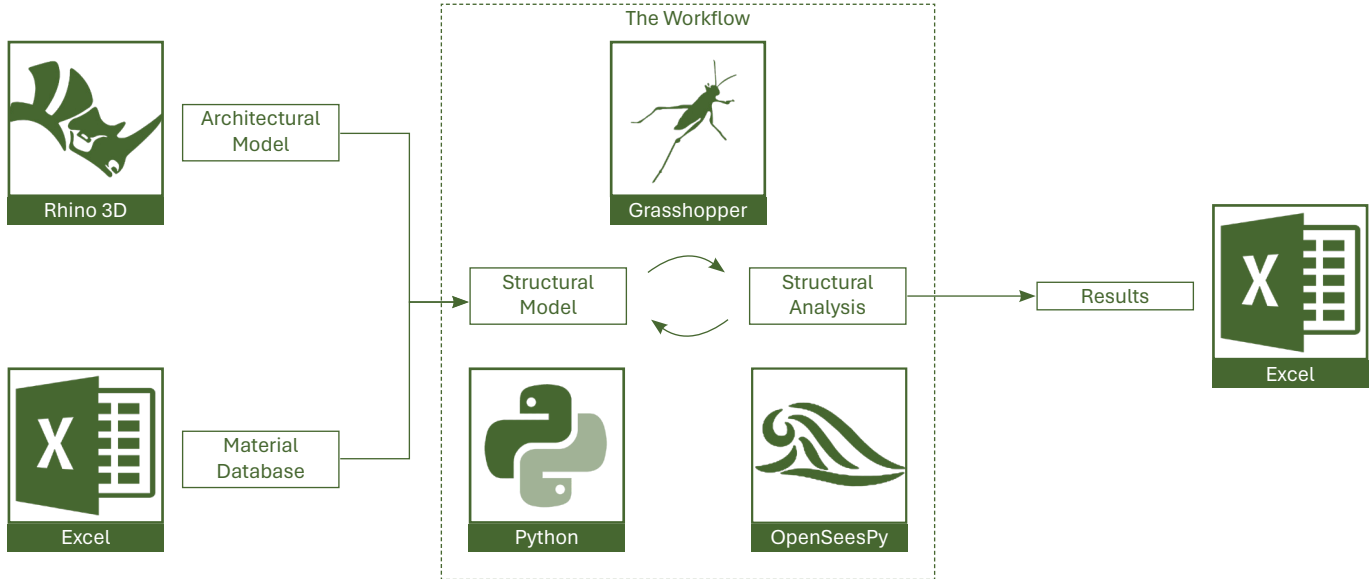
When to use the tool?



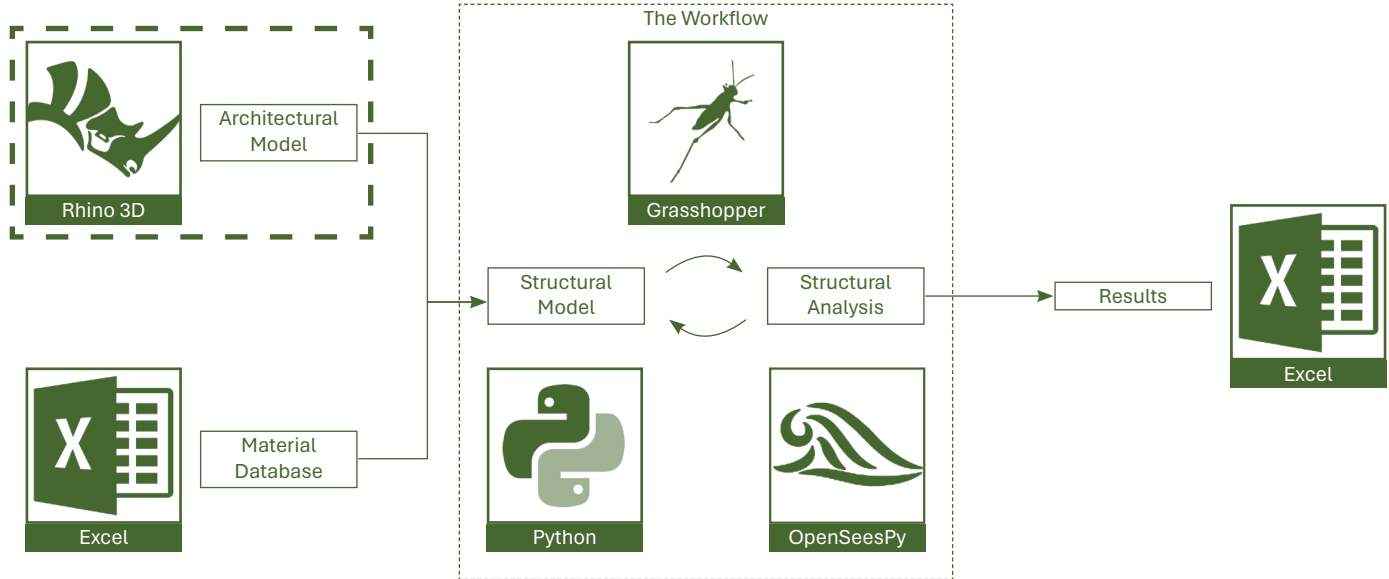
The Workflow



Programs and Workflow



Programs and Workflow

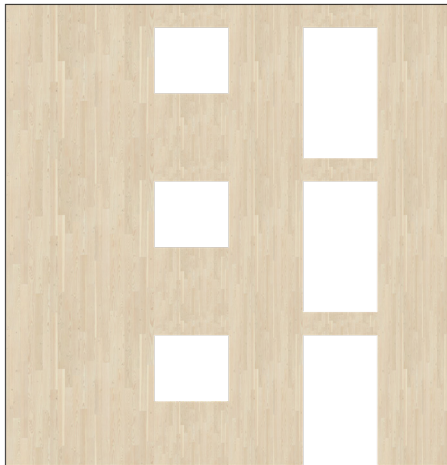


Model Generation



Create Geometry

Wall Geometry



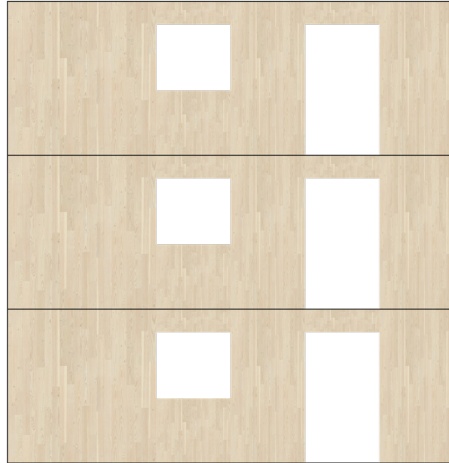
Model Generation



Create Geometry

Wall Geometry

- Split at each floor



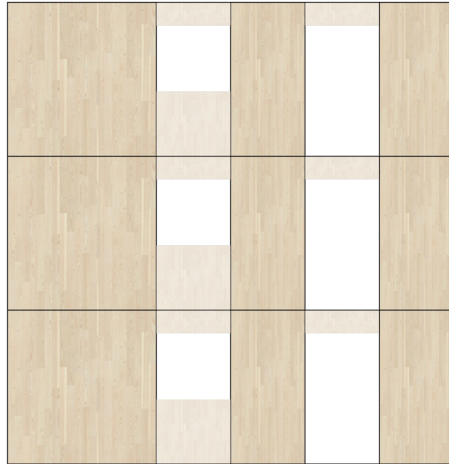
Model Generation



Create Geometry

Wall Geometry

- Split at each floor
- Ignore walls with openings



Model Generation

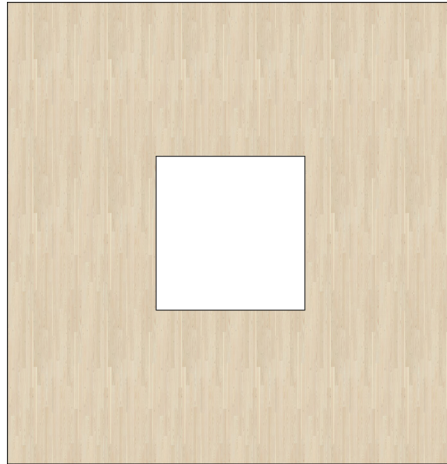


Create Geometry

Wall Geometry

- Split at each floor
- Ignore walls with openings

Floor Geometry



Model Generation



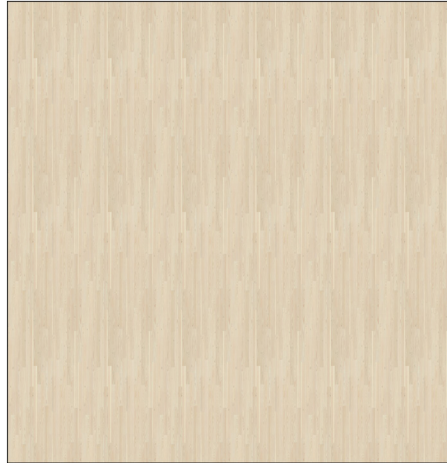
Create Geometry

Wall Geometry

- Split at each floor
- Ignore walls with openings

Floor Geometry

- Ignore shafts and openings



Model Generation



Create Geometry

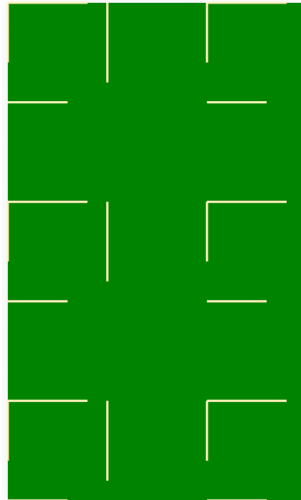
Wall Geometry

- Split at each floor
- Ignore walls with openings

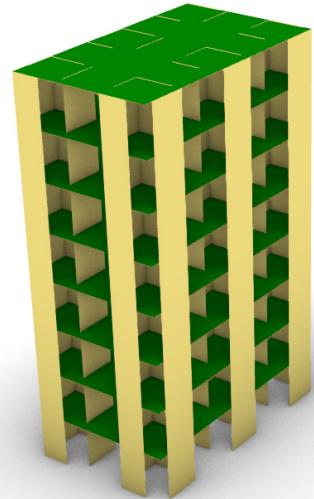
Floor Geometry

- Ignore shafts and openings

Modelled with Rectangle 3D

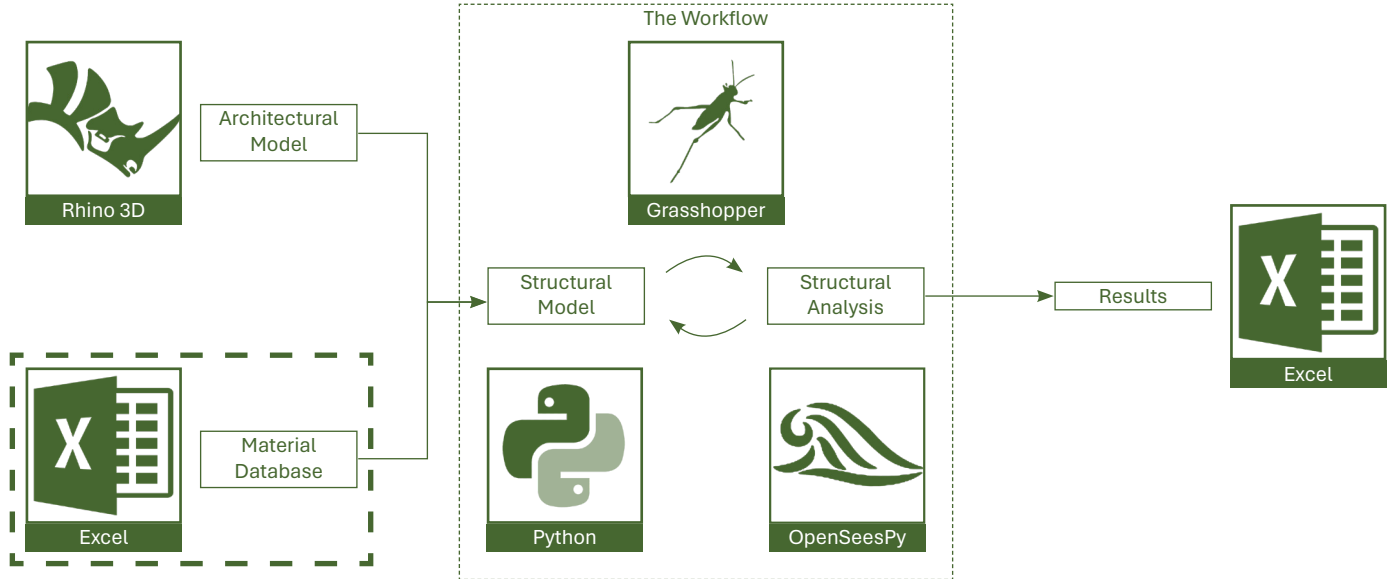


Plan view of example structure



3D model of example structure

Programs and Workflow



Materials Database



CLT properties

Raw Timber Properties

- Species



Materials Database



CLT properties

Raw Timber Properties

- Species
- Board thicknesses



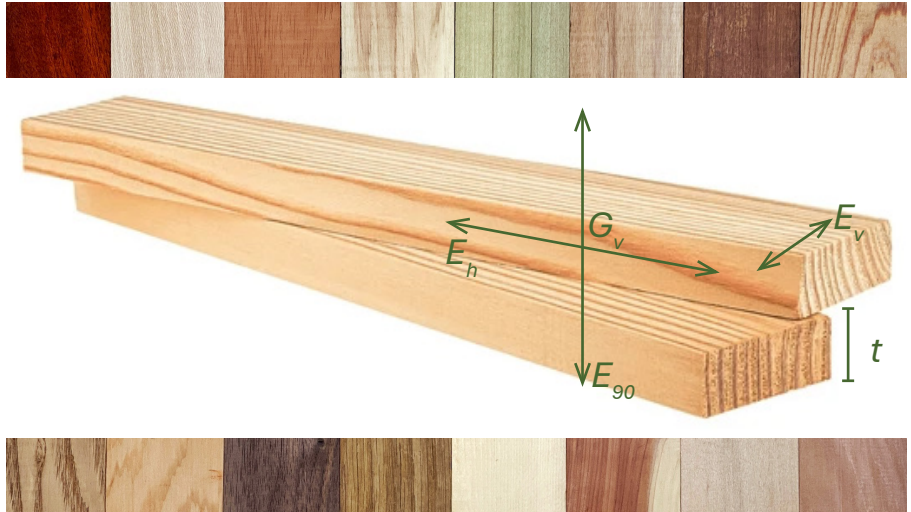
Materials Database



CLT properties

Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties



Materials Database



CLT properties

Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

Panel Properties



Materials Database



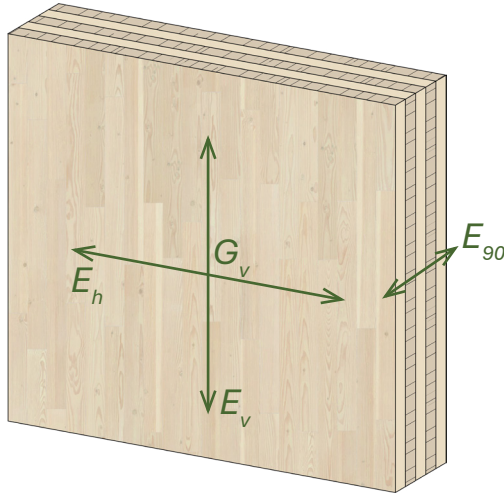
CLT properties

Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

Panel Properties

- Elastic and shear modulus



Materials Database



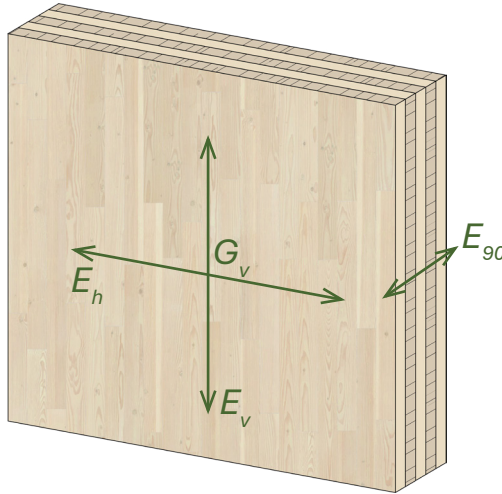
CLT properties

Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

Panel Properties

- Elastic and shear modulus
 - Calculated via research



*Elastic Moduli calculated via
Blass et al. 2004*

*Shear Modulus calculated via
Bogensperger et al. 2016*

Materials Database



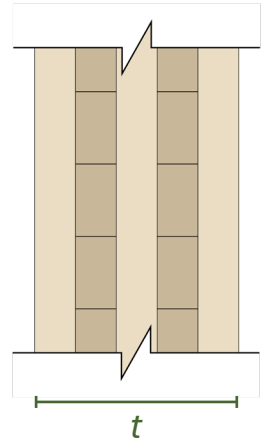
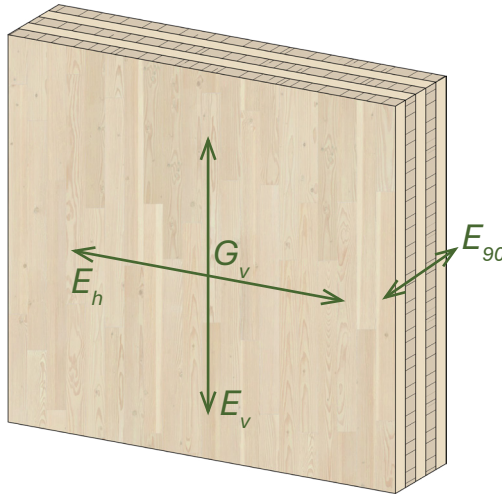
CLT properties

Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

Panel Properties

- Elastic and shear modulus
 - Calculated via research
- Thickness of panel



Materials Database



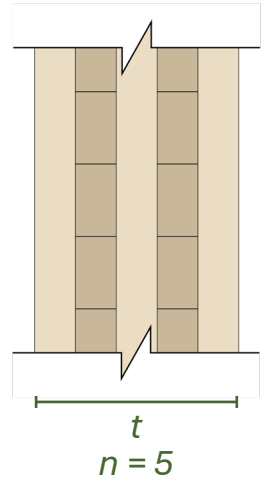
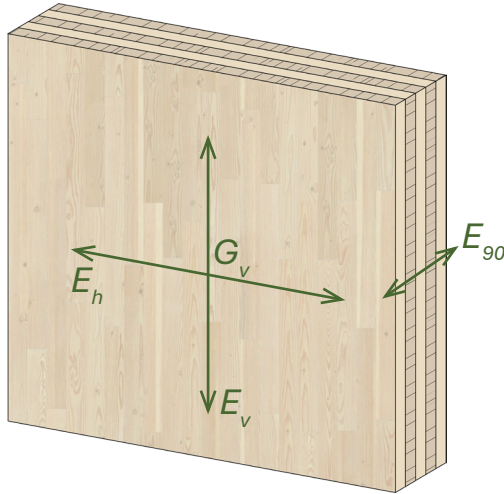
CLT properties

Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

Panel Properties

- Elastic and shear modulus
 - Calculated via research
- Thickness of panel
- Number of layers



Materials Database



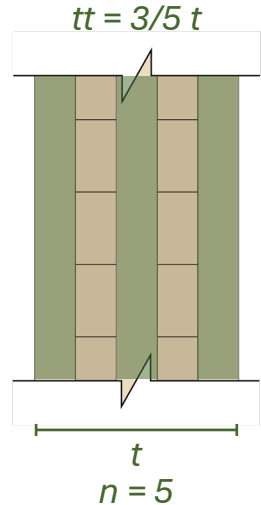
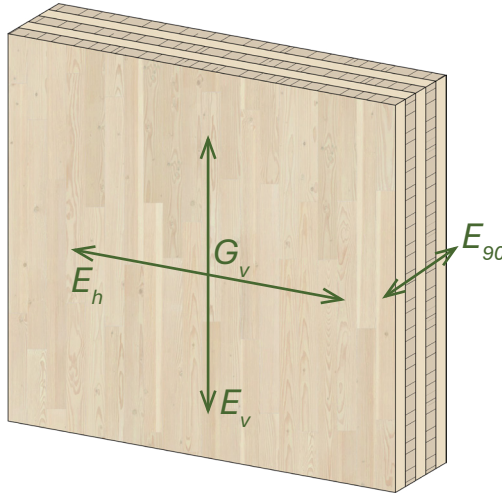
CLT properties

Raw Timber Properties

- Species
- Board thicknesses
- Raw timber properties

Panel Properties

- Elastic and shear modulus
 - Calculated via research
- Thickness of panel
- Number of layers
- Layer thickness ratio
 - Calculated via research



Materials Database



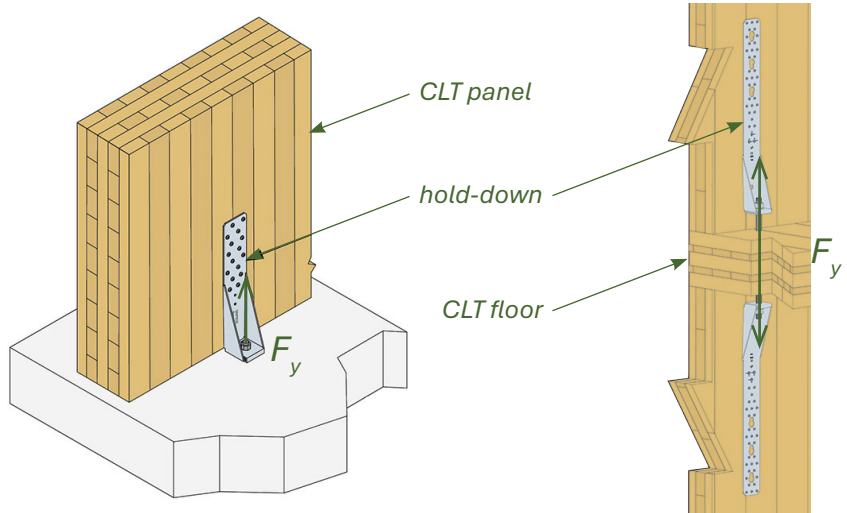
Hold Down properties

Connection Properties

- Yielding load and deflection
- Stiffness in both directions
- Plate thickness
- Overstrength

Fastener Properties

- Length
- Diameter
- Amount



Source: Simpson Strong Tie

Materials Database



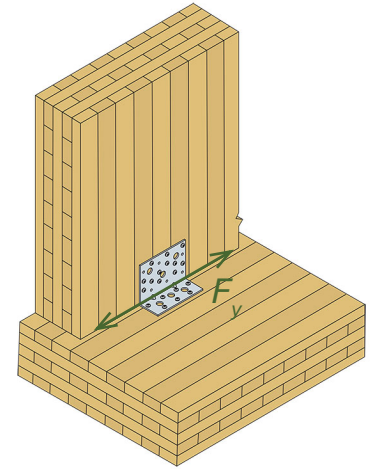
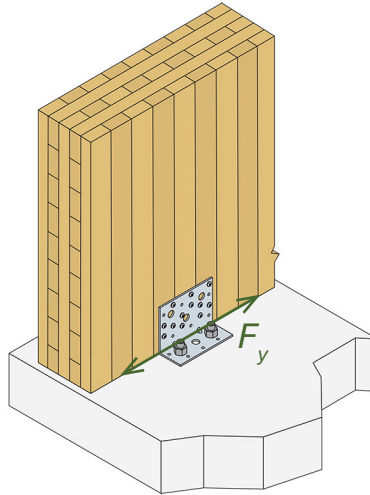
Angle Bracket properties

Connection Properties

- Yielding load and deflection
- Stiffness in both directions
- Plate thickness
- Overstrength

Fastener Properties

- Length
- Diameter
- Amount



Source: Simpson Strong Tie

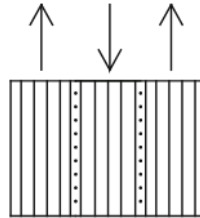
Materials Database



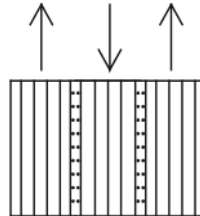
Screw Joint properties

Joint Connection Properties

- Joint type (splice or lap)
- Load direction (vertical in plane)
- Orientation (Parallel)
- Yield strength
- Stiffness



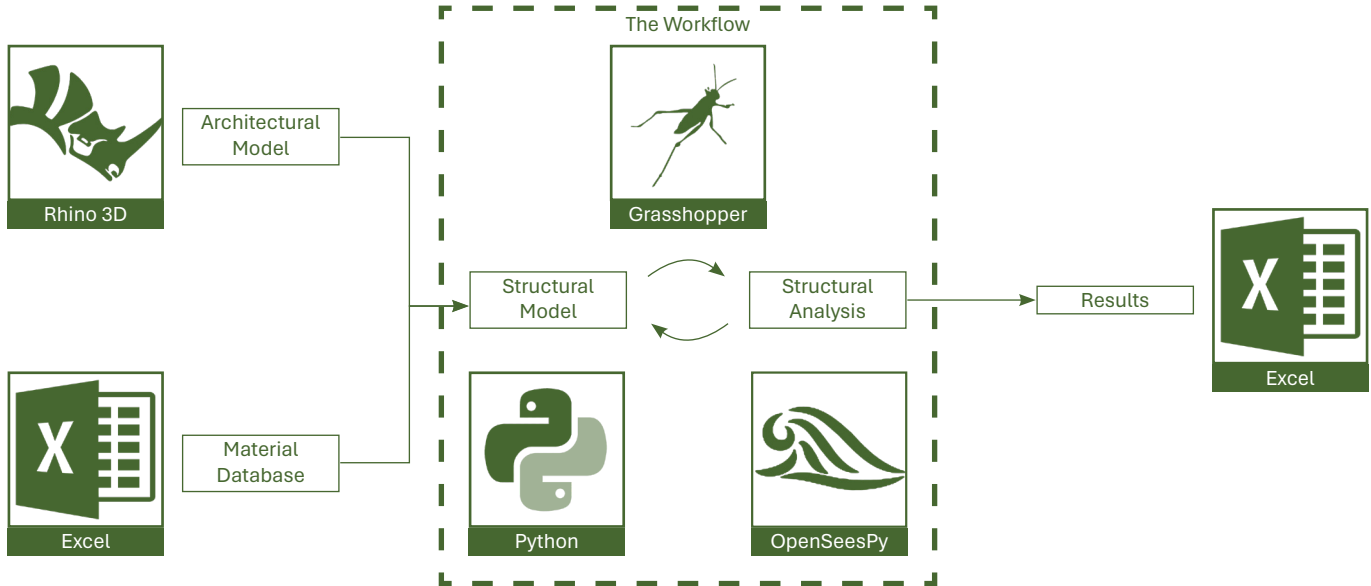
**Lateral
parallel
(lap joint)**



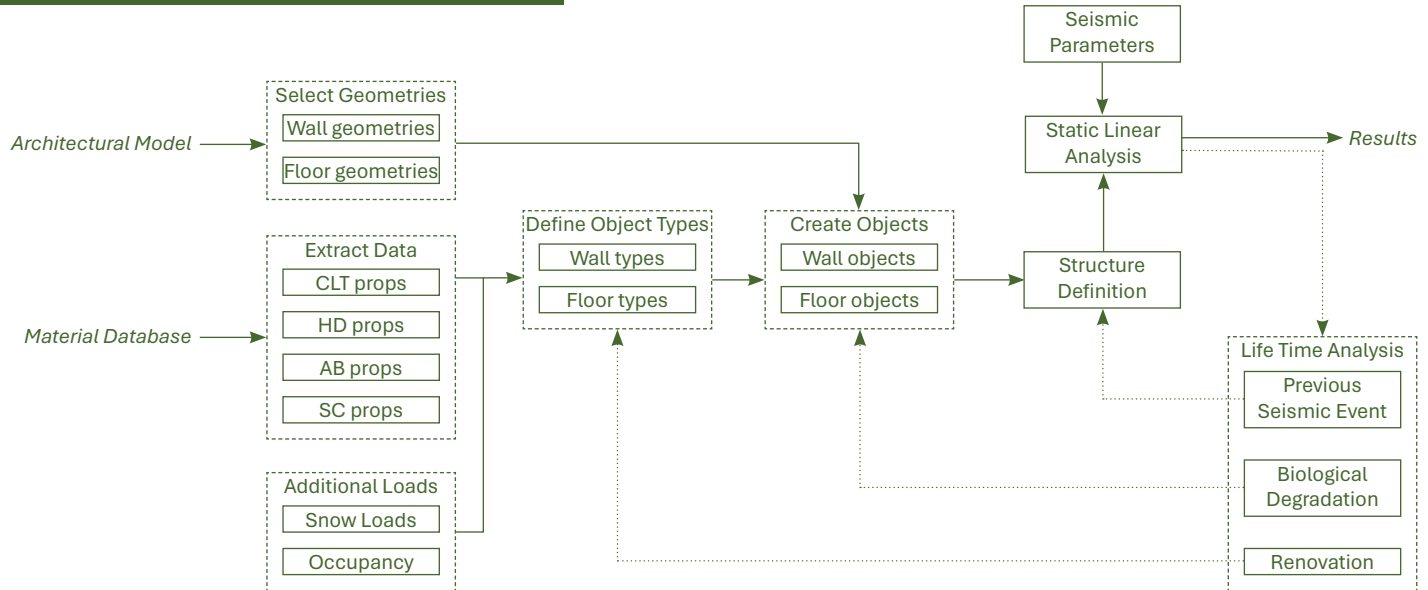
**Lateral
parallel
(spline joint)**

Source: Gavric et al. 2015

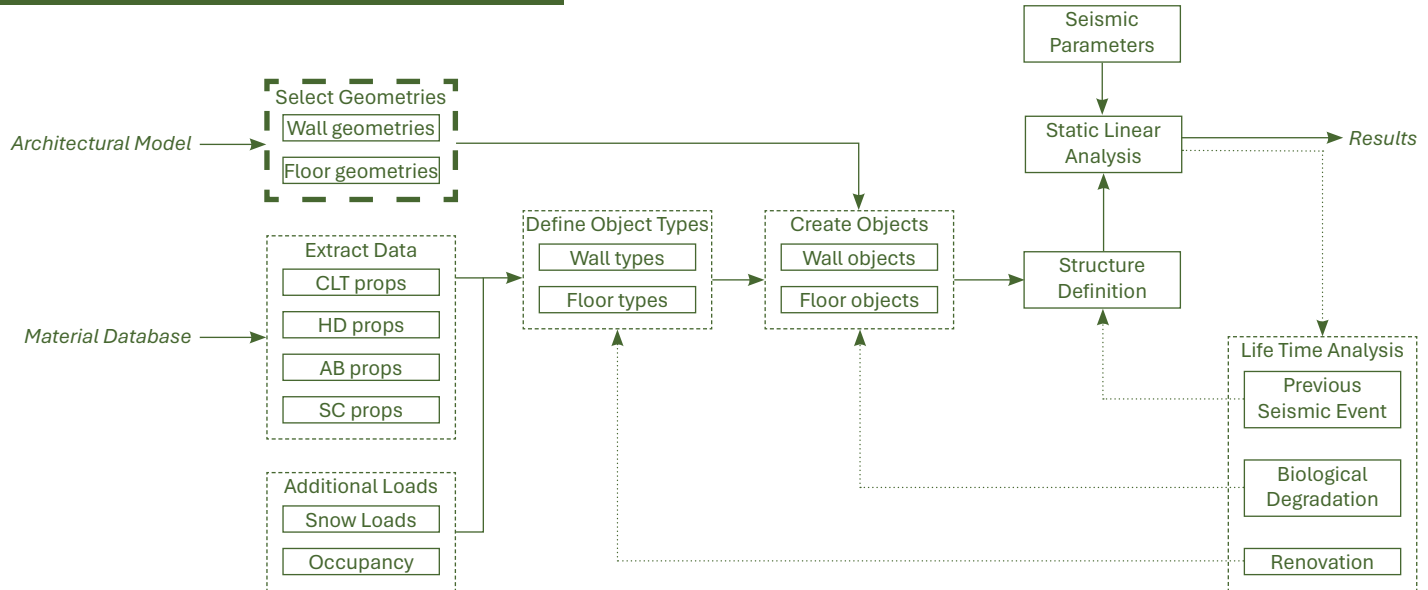
Programs and Workflow



The Workflow



The Workflow



Selecting Geometries



Organized by Layers

- Separate wall and floor layers



Walls

Floors

Selecting Geometries



Organized by Layers

- Separate wall and floor layers
- Separate by types into sublayers



Walls

Wall type 1

Wall type 2

Wall type 3

Wall type 4

Floors

Floor type 1

Floor type 2

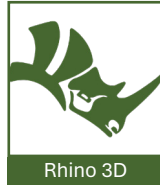
Selecting Geometries



Organized by Layers

- Separate wall and floor layers
- Separate by types into sublayers

Bring into Grasshopper



Walls

Wall type 1

Wall type 2

Wall type 3

Wall type 4

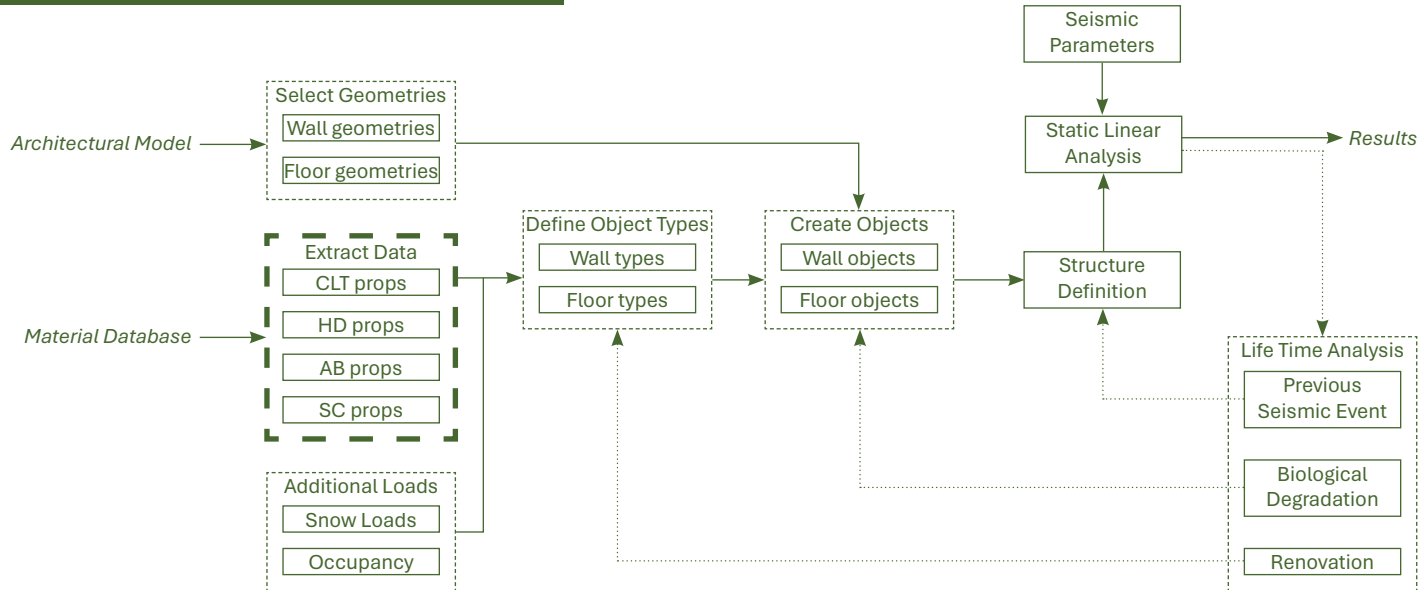
Floors

Floor type 1

Floor type 2



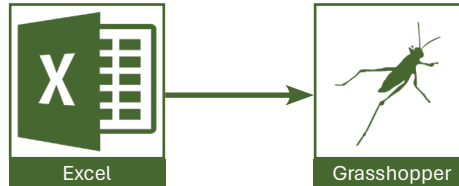
The Workflow



Model Generation

Import Material Data to Grasshopper

- Connect file path to materials database

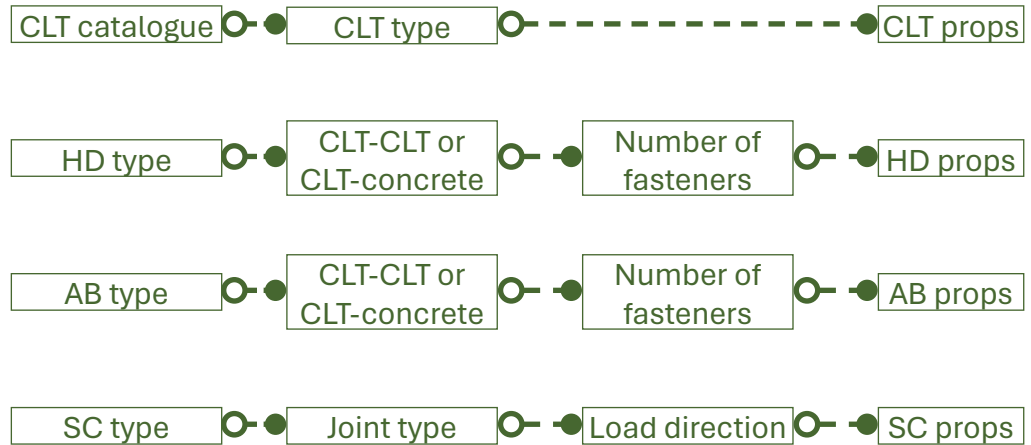


Model Generation



Import Material Data to Grasshopper

- Connect file path to materials database
- Select identifying properties to select CLT, HD, AB, or SC

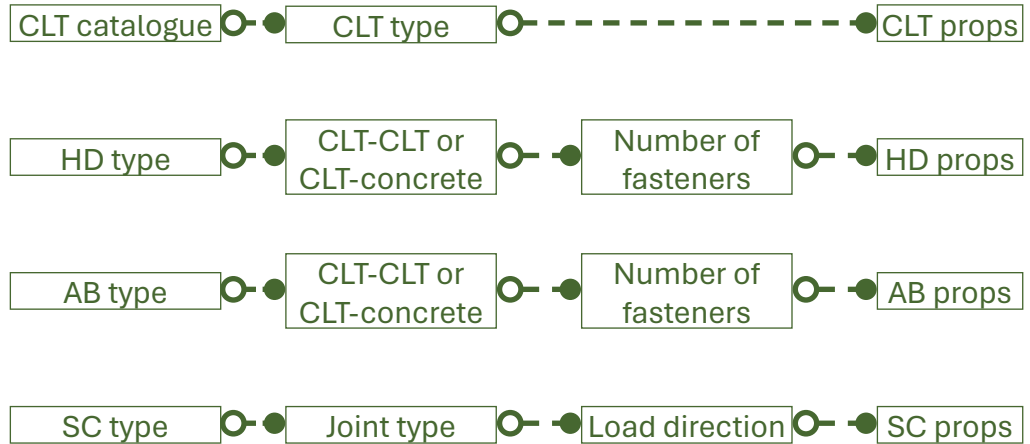


Model Generation

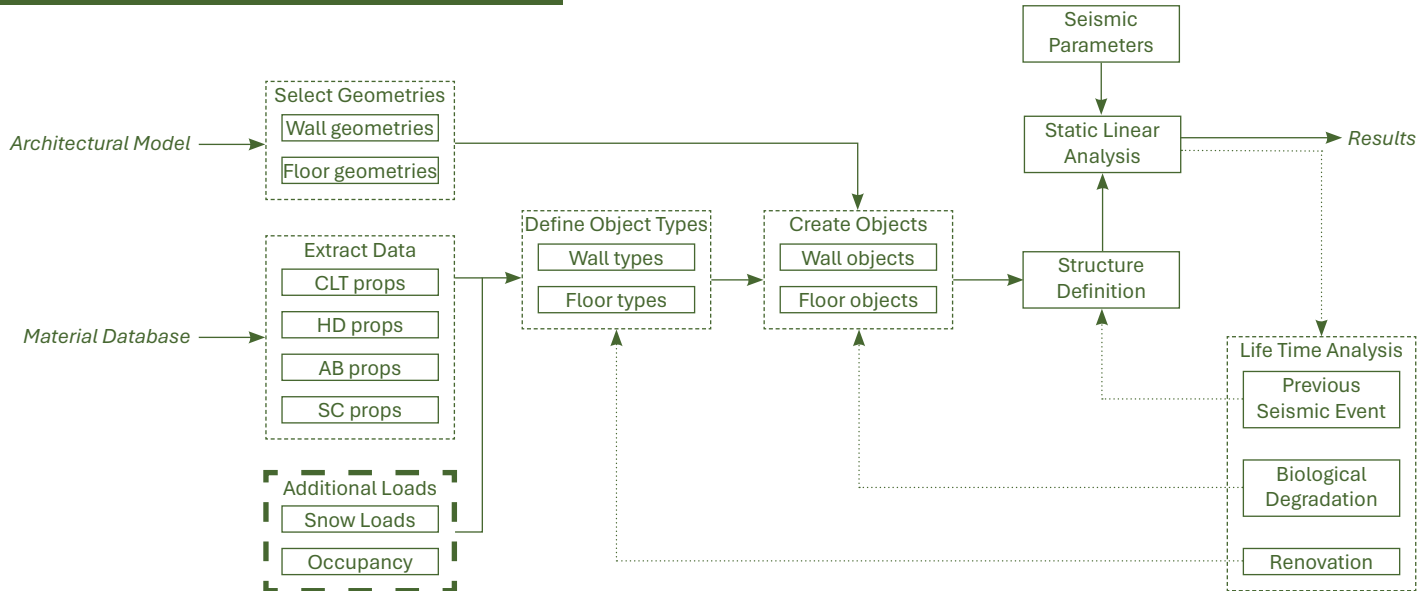


Import Material Data to Grasshopper

- Connect file path to materials database
- Select identifying properties to select CLT, HD, AB, or SC
- Mechanical and mass properties



The Workflow



Model Generation



Additional Variable Floor Loads

- Eurocodes require inclusion of reduced variable loads due to snow or occupancy types



Snow load



Variable load

Model Generation



Additional Variable Floor Loads

- Eurocodes require inclusion of reduced variable loads due to snow or occupancy types
- Standard loads and reduction factors per Eurocodes provided, user can also input their own

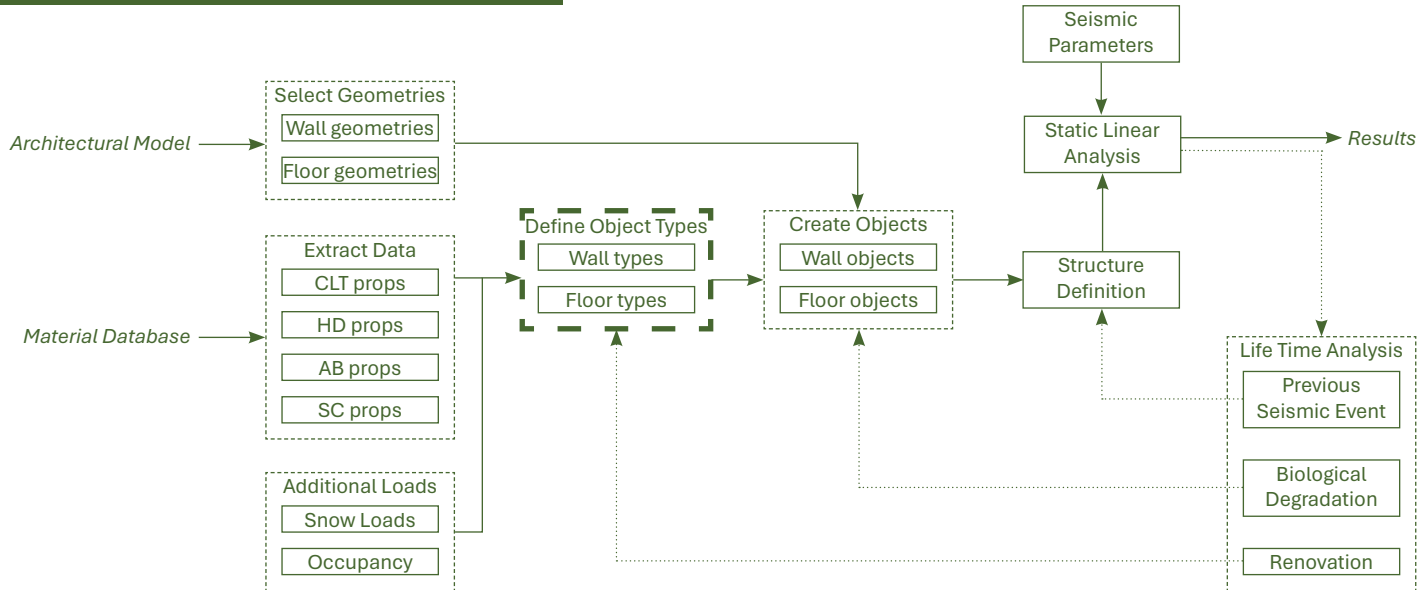


Snow load



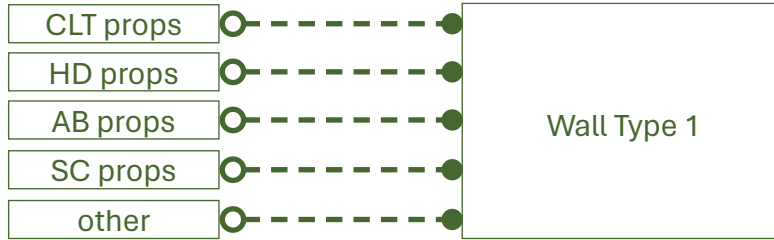
Variable load

The Workflow



Model Generation

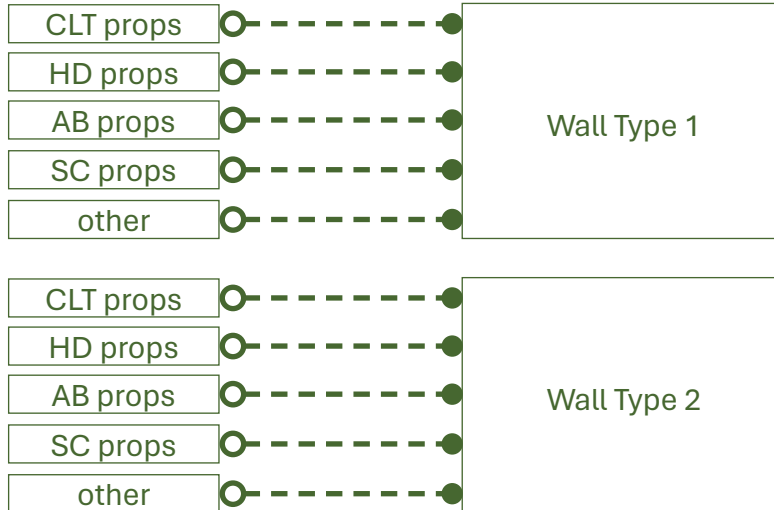
Wall and Floor Types



Model Generation



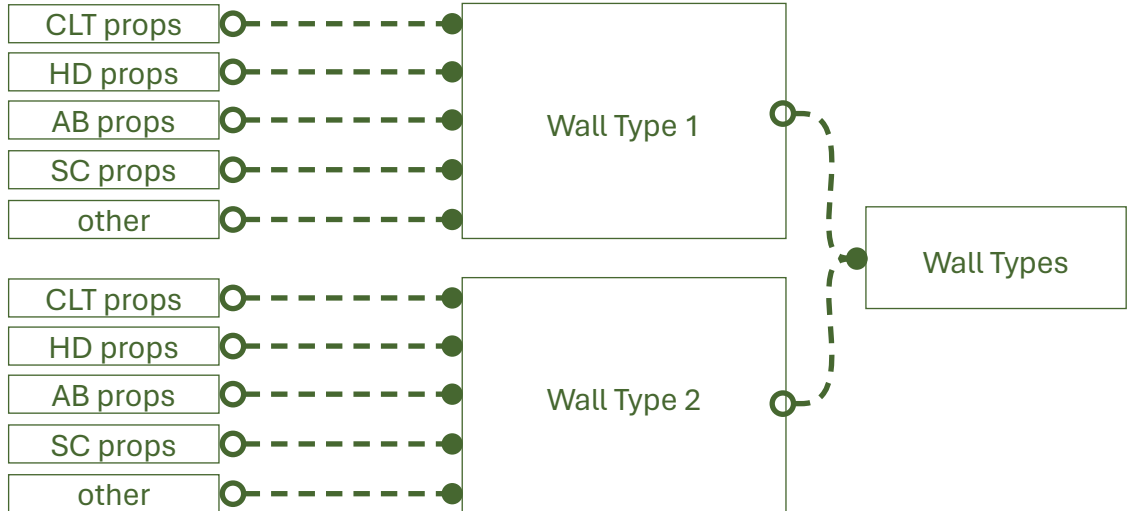
Wall and Floor Types



Model Generation

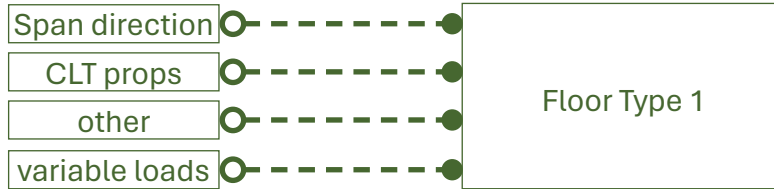


Wall and Floor Types



Model Generation

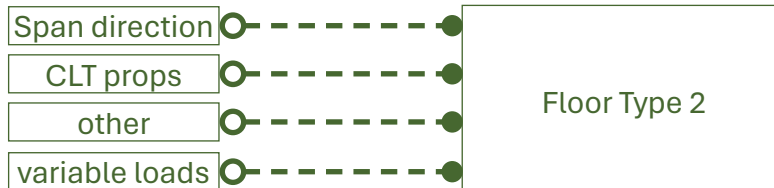
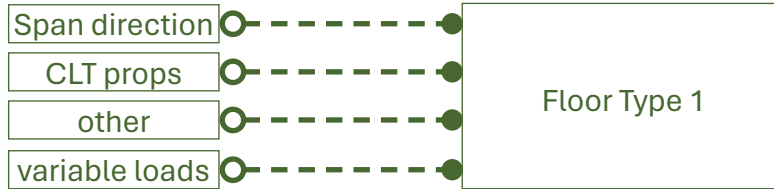
Wall and Floor Types



Model Generation



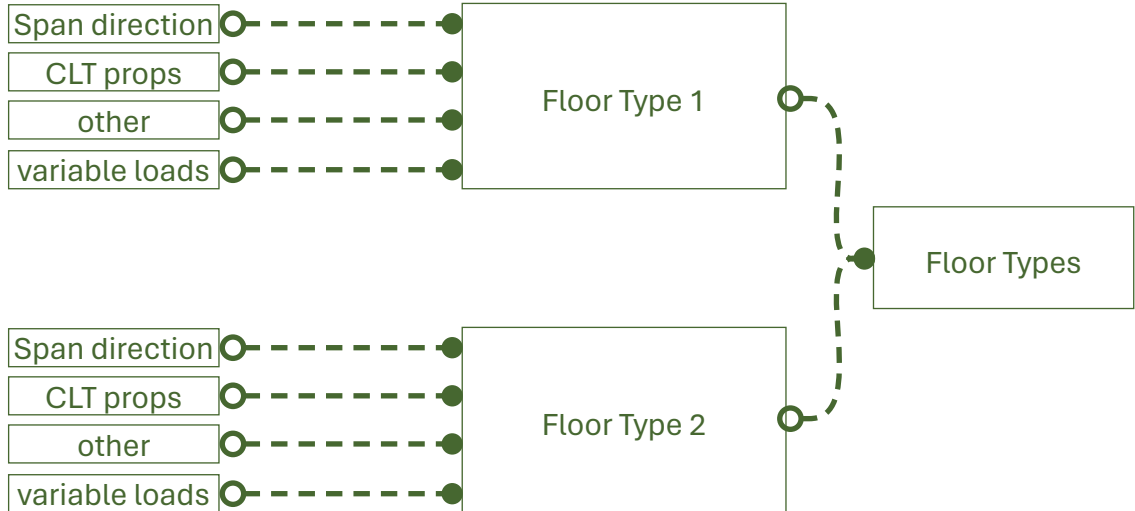
Wall and Floor Types



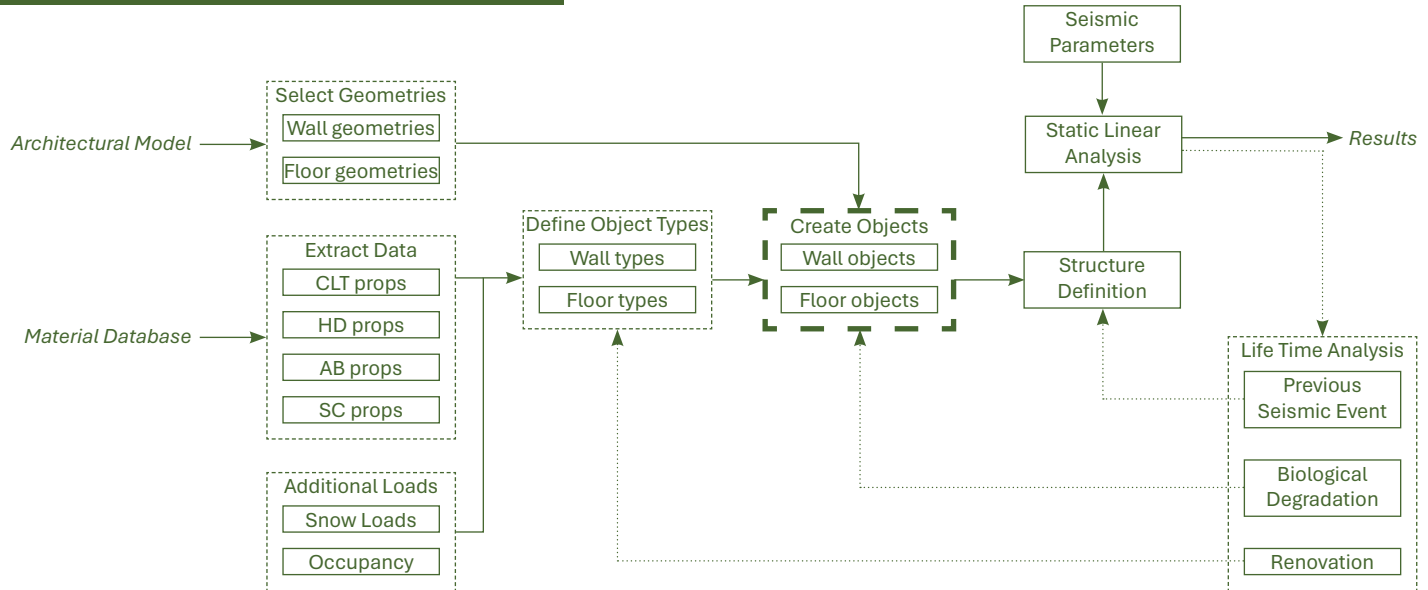
Model Generation



Wall and Floor Types



The Workflow



Model Generation



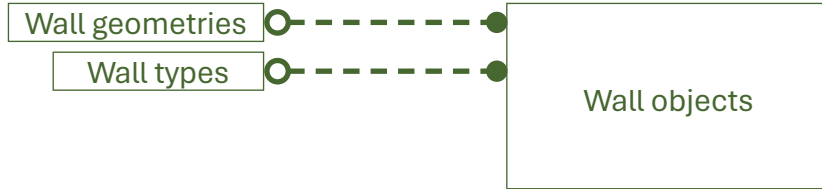
Wall and Floor Objects



Model Generation



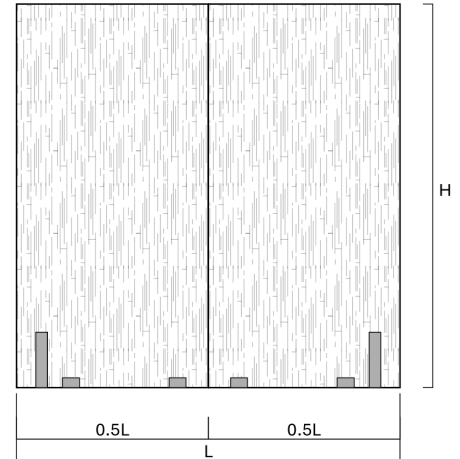
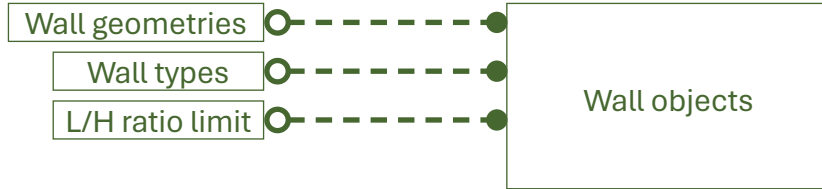
Wall and Floor Objects



Model Generation



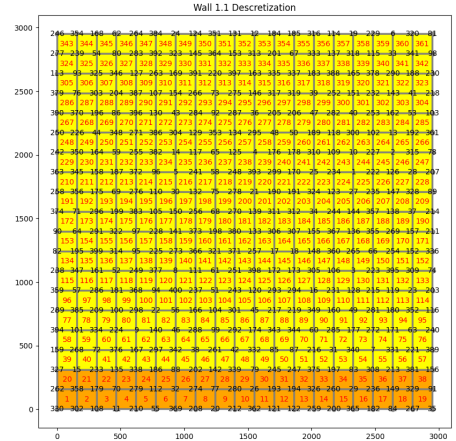
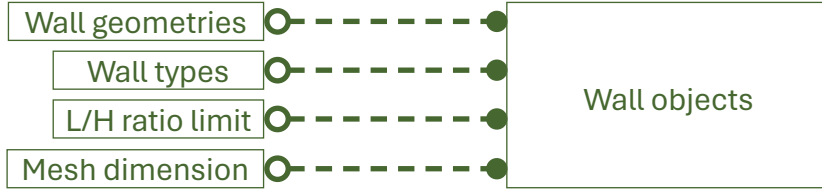
Wall and Floor Objects



Model Generation



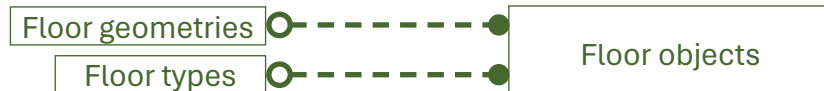
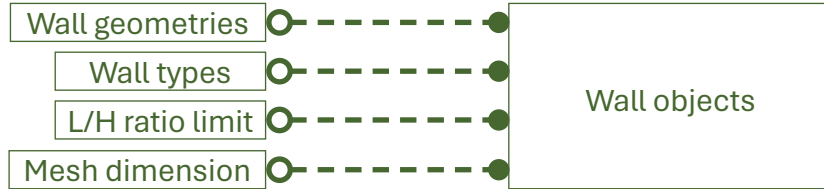
Wall and Floor Objects



Model Generation



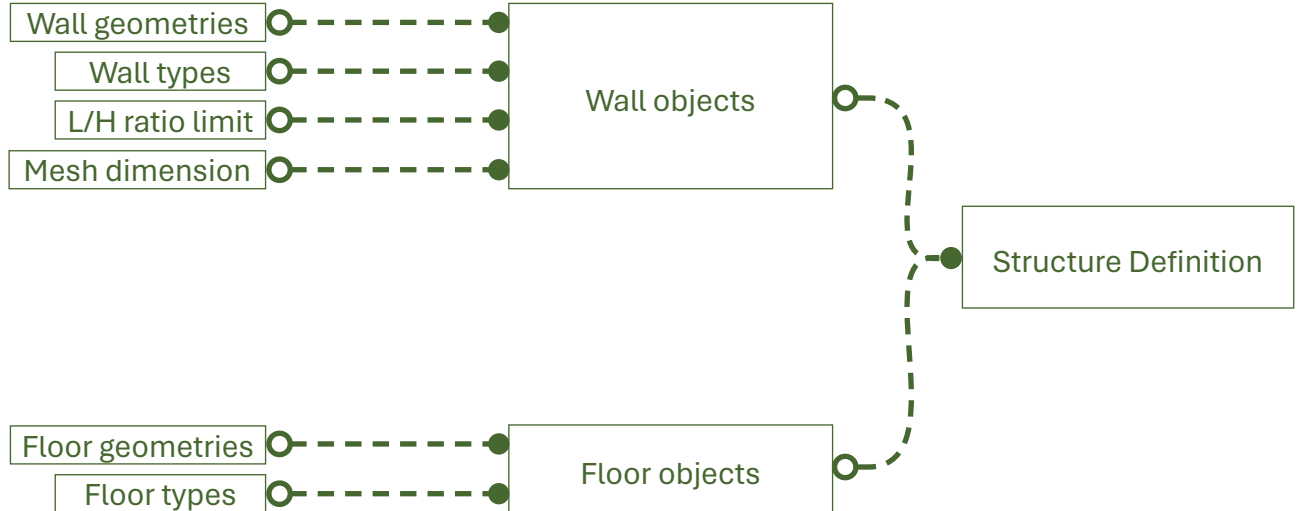
Wall and Floor Objects



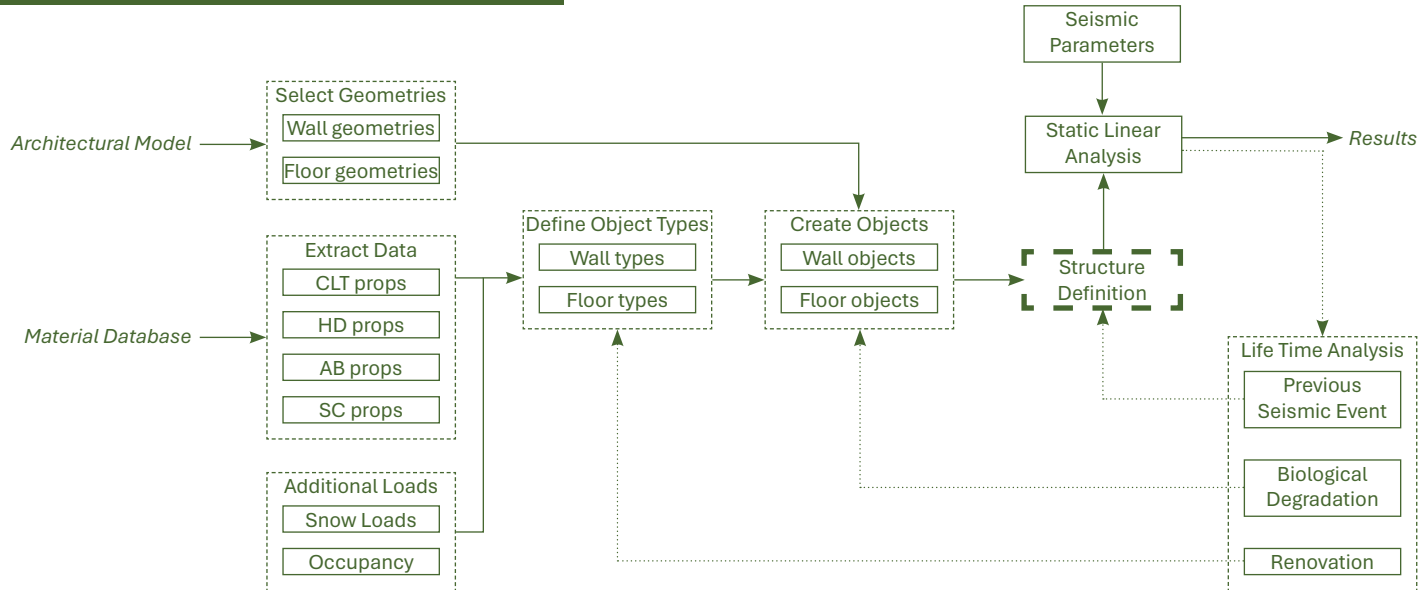
Model Generation



Wall and Floor Objects



The Workflow

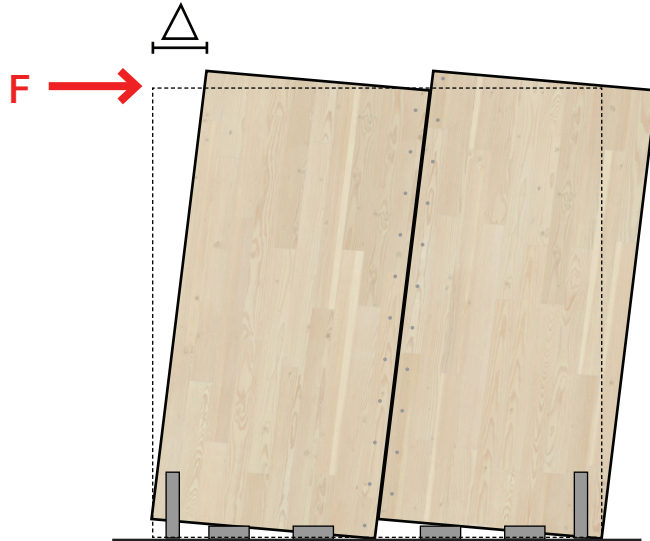


Model Generation



Full Structure Definition

Wall Stiffnesses



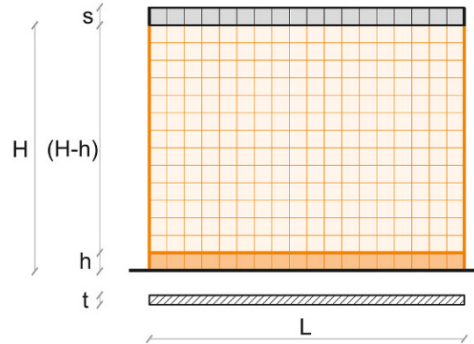
Model Generation



Full Structure Definition

Wall Stiffnesses

- Walls modelled with Rinaldi et al. 2021 strategy



Floor's properties:

$E_{c,90}$



CLT's properties:

$E_{ef,2D,v}$, $E_{ef,2D,h}$, G_{2D}



Connections' properties:

E_{strip} , G_{strip}

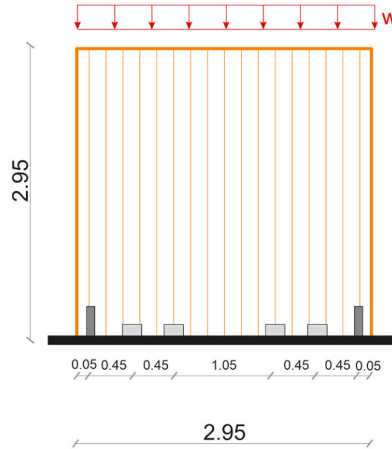
Source: Rinaldi et al. 2021

Model Generation

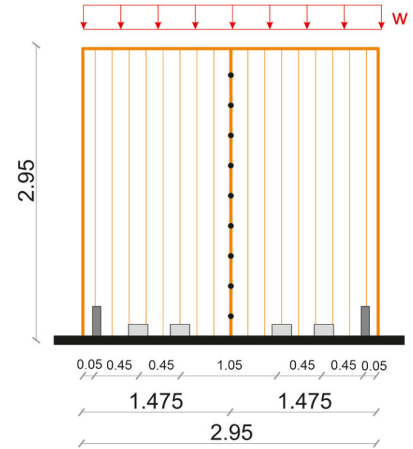


Full Structure Definition

Comparative Analysis



Single-panel wall configurations



Multi-panel wall configurations

Model Generation



Full Structure Definition

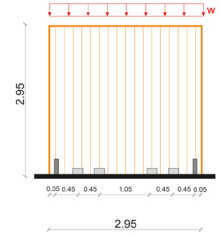
Comparative Analysis

- Compare between implemented model and that of Rinaldi et al. 2021

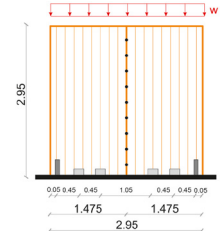
Table – Comparative analysis results for implementation of Rinaldi modelling strategy									
	Number of panels	E_{strip} (MPa)			G_{strip} (MPa)			Stiffness (N/mm)	
		Value	Rinaldi	% Diff.	Value	Rinaldi	% Diff.	Value	Rinaldi
Wall I.1	1	324.00	371.92	-12.88	3.14	3.13	+0.32	3487	3490
Wall I.2		254.00	253.61	+0.15	6.27	6.25	+0.32	6324	5490
Wall I.3		129.00	128.70	+0.23	6.27	6.25	+0.32	6288	4620
Wall II.1	2				3.67	3.66	+0.27	4068	4360
Wall III.1					3.11	3.10	+0.32	3501	3770
Wall III.2					3.11	3.10	+0.32	3501	3770
Wall III.3					3.11	3.10	+0.32	3501	3770
Wall III.4					2.12	2.11	+0.47	2454	2670

* mesh size of 100 mm

Comparative analysis results



Single-panel wall configurations



Multi-panel wall configurations

Model Generation



Full Structure Definition

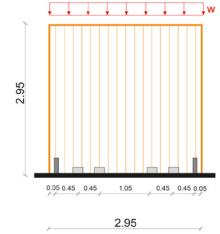
Comparative Analysis

- Compare between implemented model and that of Rinaldi et al. 2021
- Mesh dimension needs to be calibrated to match the values

	Number of panels	E_{strip} (MPa)			G_{strip} (MPa)			Stiffness (N/mm)		
		Value	Rinaldi	% Diff.	Value	Rinaldi	% Diff.	Value	Rinaldi	% Diff.
Wall I.1	1	324.00	371.92	-12.88	3.14	3.13	+0.32	3487	3490	-0.09
Wall I.2		254.00	253.61	+0.15	6.27	6.25	+0.32	6324	5490	+15.19
Wall I.3		129.00	128.70	+0.23	6.27	6.25	+0.32	6288	4620	+36.1
Wall II.1	2				3.67	3.66	+0.27	4068	4360	-6.7
Wall III.1					3.11	3.10	+0.32	3501	3770	-7.14
Wall III.2					3.11	3.10	+0.32	3501	3770	-7.14
Wall III.3					3.11	3.10	+0.32	3501	3770	-7.14
Wall III.4					2.12	2.11	+0.47	2454	2670	-8.09

* mesh size of 100 mm

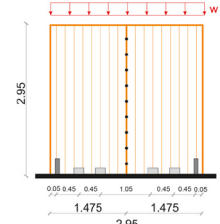
Comparative analysis results



Single-panel wall configurations

Mesh dim.	No.	R.	Stiffness (N/mm)											
			50	%	100	%	112.5	%	125	%	150	%		
Wall I.1	1	3490	2888	-17.25	3487	-0.09	3159	-9.48	2825	-19.05	2367	-32.18		
Wall I.2		5490	5316	-3.17	6324	+15.19	5780	+5.28	5216	-4.99	4428	-19.34		
Wall I.3		4620	5285	+14.39	6288	+36.1	5751	+24.48	5192	+12.38	4410	-4.55		
Wall II.1	2	4360	3376	-22.57	4068	-6.7	3688	-15.41	3299	-24.33	2767	-36.54		
Wall III.1		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16		
Wall III.2		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16		
Wall III.3		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16		
Wall III.4		2670	2021	-24.31	2454	-8.09	2215	-17.04	1973	-26.01	1645	-38.39		

Mesh dimension calibration



Multi-panel wall configurations

Model Generation



Full Structure Definition

Comparative Analysis

- Compare between implemented model and that of Rinaldi et al. 2021
- Mesh dimension needs to be calibrated to match the values
- Currently underestimating by 8% or less* with a mesh dimension of 100 mm

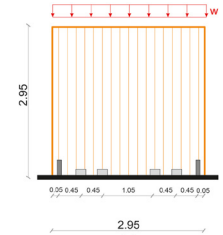
	Number of panels	E_{strip} (MPa)			G_{strip} (MPa)			Stiffness (N/mm)		
		Value	Rinaldi	% Diff.	Value	Rinaldi	% Diff.	Value	Rinaldi	% Diff.
Wall I.1	1	324.00	371.92	-12.88	3.14	3.13	+0.32	3487	3490	-0.09
Wall I.2		254.00	253.61	+0.15	6.27	6.25	+0.32	6324	5490	+15.19
Wall I.3		129.00	128.70	+0.23	6.27	6.25	+0.32	6288	4620	+36.1
Wall II.1	2				3.67	3.66	+0.27	4068	4360	-6.7
Wall III.1					3.11	3.10	+0.32	3501	3770	-7.14
Wall III.2					3.11	3.10	+0.32	3501	3770	-7.14
Wall III.3					3.11	3.10	+0.32	3501	3770	-7.14
Wall III.4					2.12	2.11	+0.47	2454	2670	-8.09

* mesh size of 100 mm

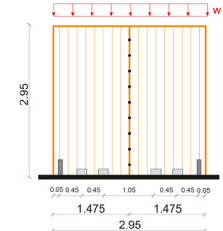
Comparative analysis results

Mesh dim.	No.	R.	Stiffness (N/mm)									
			50	%	100	%	125	%	150	%		
Wall I.1	1	3490	2888	-17.25	3487	-0.09	3159	-9.48	2825	-19.05	2367	-32.18
Wall I.2		5490	5316	-3.17	6324	+15.19	5780	+5.28	5216	-4.99	4428	-19.34
Wall I.3		4620	5285	+14.39	6288	+36.1	5751	+24.48	5192	+12.38	4410	-4.55
Wall II.1	2	4360	3376	-22.57	4068	-6.7	3688	-15.41	3299	-24.33	2767	-36.54
Wall III.1		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.2		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.3		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.4		2670	2021	-24.31	2454	-8.09	2215	-17.04	1973	-26.01	1645	-38.39

Mesh dimension calibration



Single-panel wall configurations



Multi-panel wall configurations

Model Generation



Full Structure Definition

Comparative Analysis

- Compare between implemented model and that of Rinaldi et al. 2021
- Mesh dimension needs to be calibrated to match the values
- Currently underestimating by 8% or less* with a mesh dimension of 100 mm
 - *Walls with 4 rather than 2 angle brackets are outliers, overestimating stiffness by up to 36%

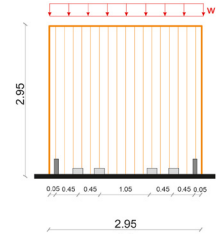
	Number of panels	E_{strip} (MPa)			G_{strip} (MPa)			Stiffness (N/mm)		
		Value	Rinaldi	% Diff.	Value	Rinaldi	% Diff.	Value	Rinaldi	% Diff.
Wall I.1	1	324.00	371.92	-12.88	3.14	3.13	+0.32	3487	3490	-0.09
Wall I.2		254.00	253.61	+0.15	6.27	6.25	+0.32	6324	5490	+15.19
Wall I.3		129.00	128.70	+0.23	6.27	6.25	+0.32	6288	4620	+36.1
Wall II.1	2				3.67	3.66	+0.27	4068	4360	-6.7
Wall III.1					3.11	3.10	+0.32	3501	3770	-7.14
Wall III.2					3.11	3.10	+0.32	3501	3770	-7.14
Wall III.3					3.11	3.10	+0.32	3501	3770	-7.14
Wall III.4					2.12	2.11	+0.47	2454	2670	-8.09

* mesh size of 100 mm

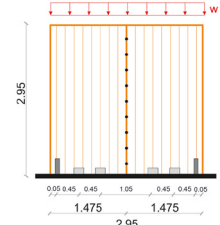
Comparative analysis results

Mesh dim.	No.	R.	Stiffness (N/mm)									
			50	%	100	%	125	%	150	%		
Wall I.1		3490	2888	-17.25	3487	-0.09	3159	-9.48	2825	-19.05	2367	-32.18
Wall I.2	1	5490	5316	-3.17	6324	+15.19	5780	+5.28	5216	-4.99	4428	-19.34
Wall I.3		4620	5285	+14.39	6288	+36.1	5751	+24.48	5192	+12.38	4410	-4.55
Wall II.1		4360	3376	-22.57	4068	-6.7	3688	-15.41	3299	-24.33	2767	-36.54
Wall III.1		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.2	2	3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.3		3770	2898	-23.13	3501	-7.14	3169	-15.94	2831	-24.91	2369	-37.16
Wall III.4		2670	2021	-24.31	2454	-8.09	2215	-17.04	1973	-26.01	1645	-38.39

Mesh dimension calibration



Single-panel wall configurations



Multi-panel wall configurations

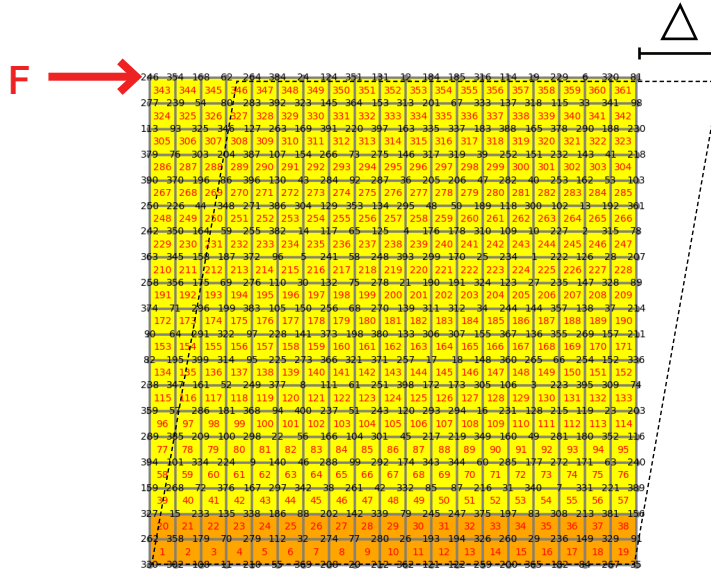
Model Generation



Full Structure Definition

Wall Stiffnesses

- Walls modelled with Rinaldi et al. 2021 strategy
- Pushover analysis



Model Generation



Full Structure Definition

Wall Stiffnesses

- Walls modelled with Rinaldi et al. 2021 strategy
- Pushover analysis

Structure Parameters

k_{n1}		k_{n2}		k_{n3}
k_{31}		k_{32}		k_{33}
k_{21}		k_{22}		k_{23}
k_{11}		k_{12}		k_{13}

Model Generation



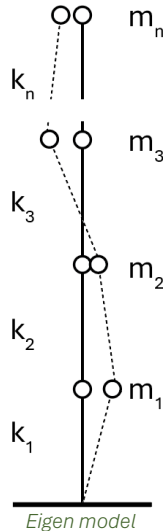
Full Structure Definition

Wall Stiffnesses

- Walls modelled with Rinaldi et al. 2021 strategy
- Pushover analysis

Structure Parameters

- Eigen analysis



Model Generation



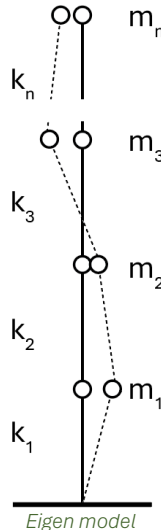
Full Structure Definition

Wall Stiffnesses

- Walls modelled with Rinaldi et al. 2021 strategy
- Pushover analysis

Structure Parameters

- Eigen analysis
- Natural building period

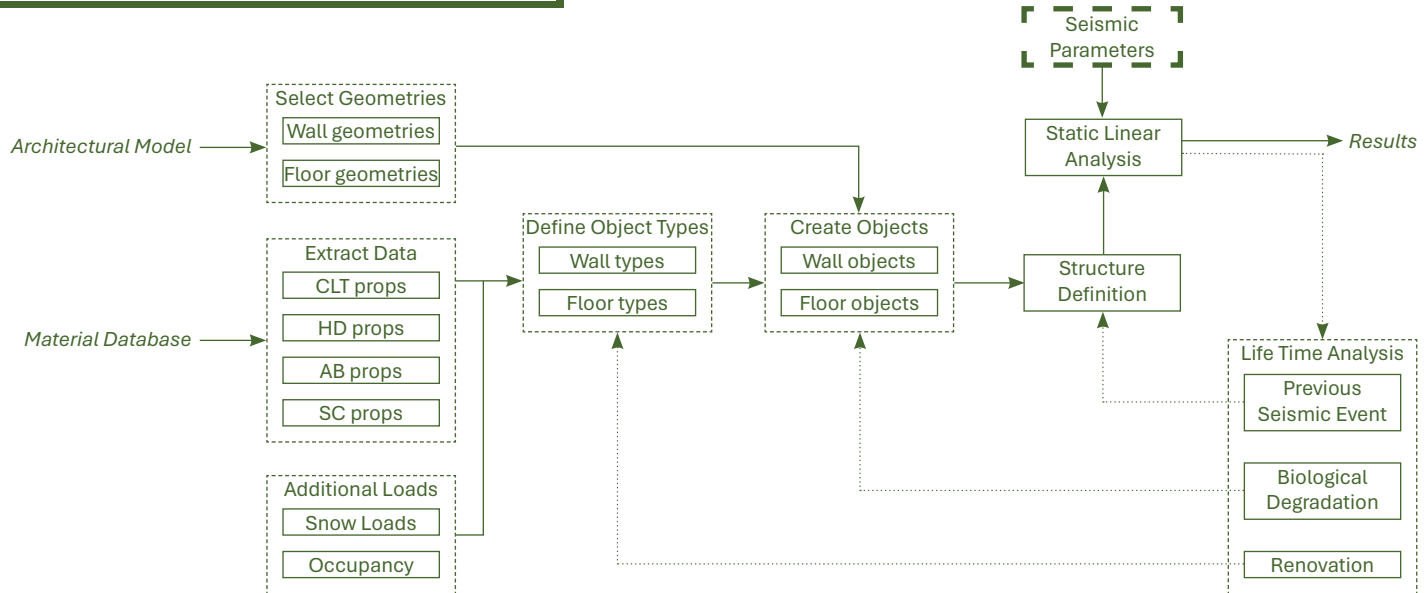


$$\omega = \sqrt{\frac{m}{k}} = \text{eigen vector}$$

$$T = 2\pi\omega$$

Relationship b/t eigen analysis and building period

The Workflow



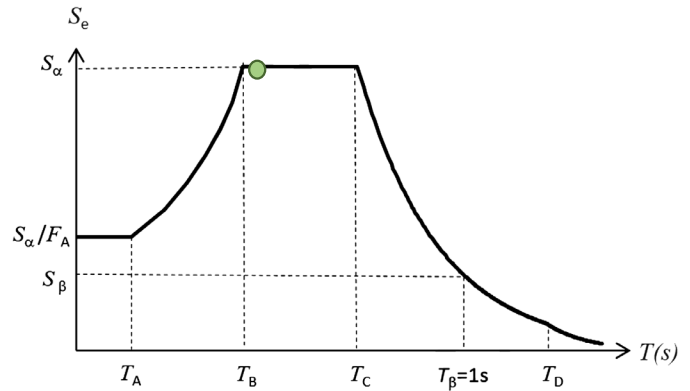
Seismic Parameters



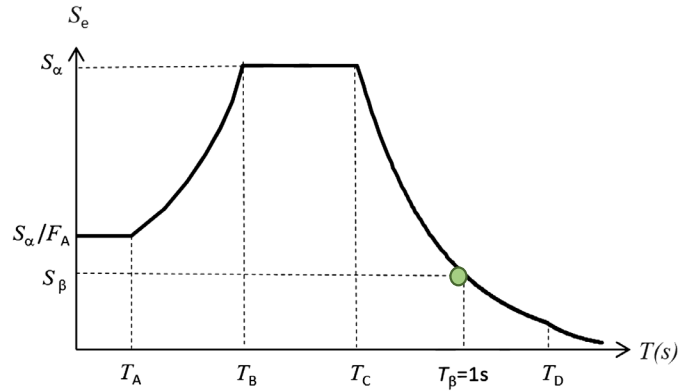
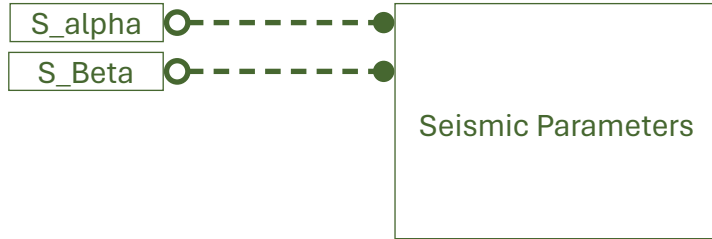
S_alpha



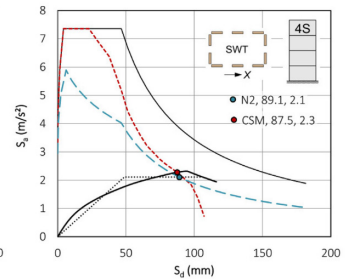
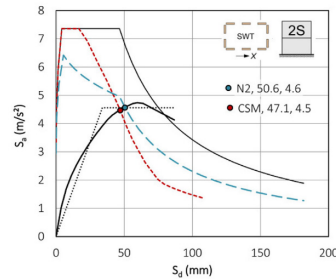
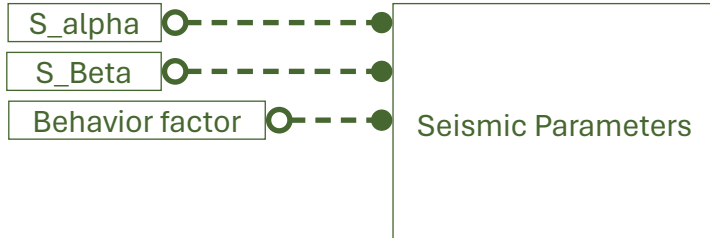
Seismic Parameters



Seismic Parameters



Seismic Parameters



Behavior factor $q = 3.0$
Source: Hummel and Seim, 2019

Seismic Parameters

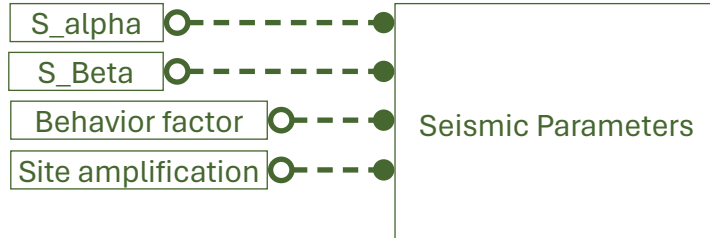


Table 5.4 — Site amplification factors F_α and F_β for the standard site categories of Table 5.1

Site category	F_α		F_β	
	H_{800} and $v_{s,H}$ available	Default value	H_{800} and $v_{s,H}$ available	Default value
A	1,0	1,0	1,0	1,0
B	$\left(\frac{v_{s,H}}{800}\right)^{-0,40} r_\alpha$	1,3 (1 - 0,1 $S_{\alpha,RP}/g$)	$\left(\frac{v_{s,H}}{800}\right)^{-0,70} r_\beta$	1,6 (1 - 0,2 $S_{\beta,RP}/g$)
C		1,6 (1 - 0,2 $S_{\alpha,RP}/g$)		2,3 (1 - 0,3 $S_{\beta,RP}/g$)
D		1,8 (1 - 0,3 $S_{\alpha,RP}/g$)		3,2 (1 - $S_{\beta,RP}/g$)
E	$\left(\frac{v_{s,H}}{800}\right)^{-0,40} r_\alpha \frac{H}{30} \left(4 - \frac{H}{10}\right)$	2,2 (1 - 0,5 $S_{\alpha,RP}/g$)	$\left(\frac{v_{s,H}}{800}\right)^{-0,70} r_\beta \frac{H}{30}$	3,2 (1 - $S_{\beta,RP}/g$)
F	$0,90 \left(\frac{v_{s,H}}{800}\right)^{-0,40} r_\alpha$	1,7 (1 - 0,3 $S_{\alpha,RP}/g$)	$1,25 \left(\frac{v_{s,H}}{800}\right)^{-0,70} r_\beta$	4,0 (1 - $S_{\beta,RP}/g$)
	with $r_\alpha = 1 - \frac{S_{\alpha,RP}/g}{v_{s,H}/150}$ and $r_\beta = 1 - \frac{S_{\beta,RP}/g}{v_{s,H}/150}$			

Site amplification factors per EN 1998

Seismic Parameters

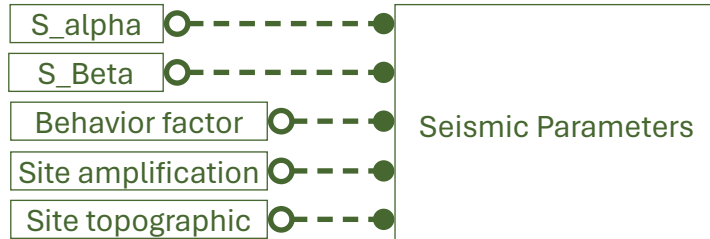
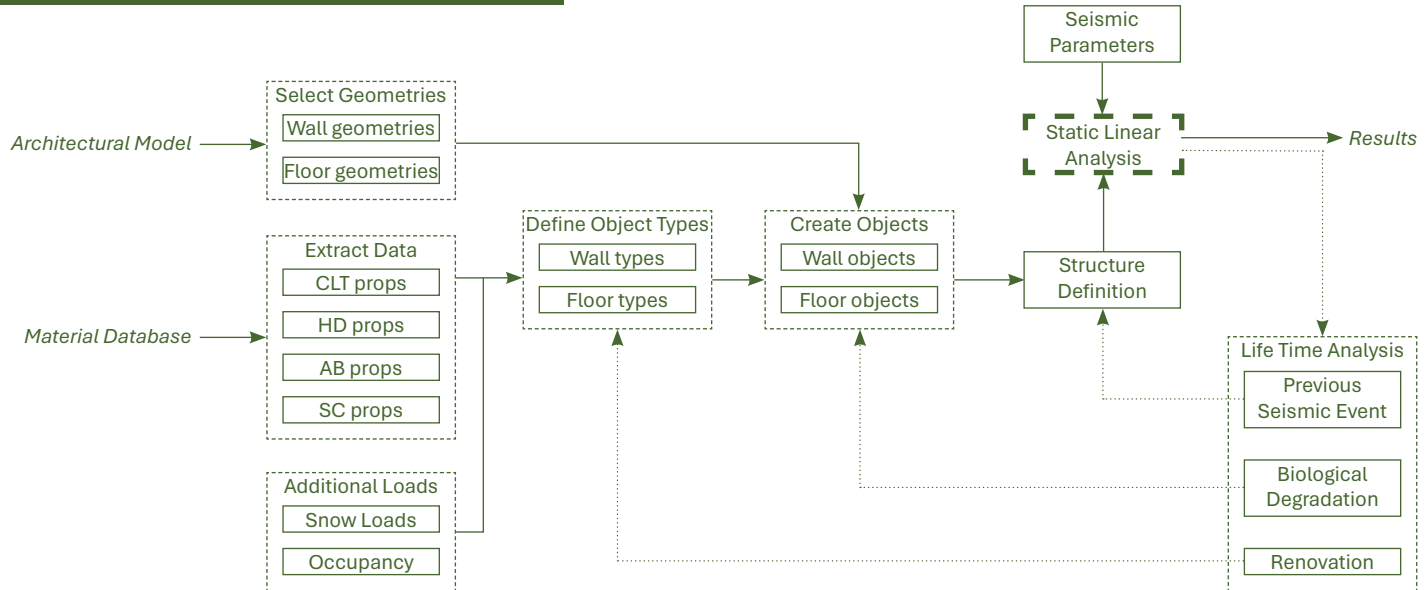


Table 5.5 — Topography amplification factors for simple topographic irregularities

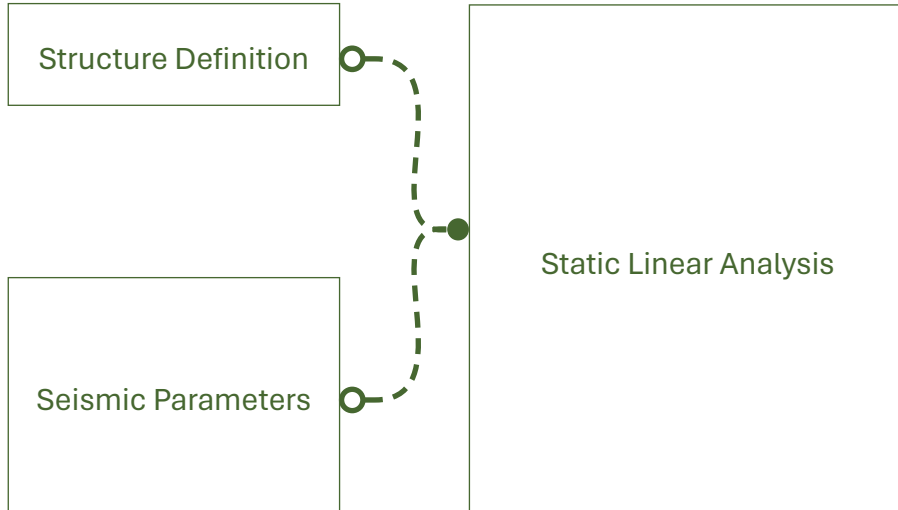
Topography description	F_t	Simplified sketch*
Flat ground surface, slopes and isolated ridges with average slope angle $i < 15^\circ$ or height < 30 m	1,0	
Slopes with average slope angle $i > 15^\circ$	1,2	
Ridges with width at the top much smaller than at the base and average slope angle $15^\circ < i < 30^\circ$	1,2	
Ridges with width at the top much smaller than at the base and average slope angle $i > 30^\circ$	1,4	

Topographic amplification factors per EN 1998

The Workflow

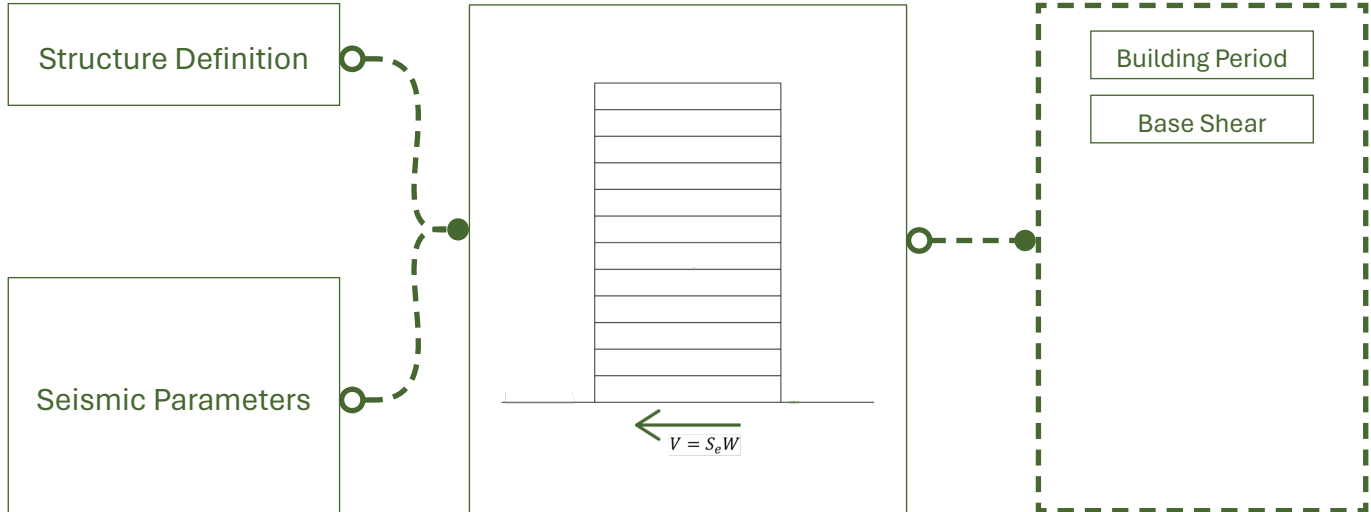


Static Linear Analysis

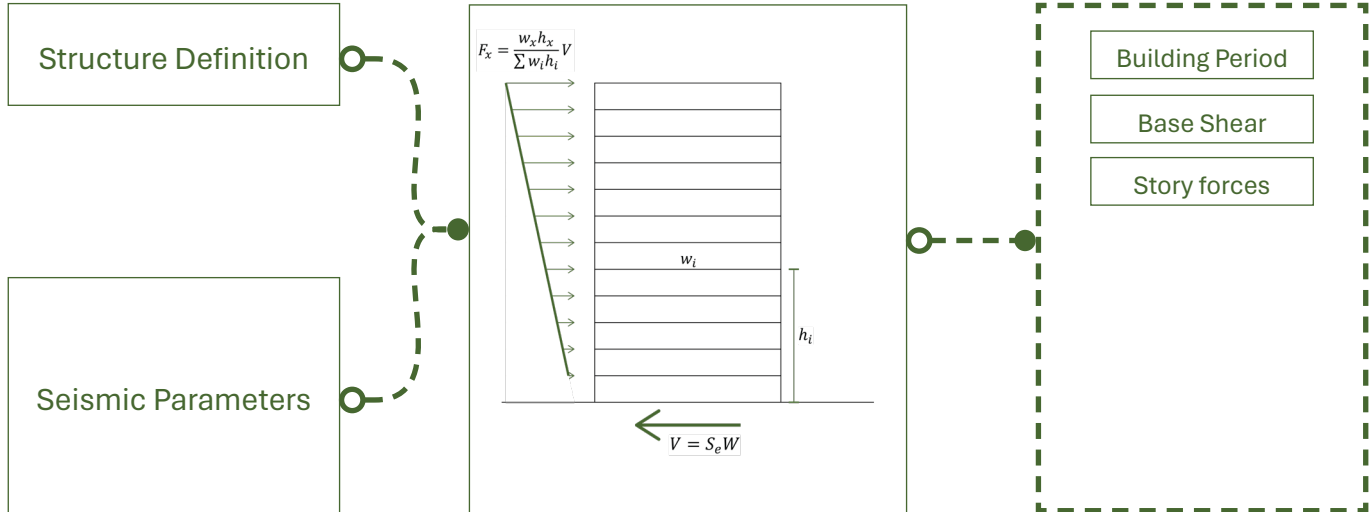




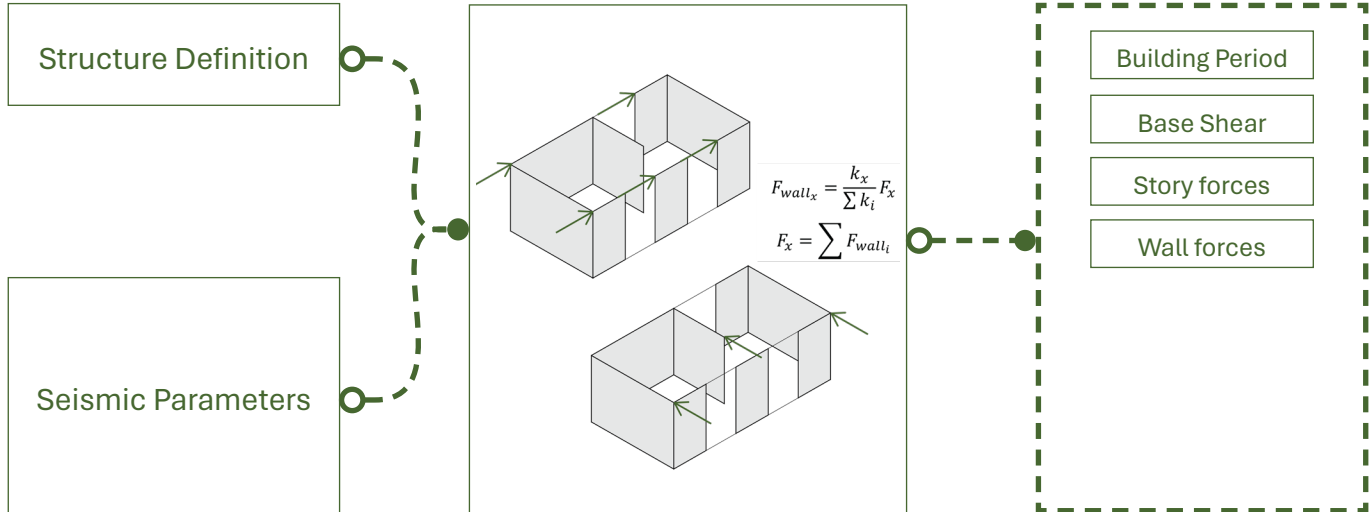
Static Linear Analysis



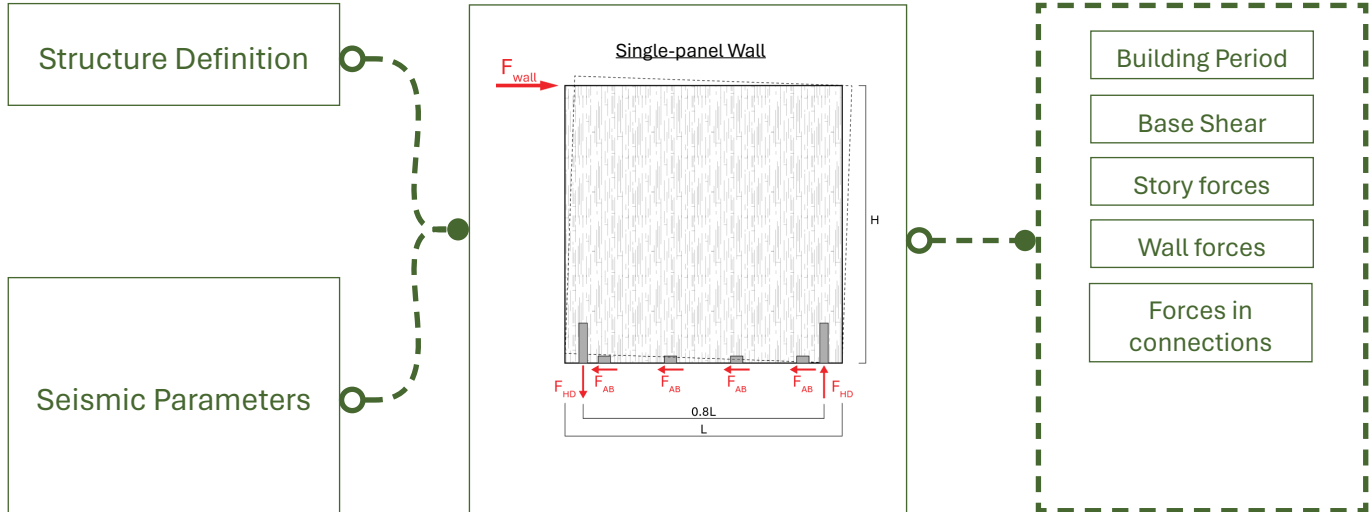
Static Linear Analysis



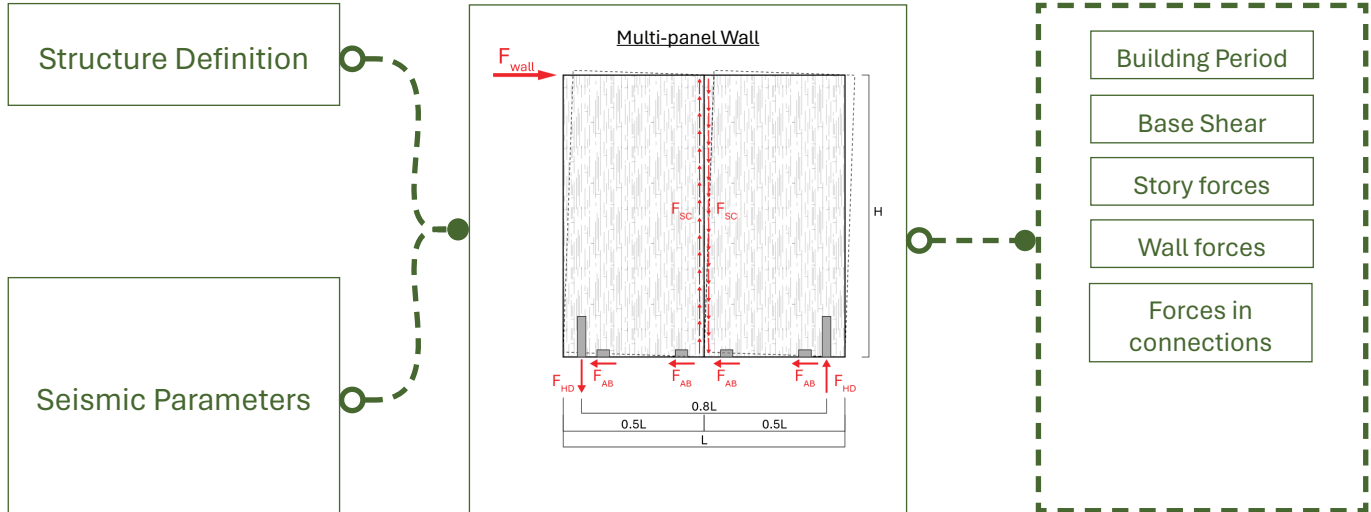
Static Linear Analysis



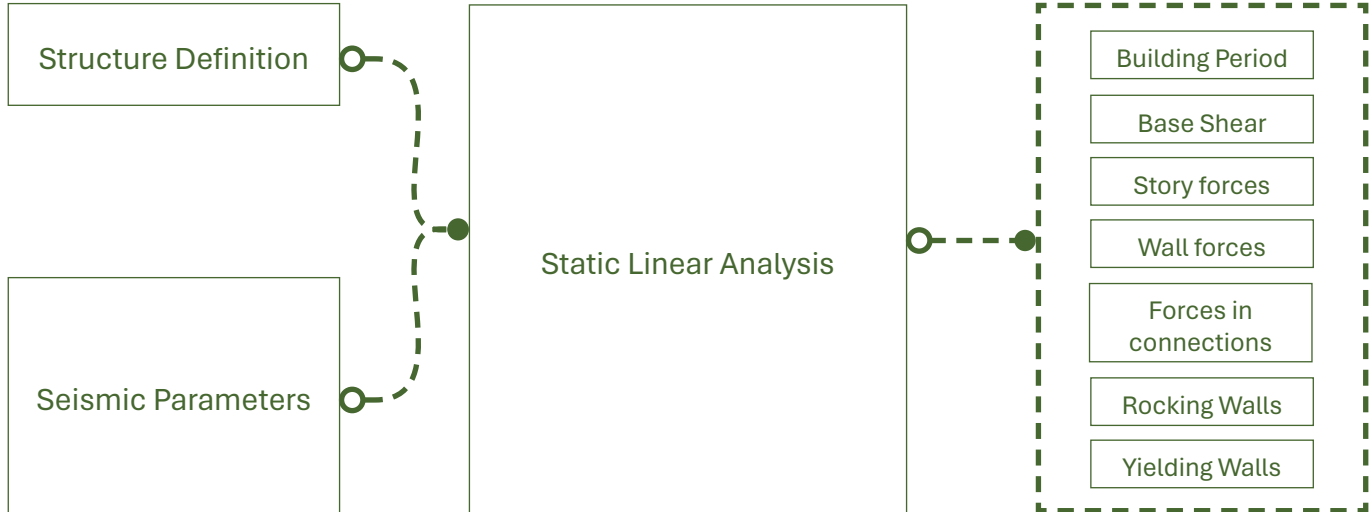
Static Linear Analysis



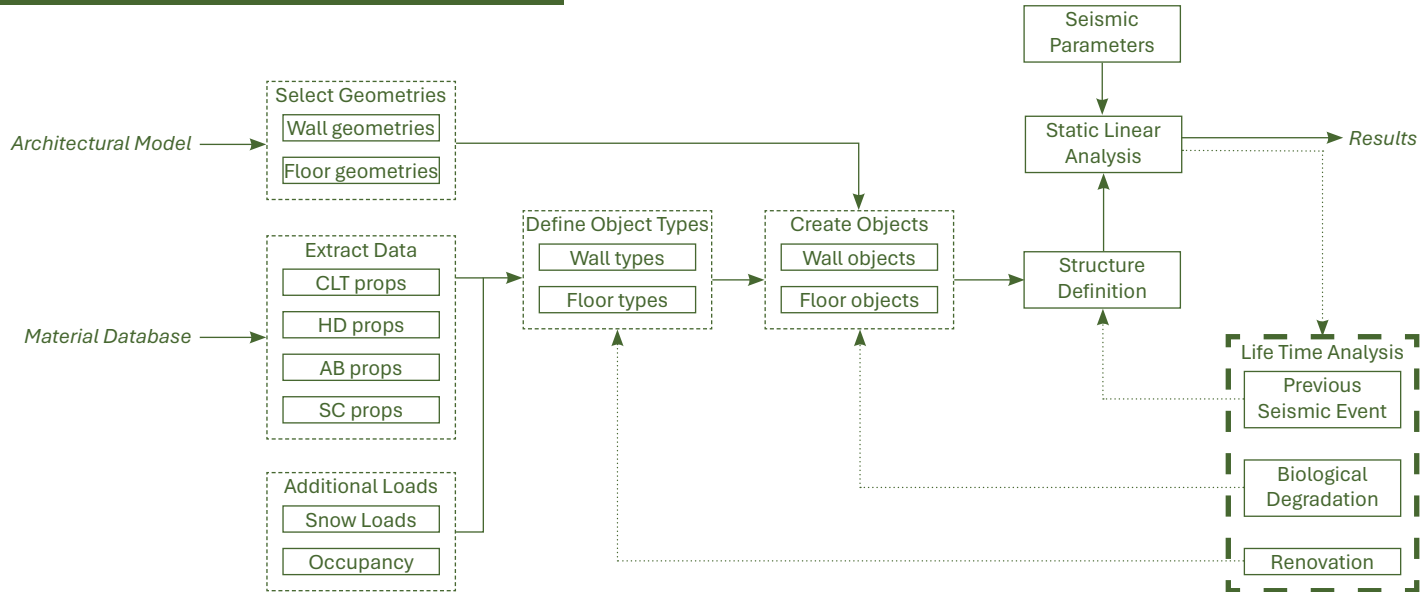
Static Linear Analysis



Static Linear Analysis



The Workflow



Life Time Analysis

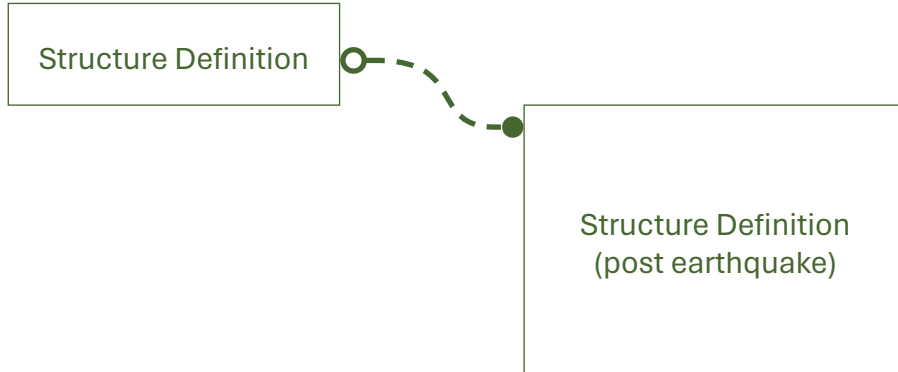
Previous Seismic Event



Structure Definition
(post earthquake)

Life Time Analysis

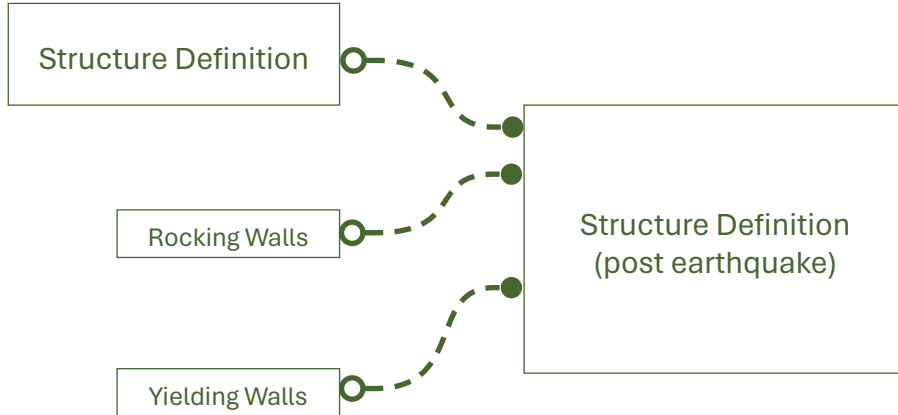
Previous Seismic Event



Life Time Analysis

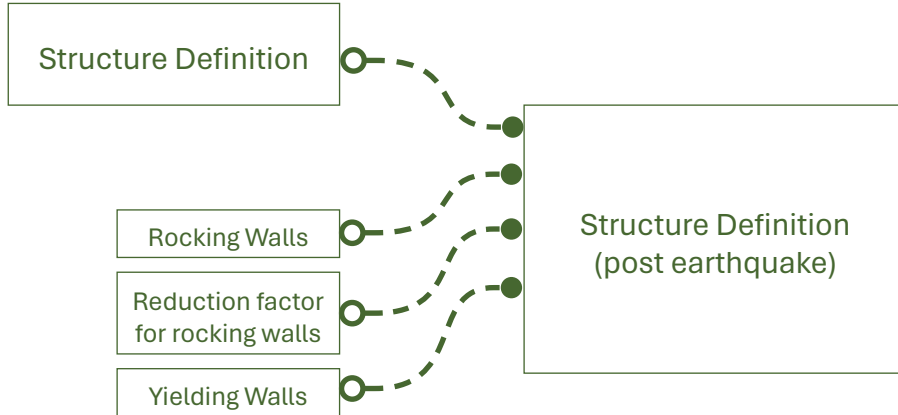


Previous Seismic Event



Life Time Analysis

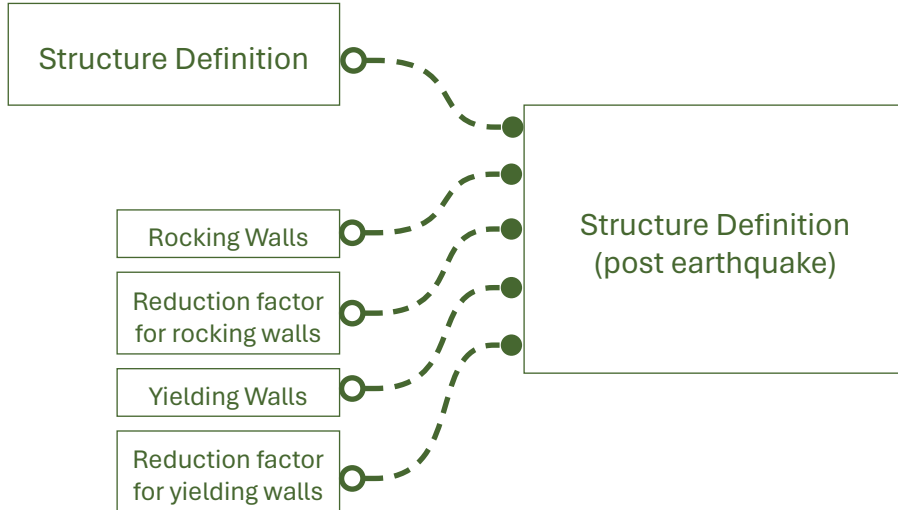
Previous Seismic Event



Life Time Analysis

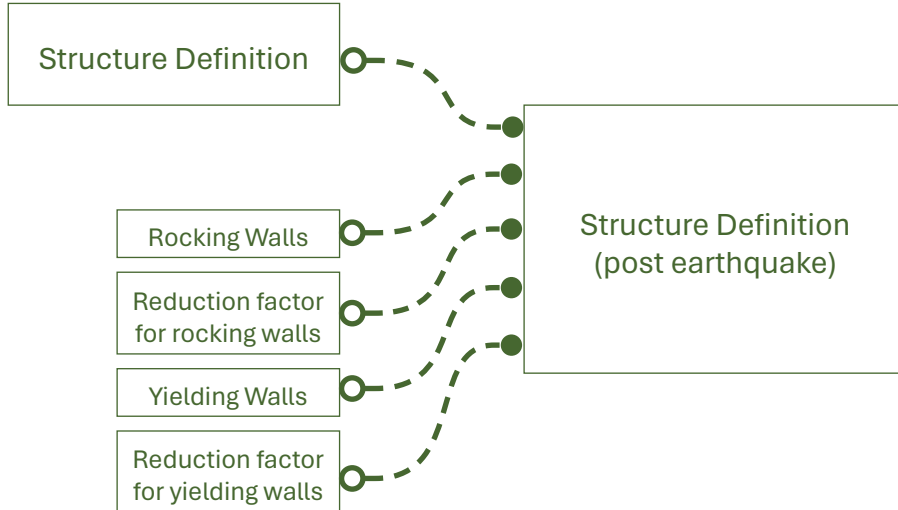


Previous Seismic Event



Life Time Analysis

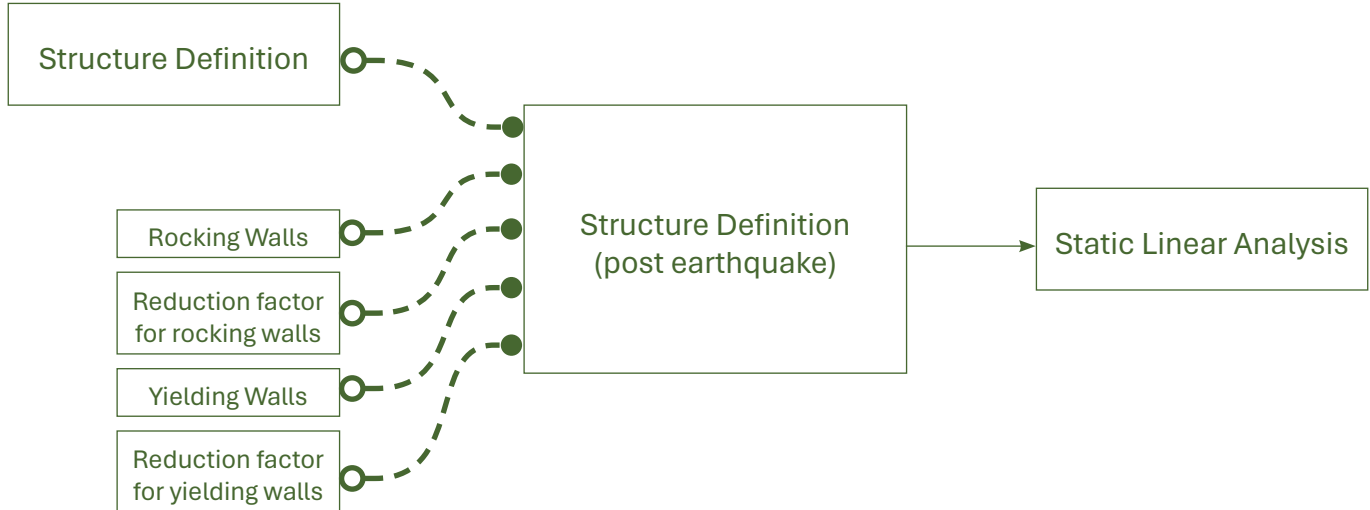
Previous Seismic Event



Life Time Analysis



Previous Seismic Event



Life Time Analysis

Renovation



Structure Definition
(renovation)

Life Time Analysis

Renovation

Structure Definition



Structure Definition
(renovation)

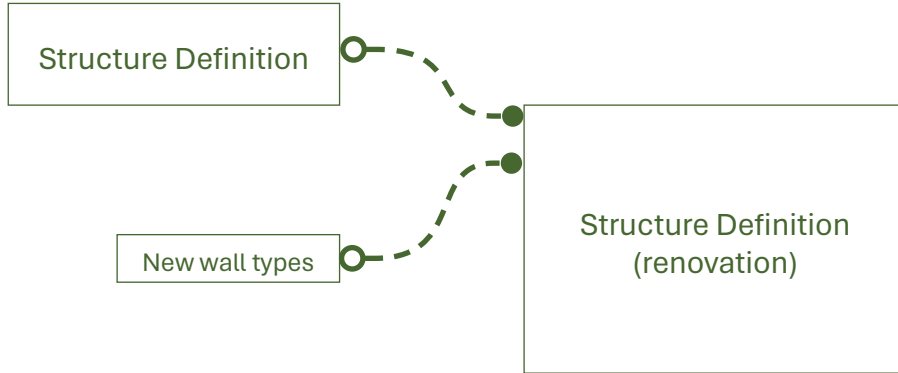
Wall objects

Floor objects



Life Time Analysis

Renovation

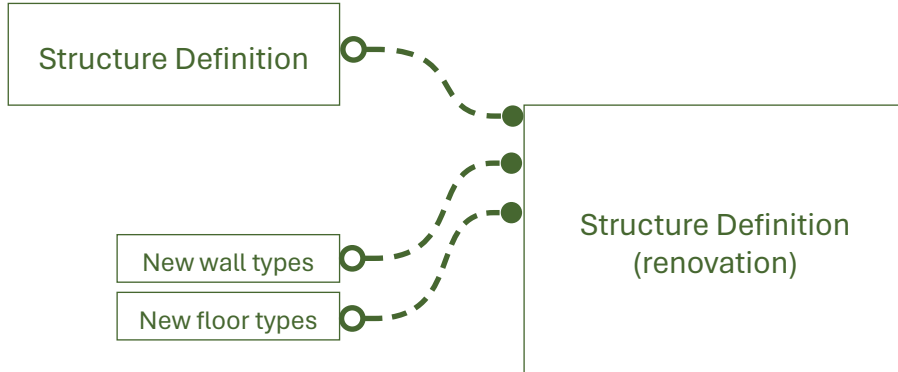


Facade renovation



Life Time Analysis

Renovation

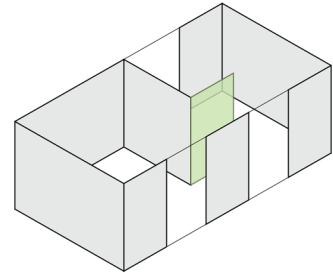
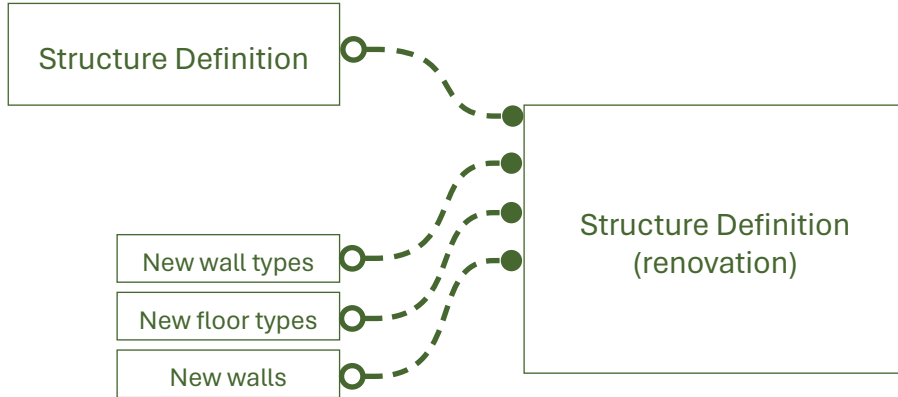


Change in occupancy



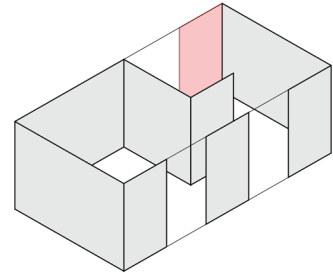
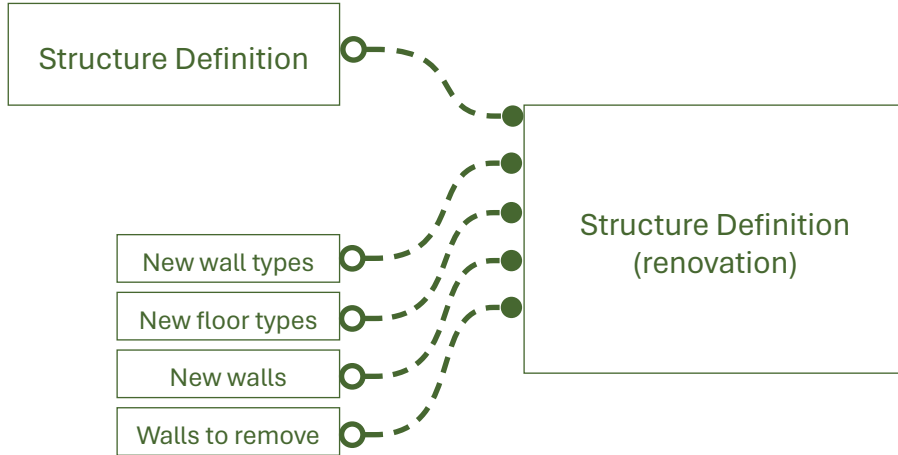
Life Time Analysis

Renovation



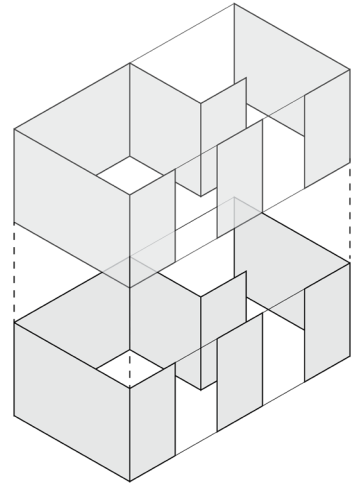
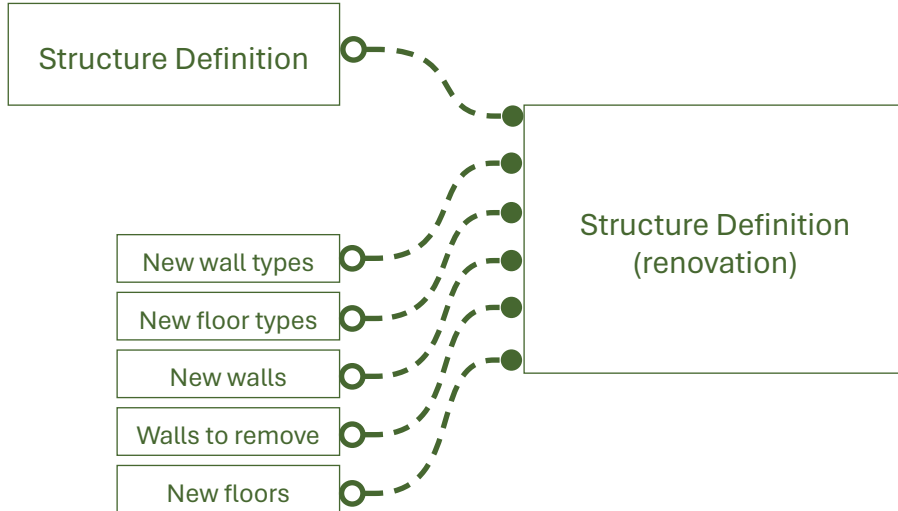
Life Time Analysis

Renovation



Life Time Analysis

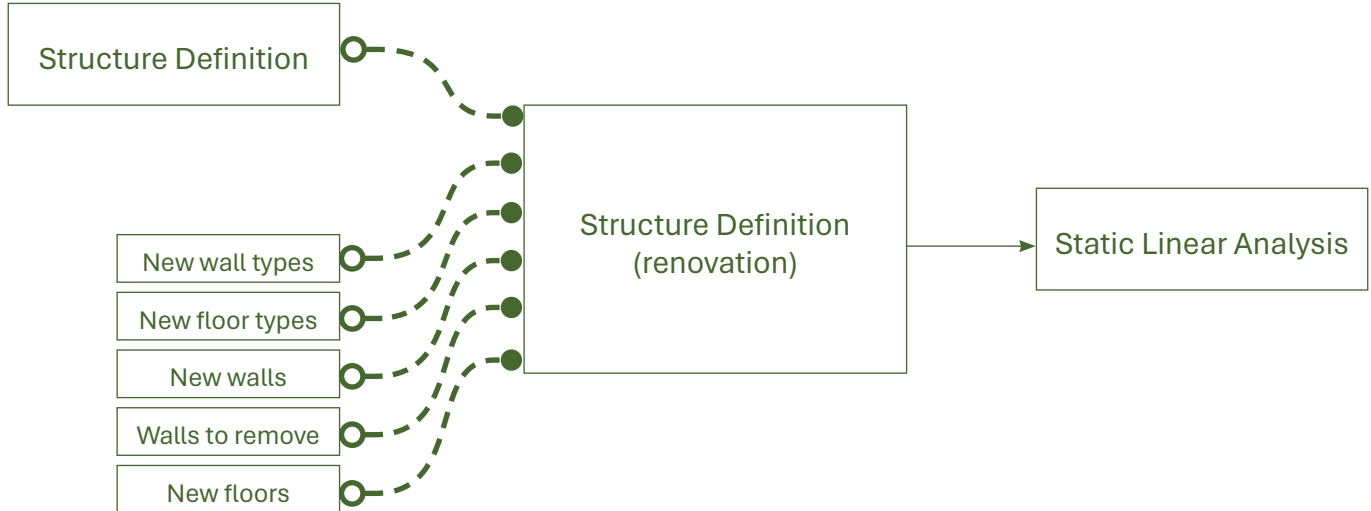
Renovation



Life Time Analysis



Renovation



Life Time Analysis

Biological Degradation - Walls over time



Biological Degradation
of walls over time

Life Time Analysis

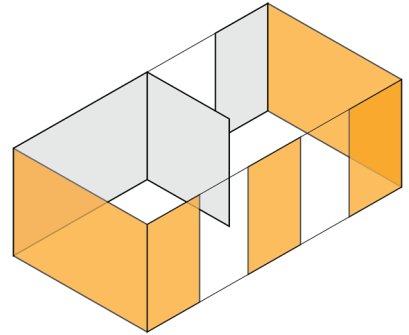
Biological Degradation - Walls over time



Walls impacted

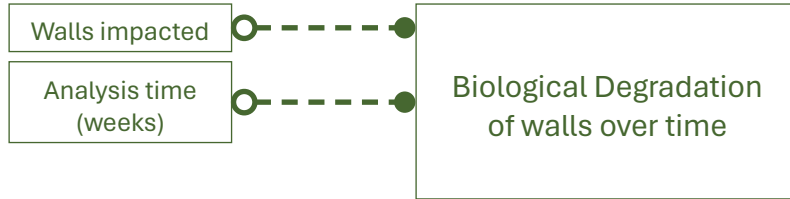


Biological Degradation
of walls over time



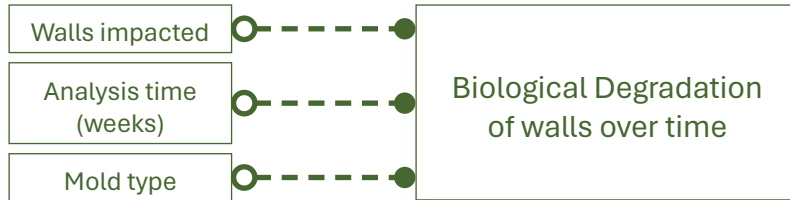
Life Time Analysis

Biological Degradation - Walls over time



Life Time Analysis

Biological Degradation - Walls over time



Gloeophyllum trabeum

$$F_{dbs_{DF}} = \sqrt{-16(time) + 1891.2}$$

$$F_{dbs_{NS}} = -0.31(time) + 32.3$$

$$F_{dbs_{SPF}} = -0.26(time) + 37.1$$

Rhodonía placenta

$$F_{dbs_{DF}} = -0.58(time) + 45.2$$

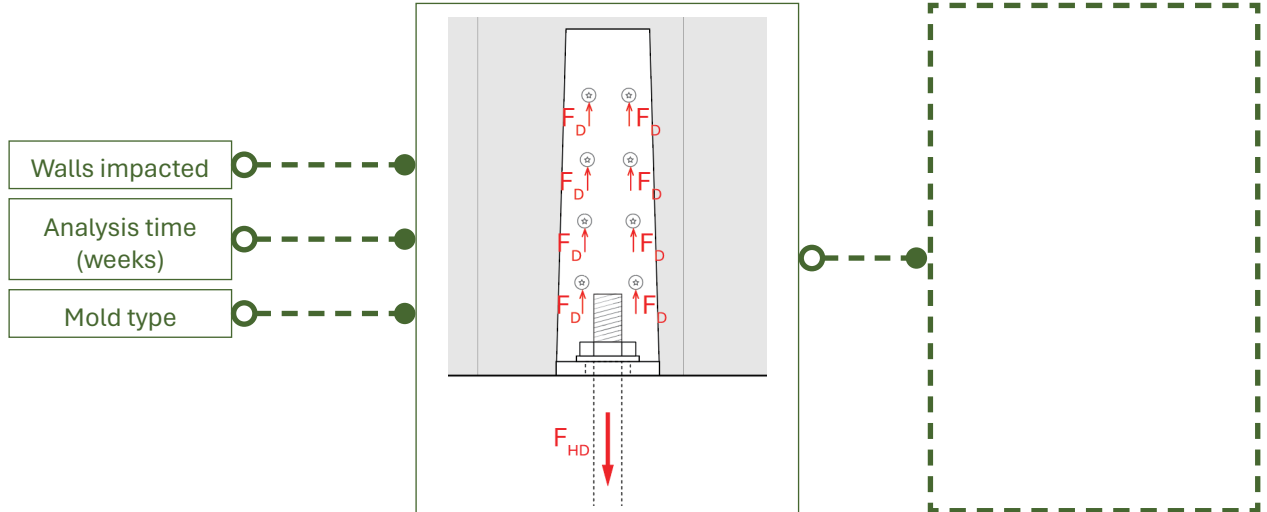
$$F_{dbs_{NS}} = -0.23(time) + 32.7$$

$$F_{dbs_{SPF}} = -0.32(time) + 33.6$$

Life Time Analysis

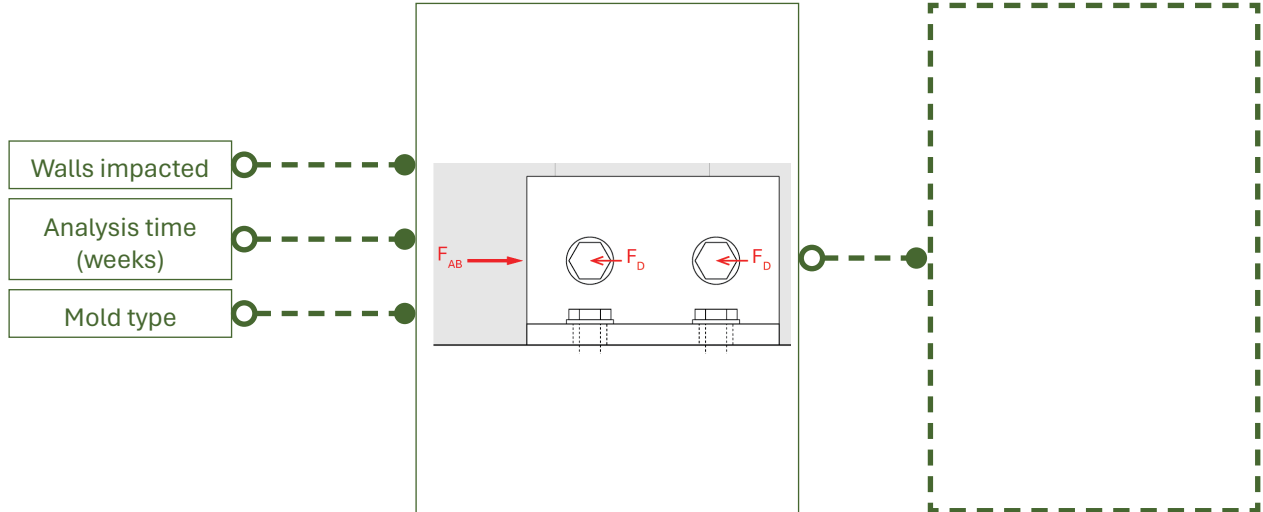


Biological Degradation - Walls over time



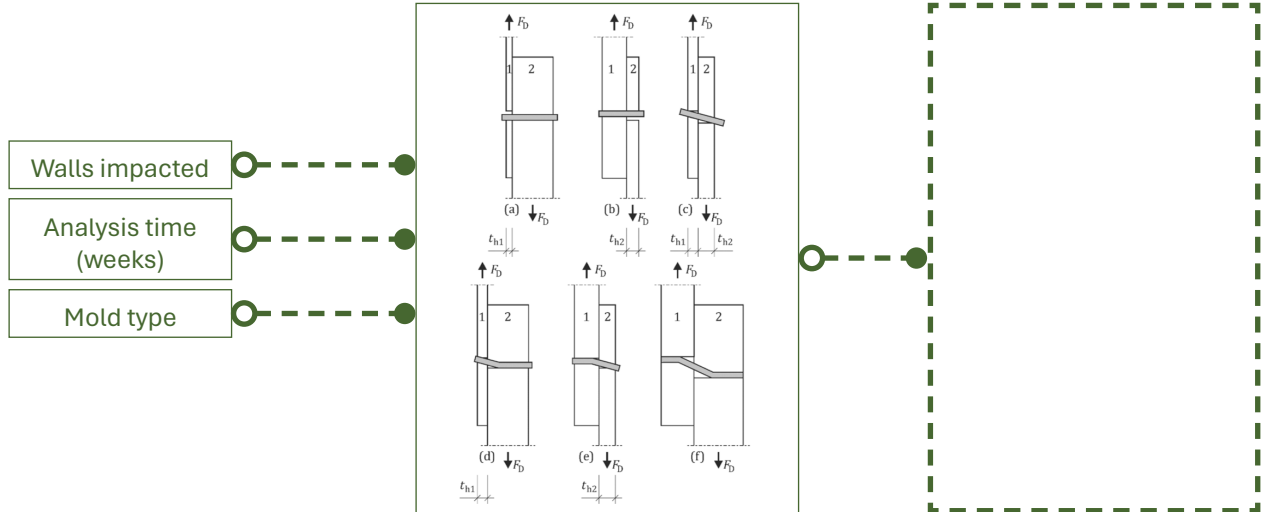
Life Time Analysis

Biological Degradation - Walls over time



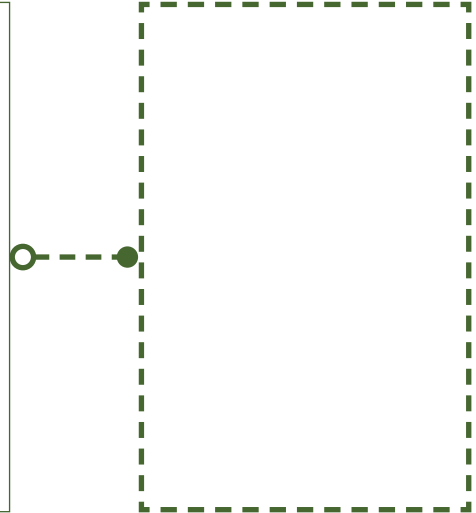
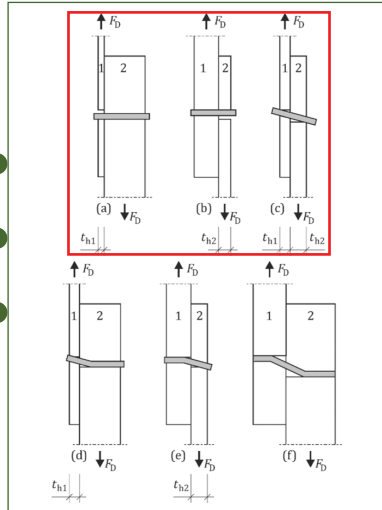
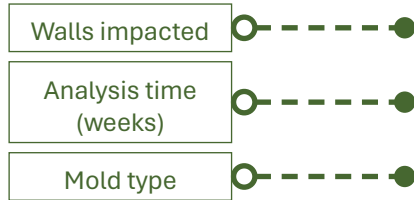
Life Time Analysis

Biological Degradation - Walls over time



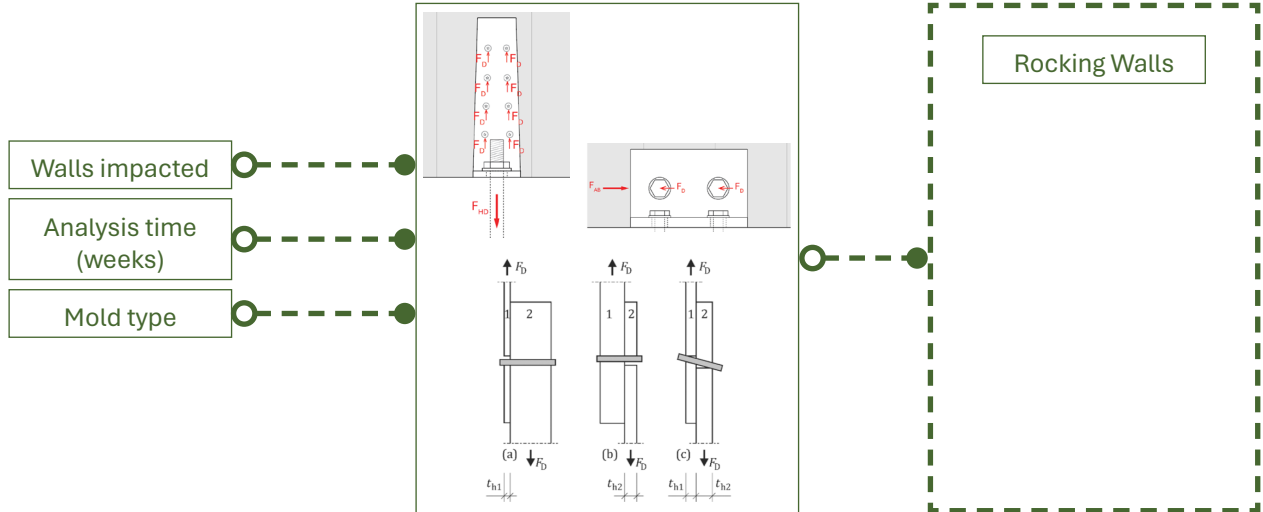
Life Time Analysis

Biological Degradation - Walls over time



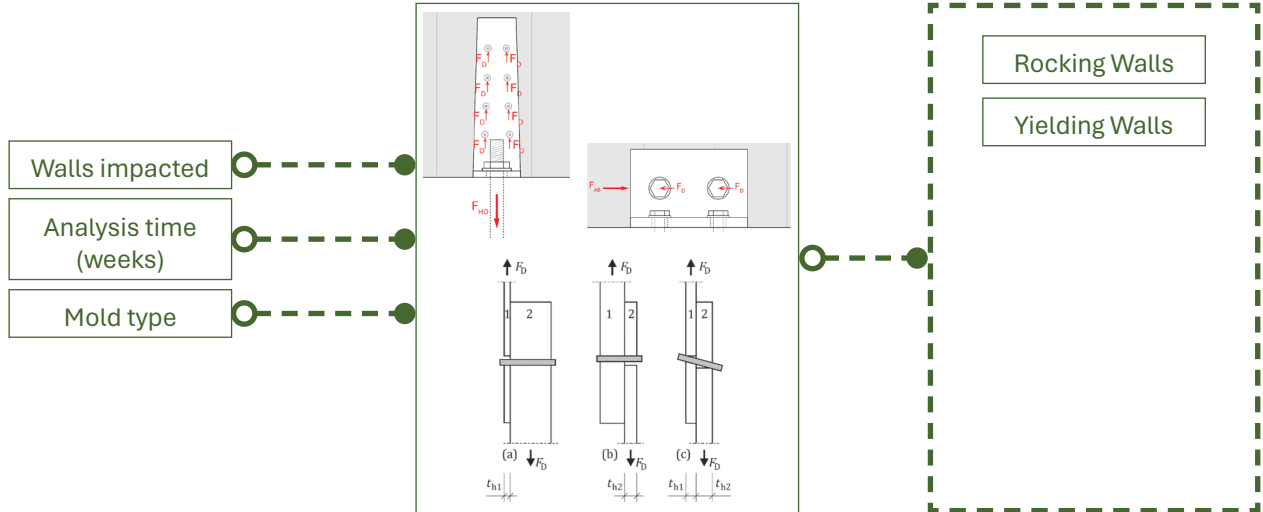
Life Time Analysis

Biological Degradation - Walls over time



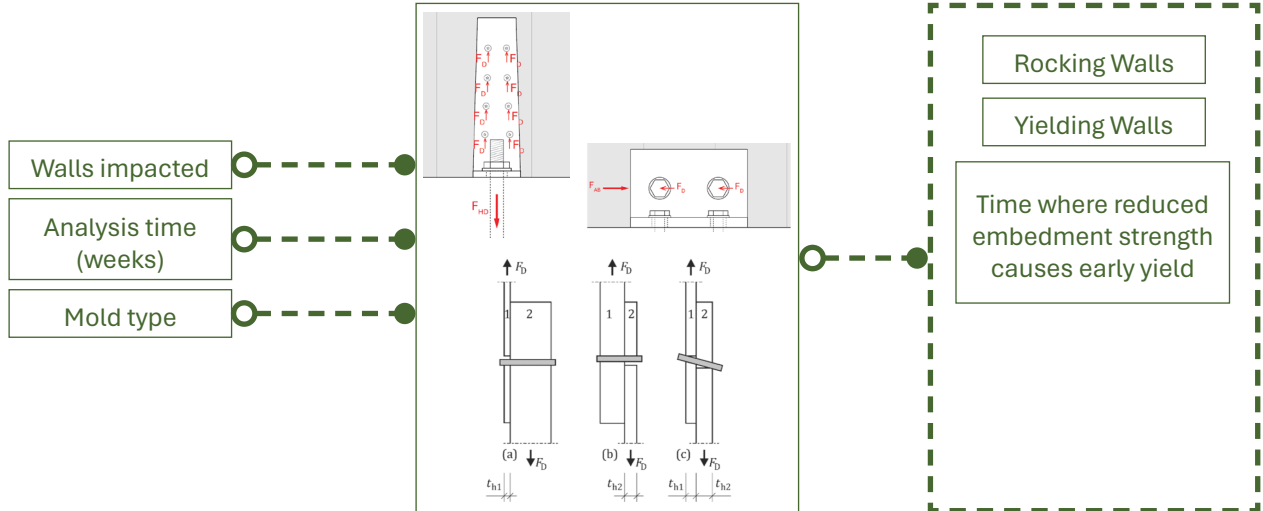
Life Time Analysis

Biological Degradation - Walls over time



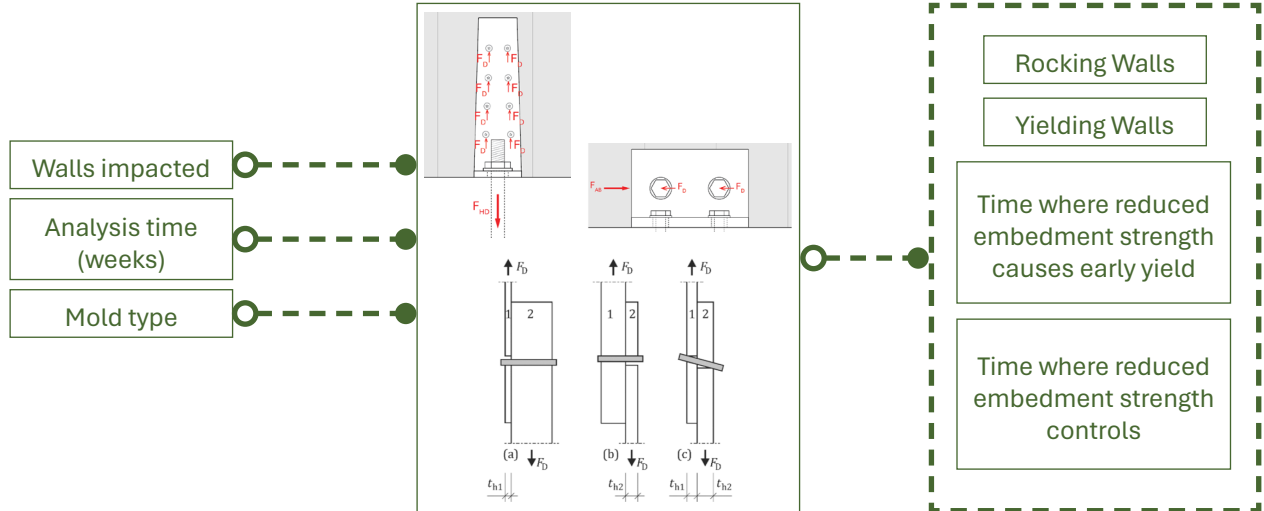
Life Time Analysis

Biological Degradation - Walls over time



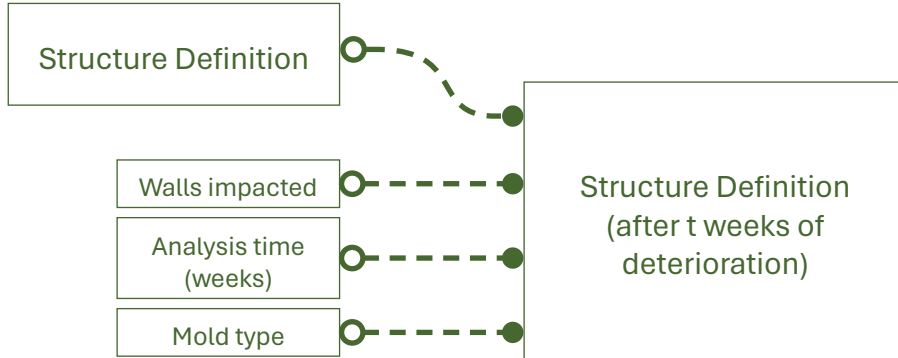
Life Time Analysis

Biological Degradation - Walls over time



Life Time Analysis

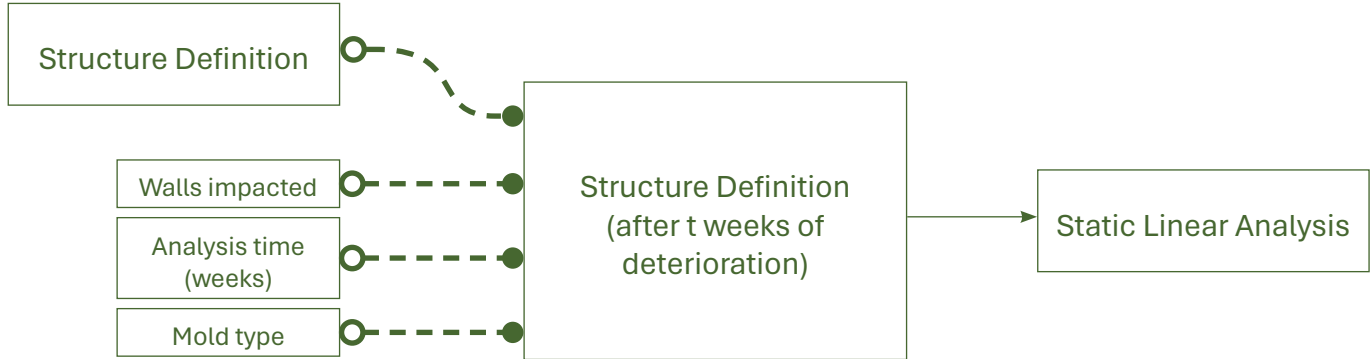
Biological Degradation - Structure at time t



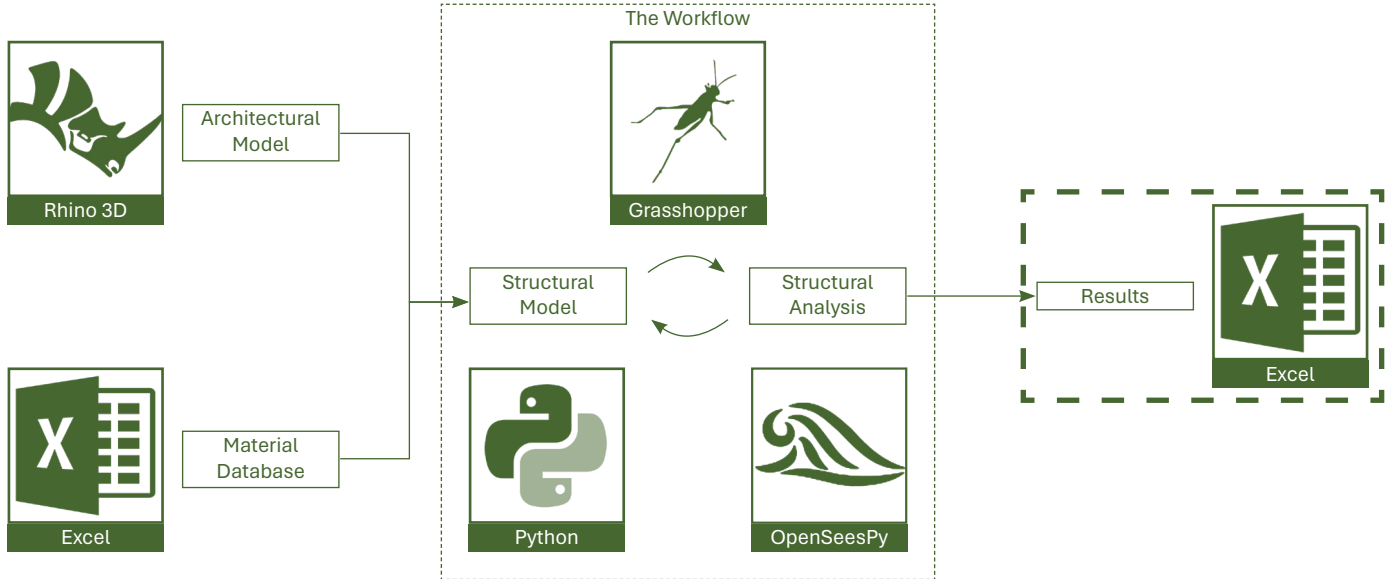
Life Time Analysis



Biological Degradation - Structure at time t



Programs and Workflow



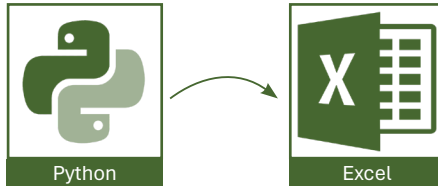
Results

Automatically Exported to Excel for Post-Processing



Results

Automatically Exported to Excel for Post-Processing



Results

Automatically Exported to Excel for Post-Processing

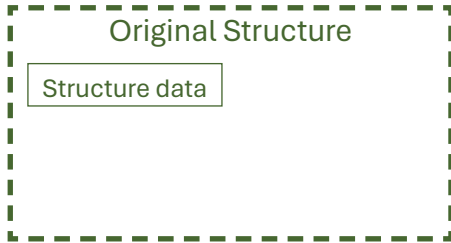


Original Structure

Results



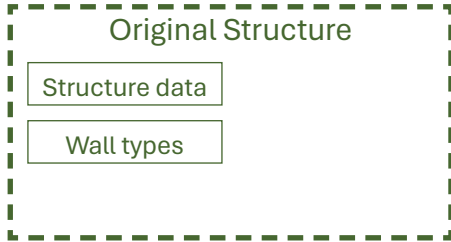
Automatically Exported to Excel for Post-Processing



Results



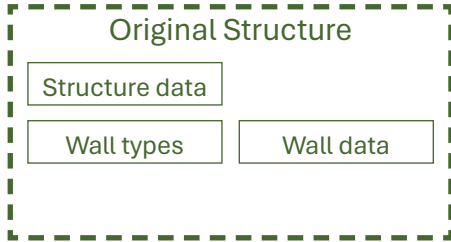
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Results



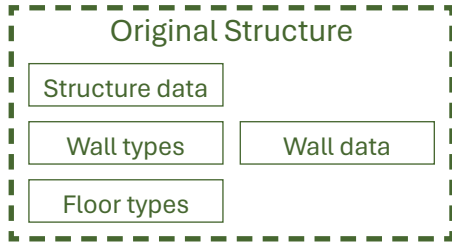
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Results



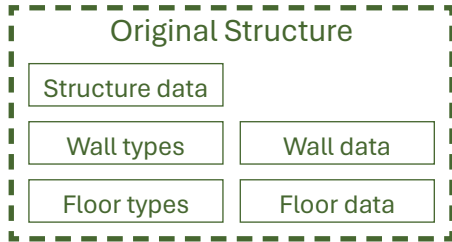
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Results



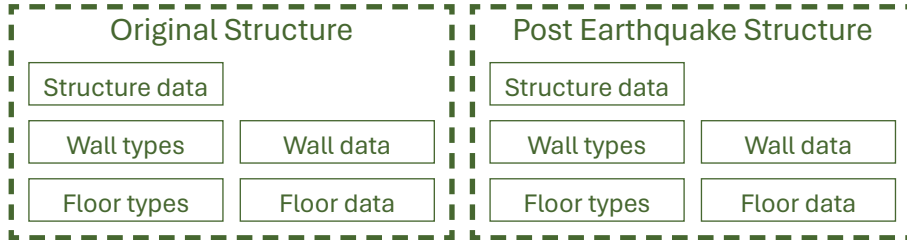
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Results



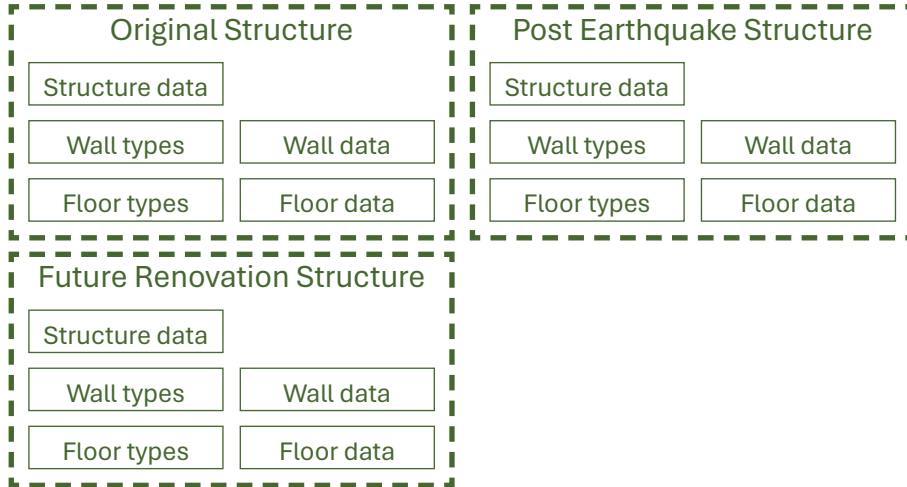
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Results



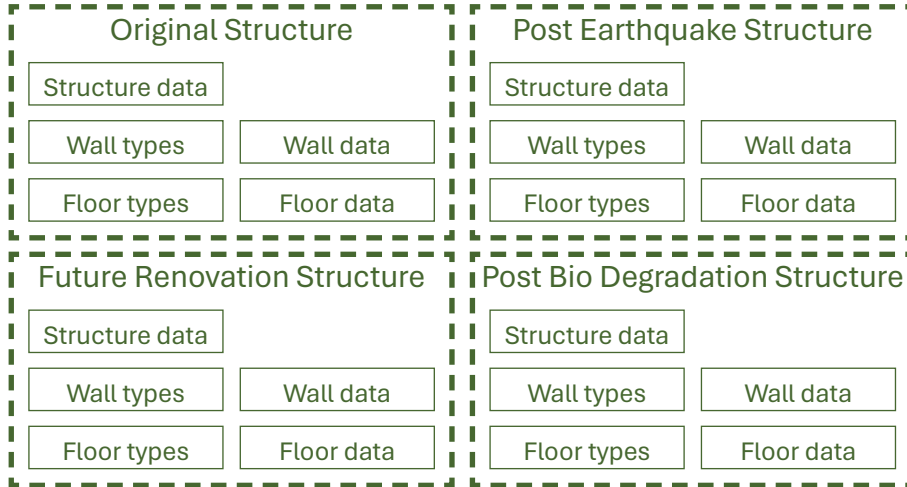
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Results



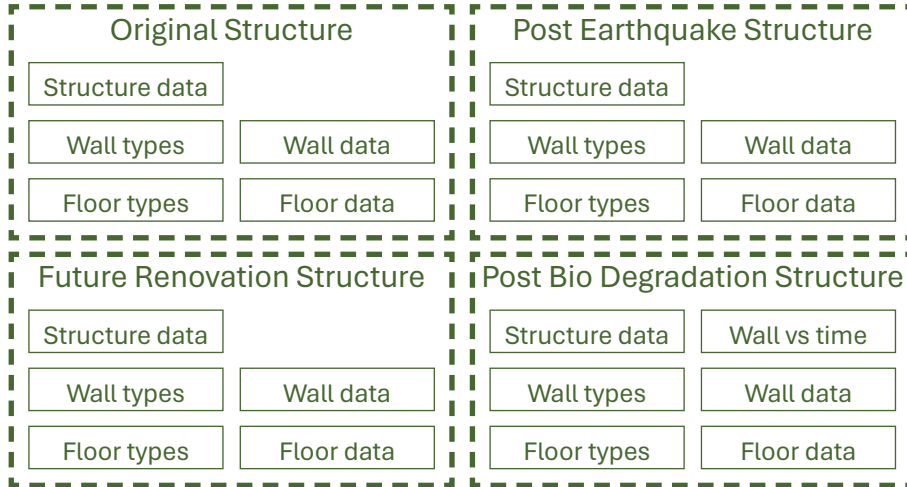
Automatically Exported to Excel for Post-Processing



Results



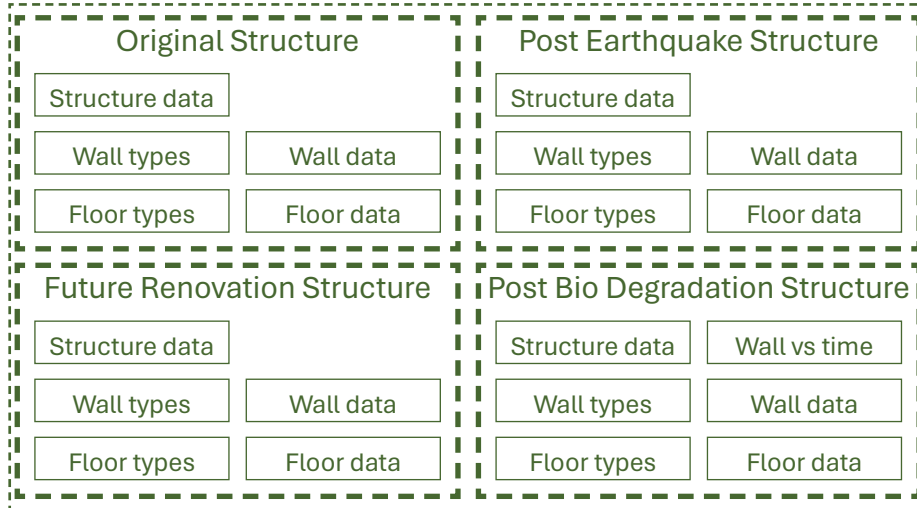
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Results



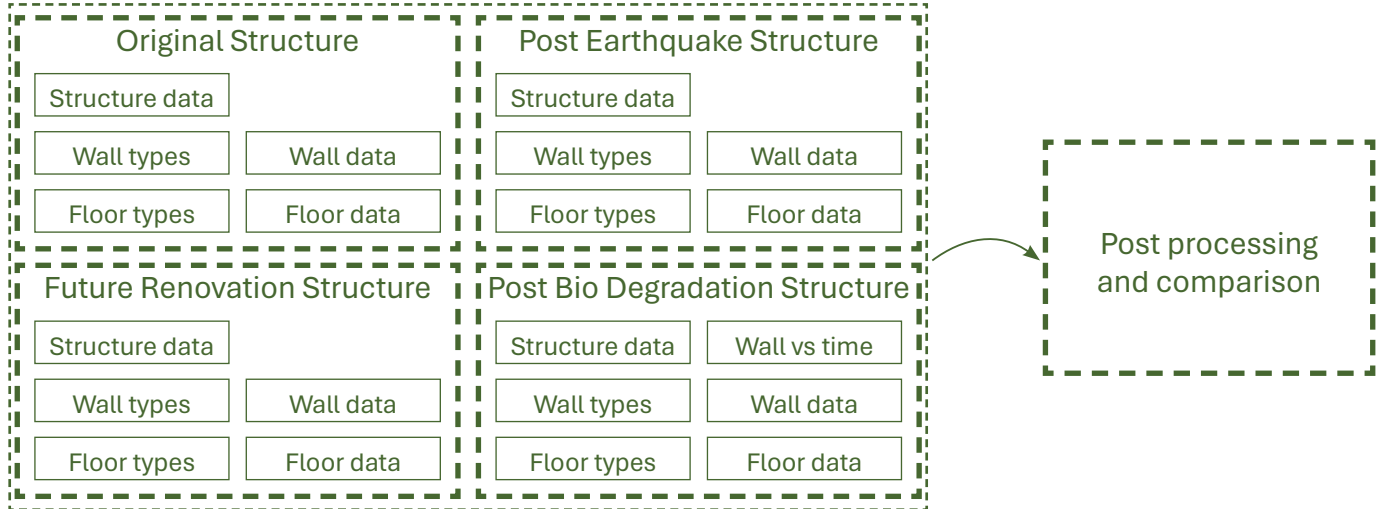
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Results

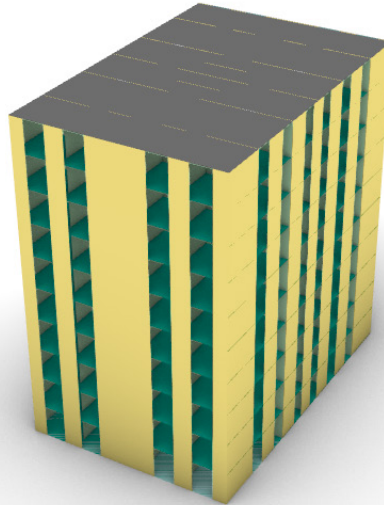


Automatically Exported to Excel for Post-Processing



Case Study

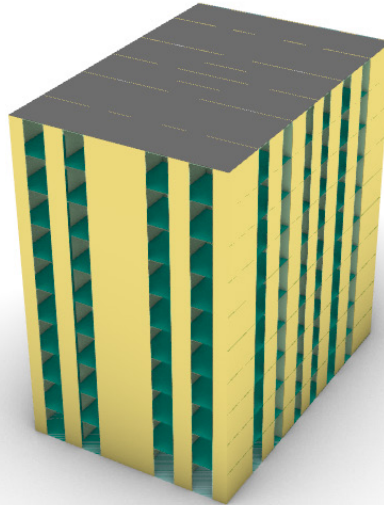
Case Study Parameters



Case Study Parameters



Rectilinear



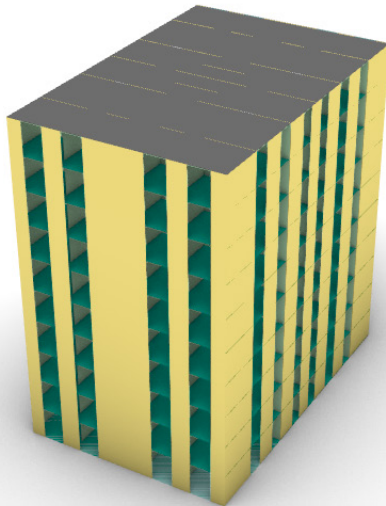
Case Study Parameters



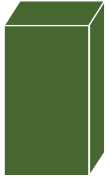
Rectilinear



Residential



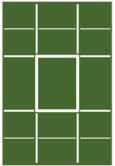
Case Study Parameters



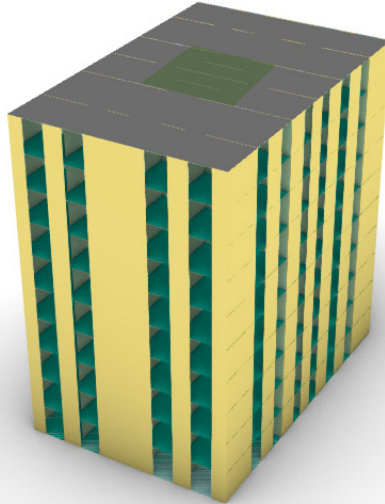
Rectilinear



Residential



Central Core



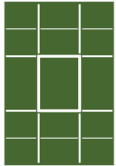
Case Study Parameters



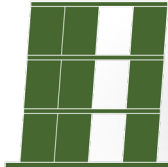
Rectilinear



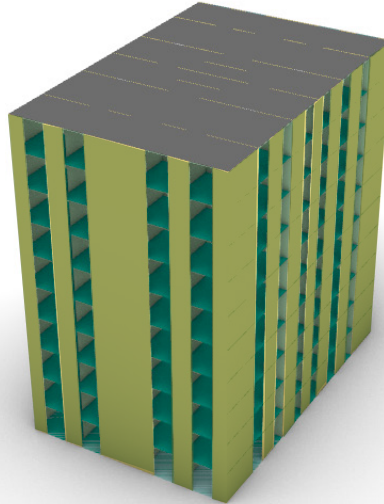
Residential



Central Core



Shear Walls



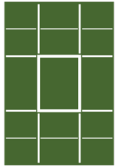
Case Study Parameters



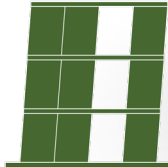
Rectilinear



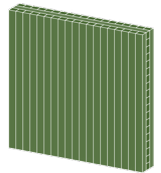
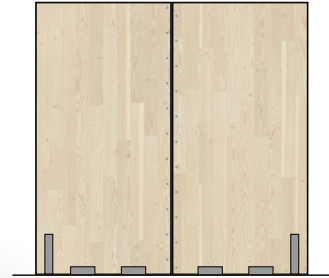
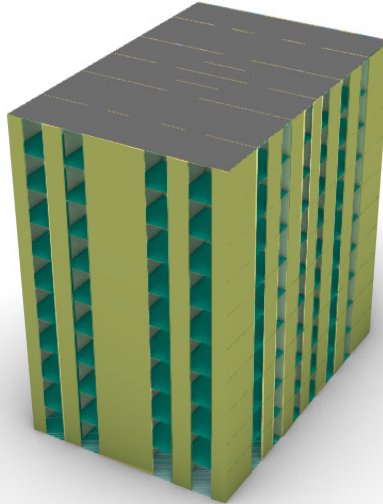
Residential



Central Core



Shear Walls



CLT Walls

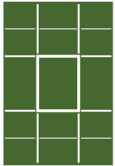
Case Study Parameters



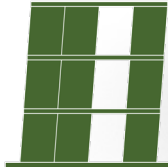
Rectilinear



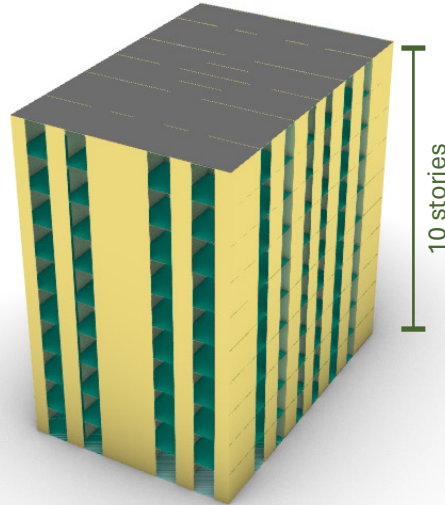
Residential



Central Core



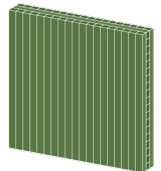
Shear Walls



10 stories



8+ stories



CLT Walls

Case Study Parameters



Europe Region

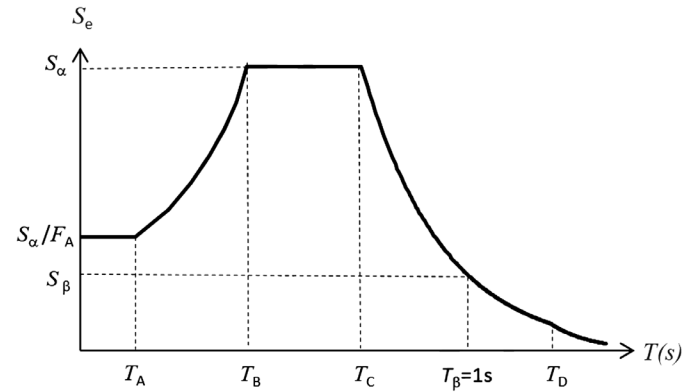
Case Study Parameters



Case Study Parameters



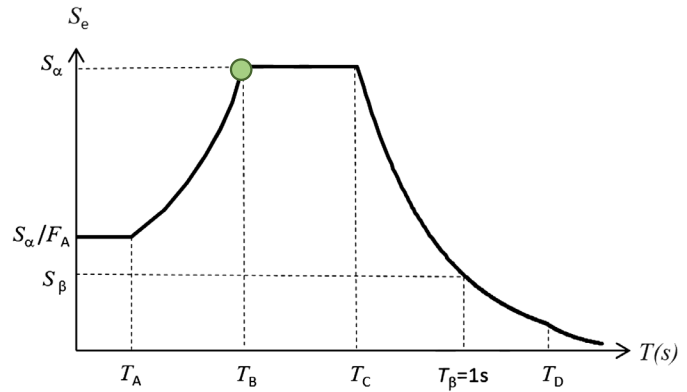
Case Study Parameters



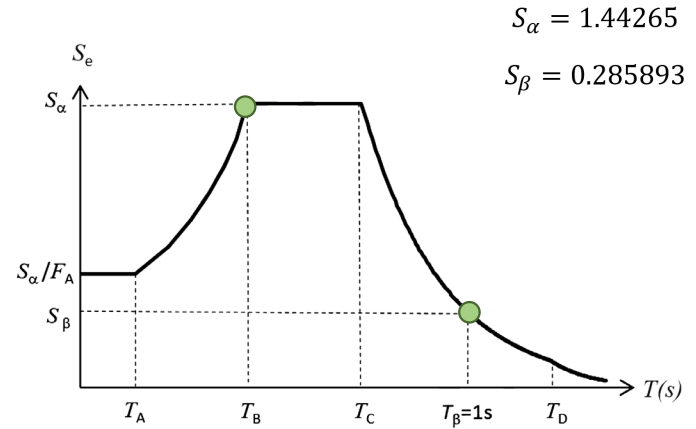
Case Study Parameters



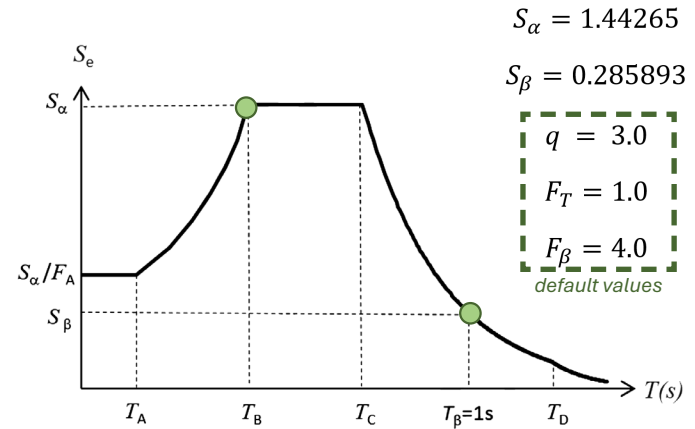
$$S_\alpha = 1.44265$$



Case Study Parameters

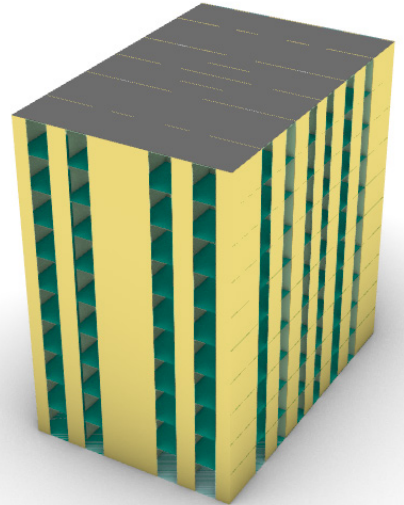


Case Study Parameters



Original Structure

Structure Parameters

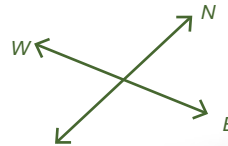


Original Structure

Structure Parameters

- Building period

$$T_{EW} = 0.0673 \text{ sec}$$



$$T_{NS} = 0.0829 \text{ sec}$$

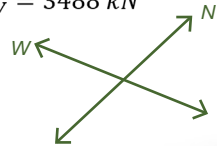
Original Structure

Structure Parameters

- Building period
- Base Shear

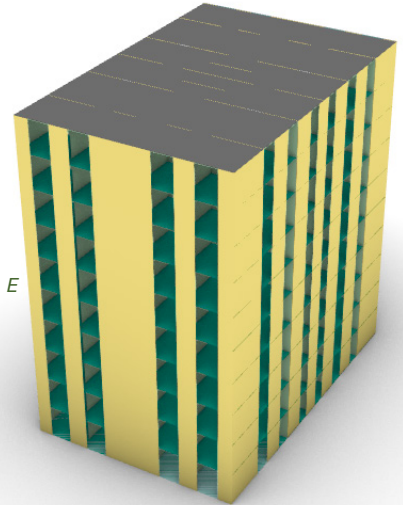
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3488 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$

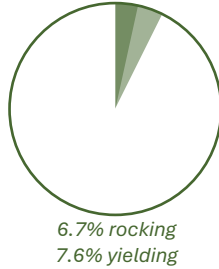


Original Structure

Structure Parameters

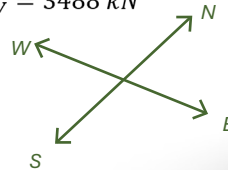
- Building period
- Base Shear

Rocking and Yielding Walls



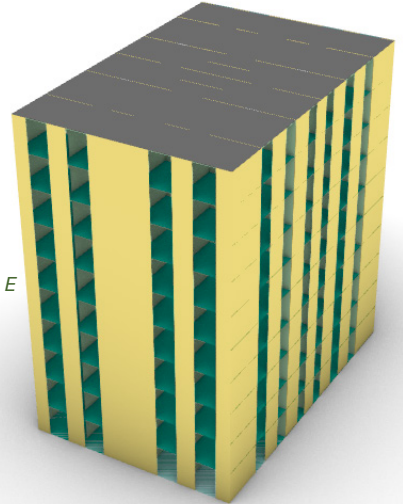
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3488 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



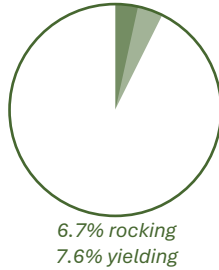
Original Structure

Structure Parameters

- Building period
- Base Shear

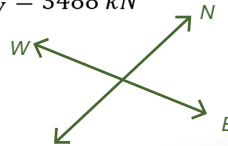
Rocking and Yielding Walls

- Only on top two stories
- Not enough weight to resist overturning moment



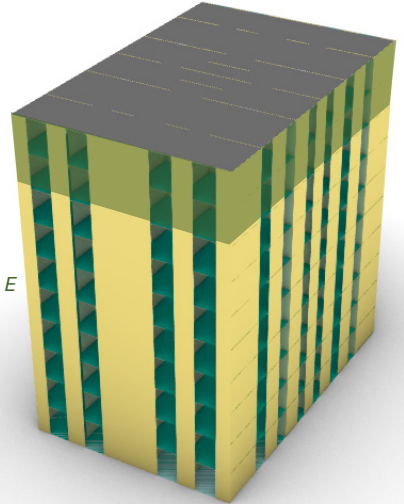
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3488 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



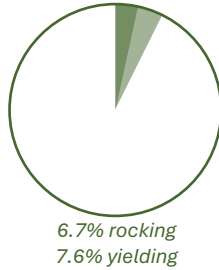
Original Structure

Structure Parameters

- Building period
- Base Shear

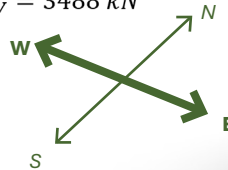
Rocking and Yielding Walls

- Only on top two stories
 - Not enough weight to resist overturning moment
- East-west direction only



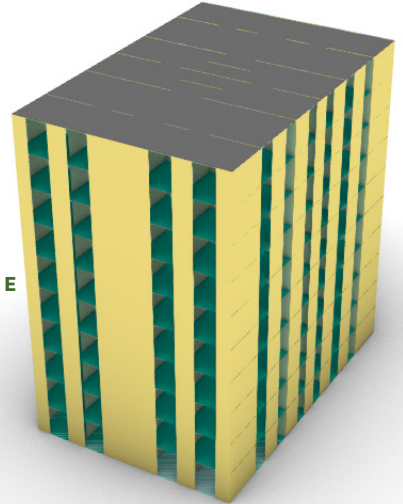
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3488 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



Original Structure

Structure Parameters

- Building period
- Base Shear

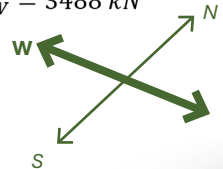
Rocking and Yielding Walls

- Only on top two stories
- Not enough weight to resist overturning moment
- East-west direction only
- Yielding in from shear, not overturning moment



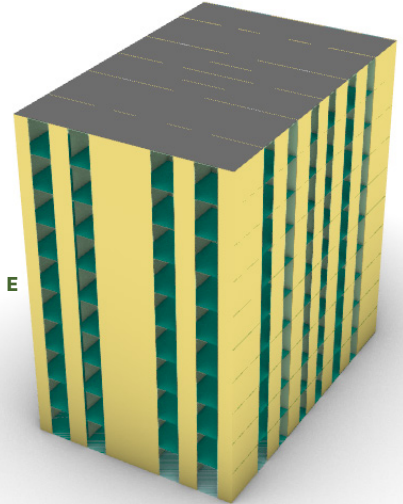
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3488 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

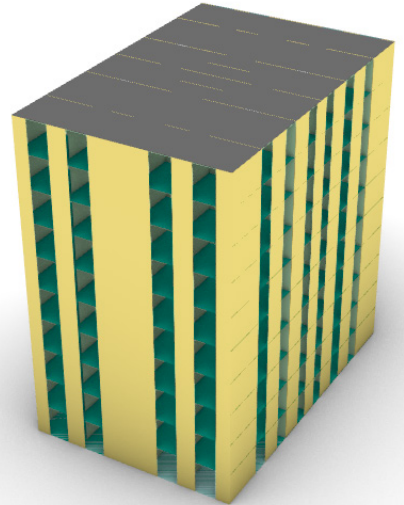
$$V_{NS} = 3513 \text{ kN}$$



Post Earthquake

Change

- Stiffness reduction
 - Rocking walls: 80%
 - Yielded walls: 50%



Post Earthquake

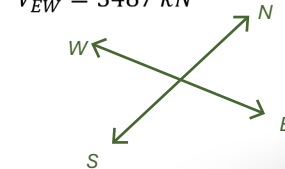
Change

- Stiffness reduction
 - Rocking walls: 80%
 - Yielded walls: 50%

Structure Parameters

$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$

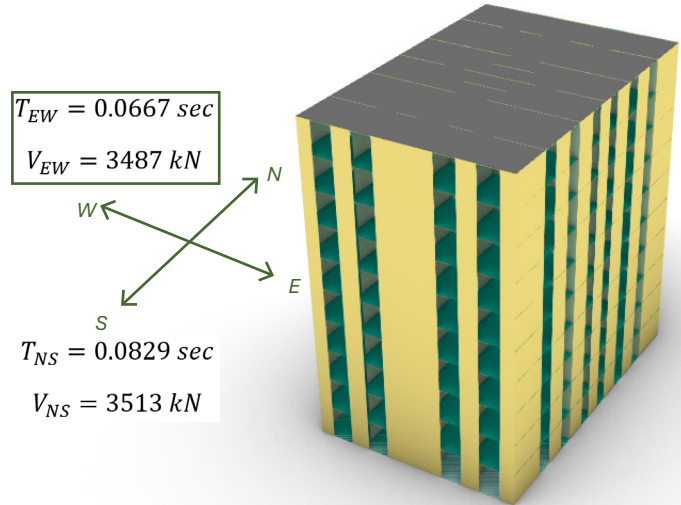
Post Earthquake

Change

- Stiffness reduction
 - Rocking walls: 80%
 - Yielded walls: 50%

Structure Parameters

- Slight decrease in EW



Post Earthquake

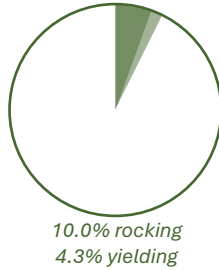
Change

- Stiffness reduction
 - Rocking walls: 80%
 - Yielded walls: 50%

Structure Parameters

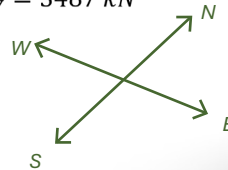
- Slight decrease in EW

Rocking and Yielding Walls



$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$

Post Earthquake

Change

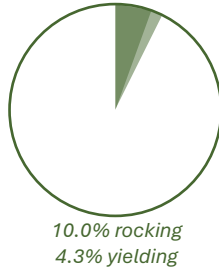
- Stiffness reduction
 - Rocking walls: 80%
 - Yielded walls: 50%

Structure Parameters

- Slight decrease in EW

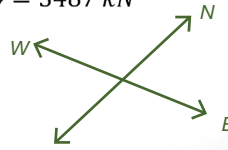
Rocking and Yielding Walls

- Only on top two stories



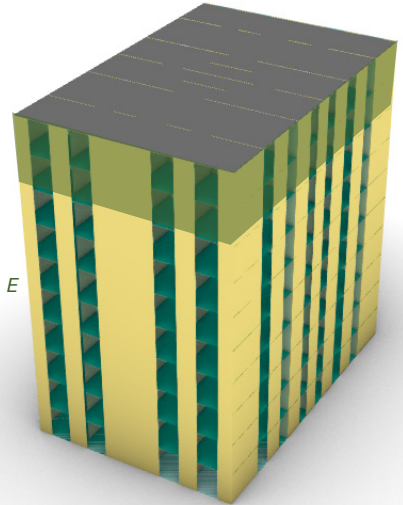
$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



Post Earthquake

Change

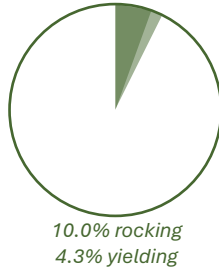
- Stiffness reduction
 - Rocking walls: 80%
 - Yielded walls: 50%

Structure Parameters

- Slight decrease in EW

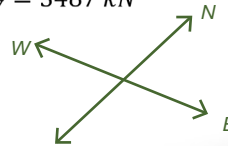
Rocking and Yielding Walls

- Only on top two stories
 - More in 9th story now



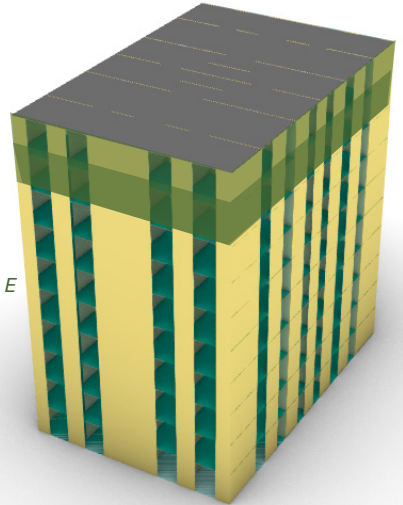
$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



Post Earthquake

Change

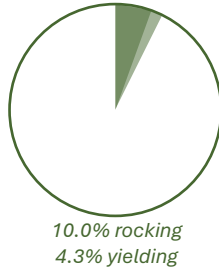
- Stiffness reduction
 - Rocking walls: 80%
 - Yielded walls: 50%

Structure Parameters

- Slight decrease in EW

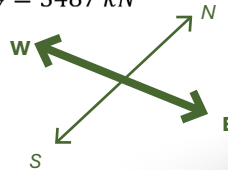
Rocking and Yielding Walls

- Only on top two stories
 - More in 9th story now
- East-west direction only



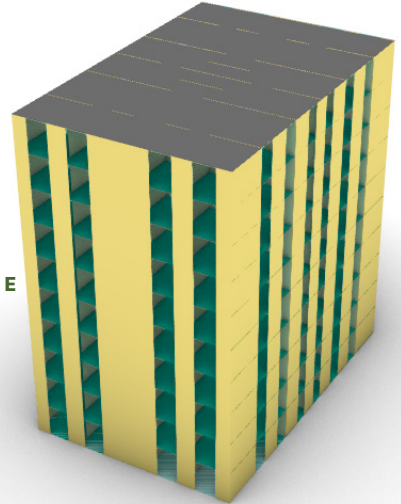
$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



Post Earthquake

Change

- Stiffness reduction
 - Rocking walls: 80%
 - Yielded walls: 50%

Structure Parameters

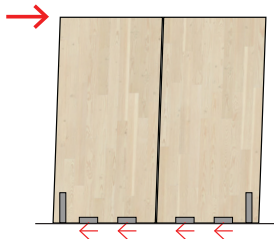
- Slight decrease in EW

Rocking and Yielding Walls

- Only on top two stories
 - More in 9th story now
- East-west direction only
- Yielding in from shear, not overturning moment

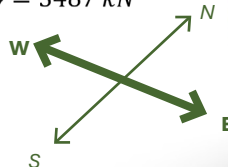


10.0% rocking
4.3% yielding



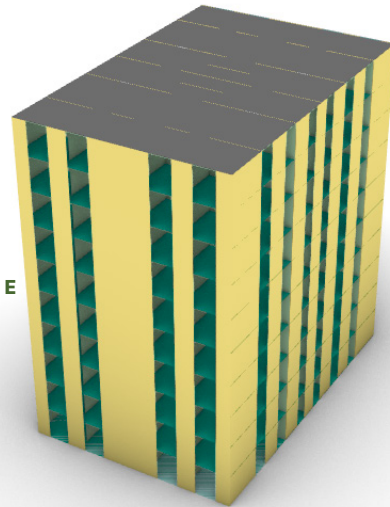
$$T_{EW} = 0.0667 \text{ sec}$$

$$V_{EW} = 3487 \text{ kN}$$



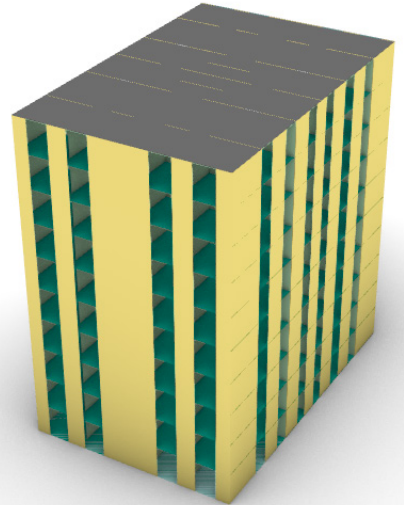
$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3513 \text{ kN}$$



Renovation

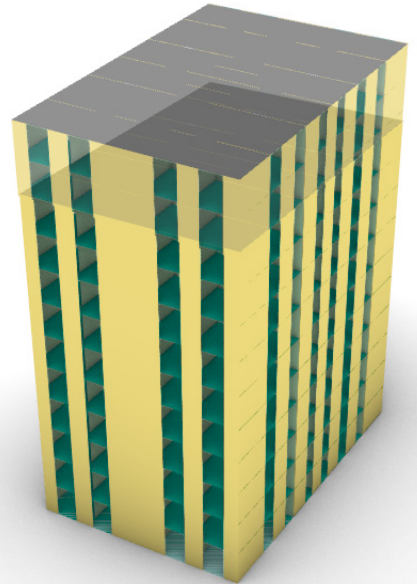
Change



Renovation

Change

- Add two new stories

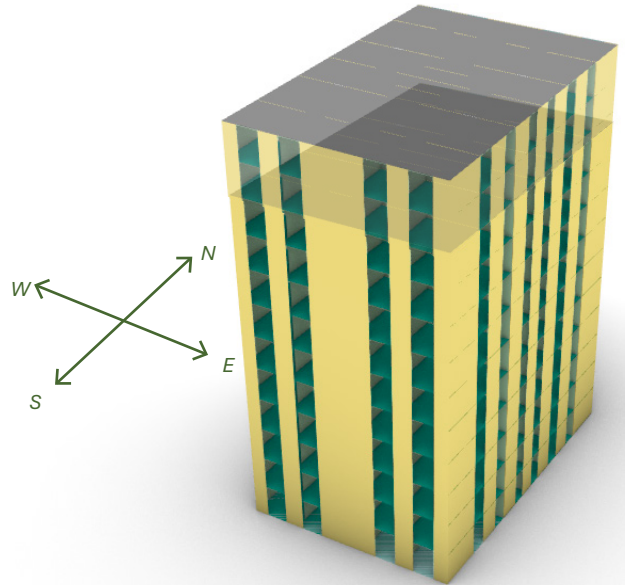


Renovation

Change

- Add two new stories

Structure Parameters



Renovation

Change

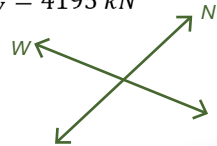
- Add two new stories

Structure Parameters

- Building period is similar
- Base shear increases

$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 4195 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 4225 \text{ kN}$$

Renovation

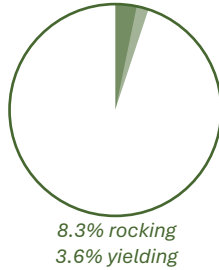
Change

- Add two new stories

Structure Parameters

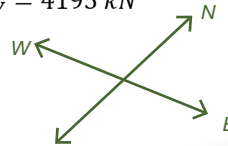
- Building period is similar
- Base shear increases

Rocking and Yielding Walls



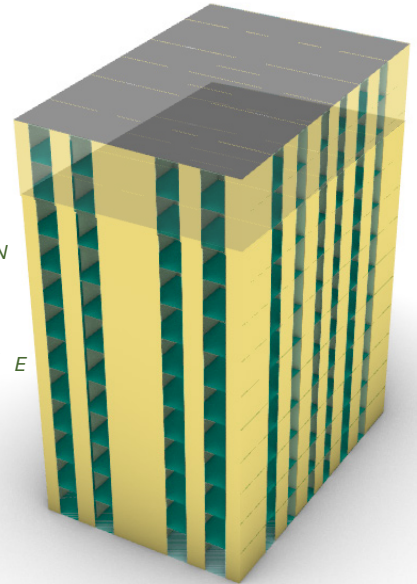
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 4195 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 4225 \text{ kN}$$



Renovation

Change

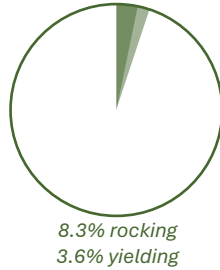
- Add two new stories

Structure Parameters

- Building period is similar
- Base shear increases

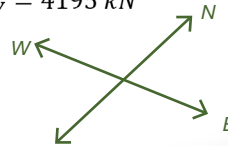
Rocking and Yielding Walls

- Only in top two stories



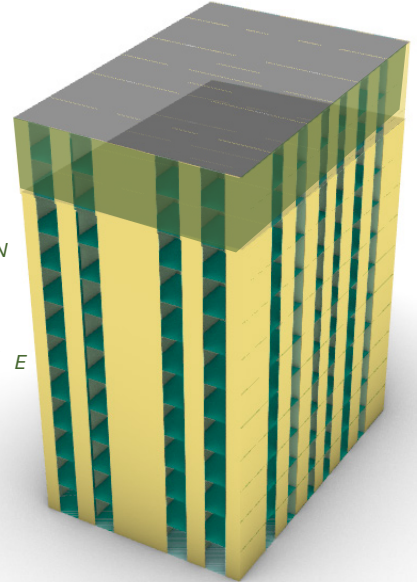
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 4195 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 4225 \text{ kN}$$



Renovation

Change

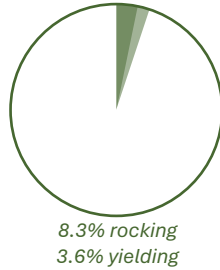
- Add two new stories

Structure Parameters

- Building period is similar
- Base shear increases

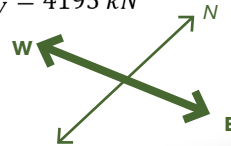
Rocking and Yielding Walls

- Only in top two stories
- East-west direction only



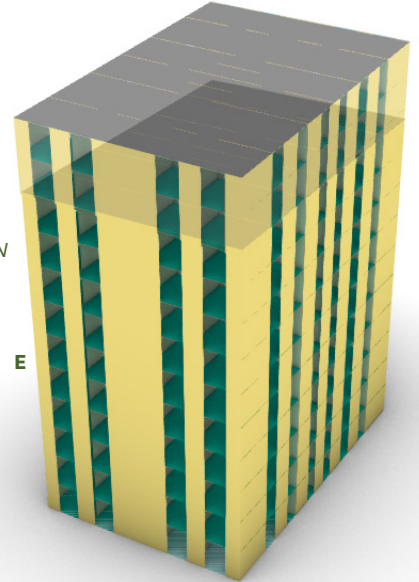
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 4195 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 4225 \text{ kN}$$



Renovation

Change

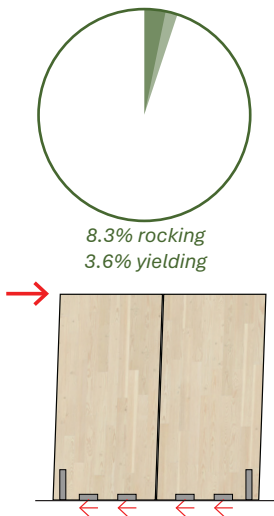
- Add two new stories

Structure Parameters

- Building period is similar
- Base shear increases

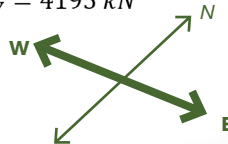
Rocking and Yielding Walls

- Only in top two stories
- East-west direction only
- Yielding in from shear in angle brackets



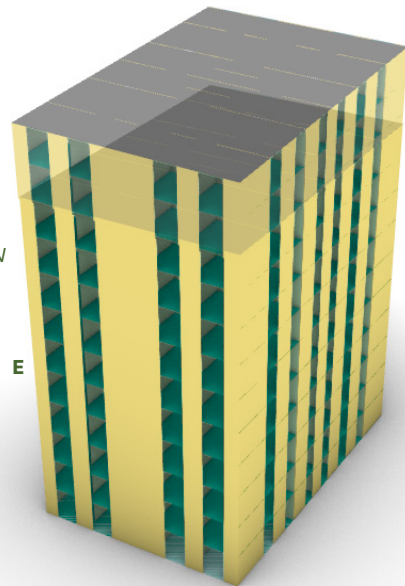
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 4195 \text{ kN}$$



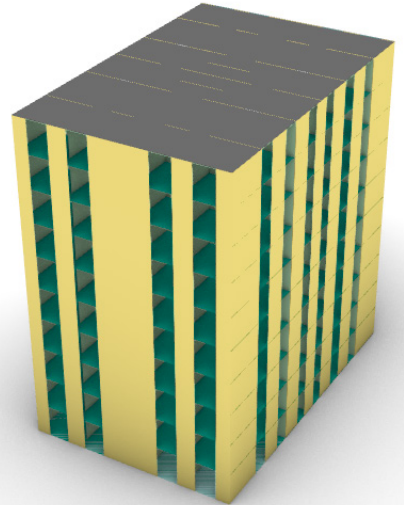
$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 4225 \text{ kN}$$



Biological Degradation

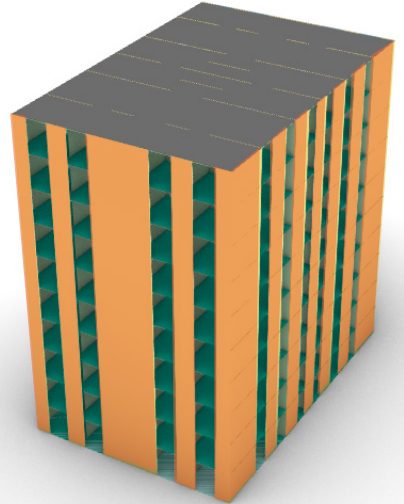
Change



Biological Degradation

Change

- 52 weeks of deterioration at exterior walls

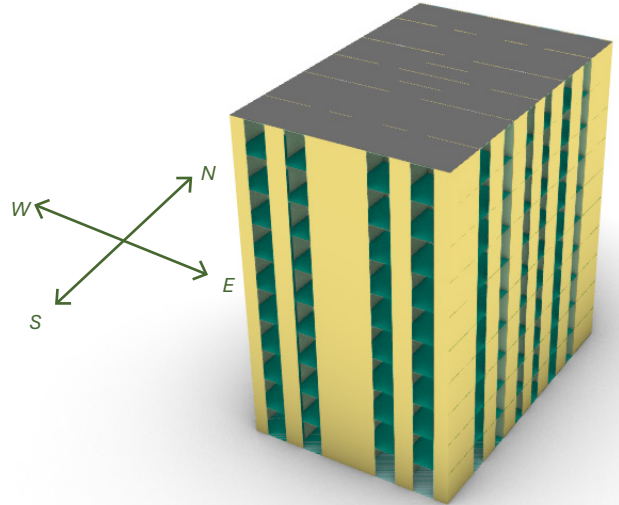


Biological Degradation

Change

- 52 weeks of deterioration at exterior walls

Structure Parameters



Biological Degradation

Change

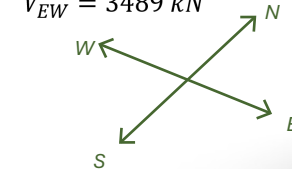
- 52 weeks of deterioration at exterior walls

Structure Parameters

- Building period and base shear remain the same

$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3489 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3514 \text{ kN}$$

Biological Degradation

Change

- 52 weeks of deterioration at exterior walls

Structure Parameters

- Building period and base shear remain the same

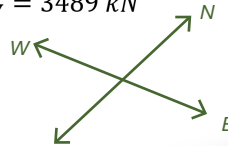
Rocking and Yielding Walls



8.3% rocking
10% yielding

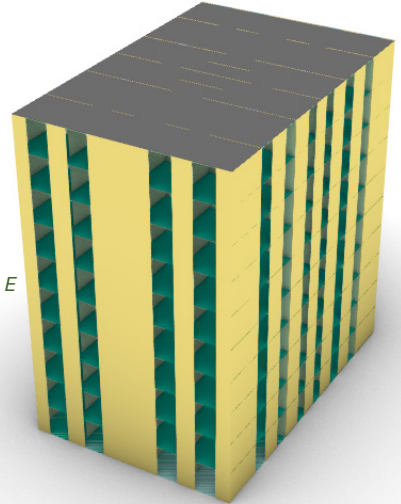
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3489 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3514 \text{ kN}$$



Biological Degradation

Change

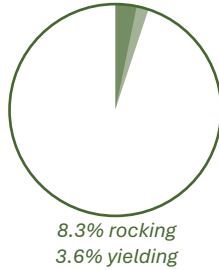
- 52 weeks of deterioration at exterior walls

Structure Parameters

- Building period and base shear remain the same

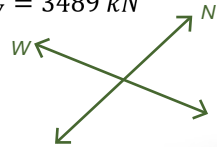
Rocking and Yielding Walls

- Stories 8-10



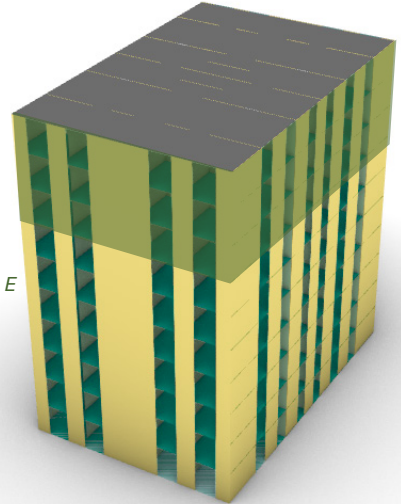
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3489 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3514 \text{ kN}$$



Biological Degradation

Change

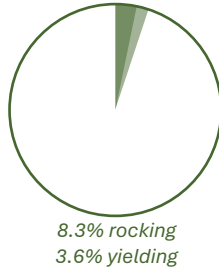
- 52 weeks of deterioration at exterior walls

Structure Parameters

- Building period and base shear remain the same

Rocking and Yielding Walls

- Stories 8-10
- East-west direction only



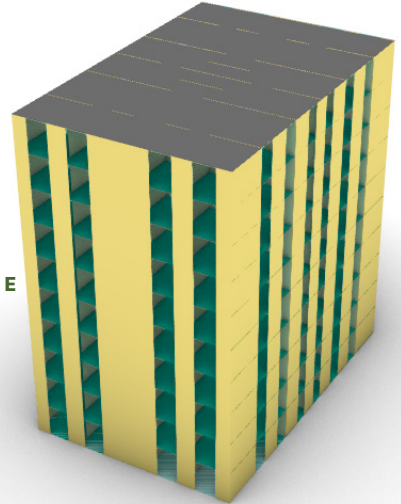
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3489 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3514 \text{ kN}$$



Biological Degradation

Change

- 52 weeks of deterioration at exterior walls

Structure Parameters

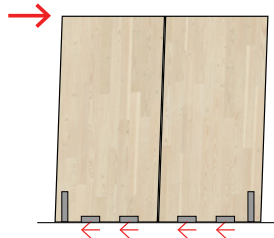
- Building period and base shear remain the same

Rocking and Yielding Walls

- Stories 8-10
- East-west direction only
- Yielding from shear in angle brackets, and



8.3% rocking
3.6% yielding



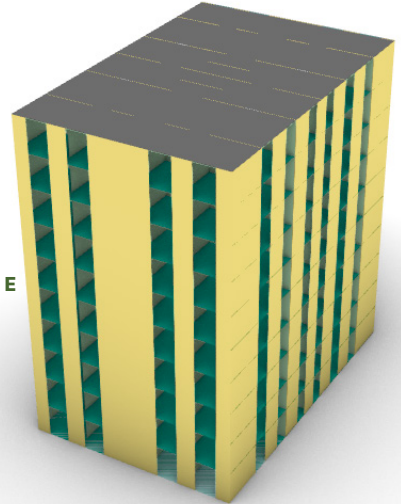
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3489 \text{ kN}$$



$$T_{NS} = 0.0829 \text{ sec}$$

$$V_{NS} = 3514 \text{ kN}$$



Biological Degradation

Change

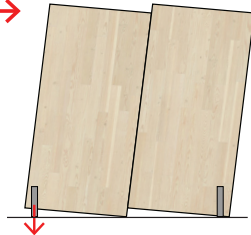
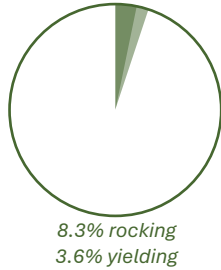
- 52 weeks of deterioration at exterior walls

Structure Parameters

- Building period and base shear remain the same

Rocking and Yielding Walls →

- Stories 8-10
- East-west direction only
- Yielding from shear in angle brackets, and from overturning in hold-downs



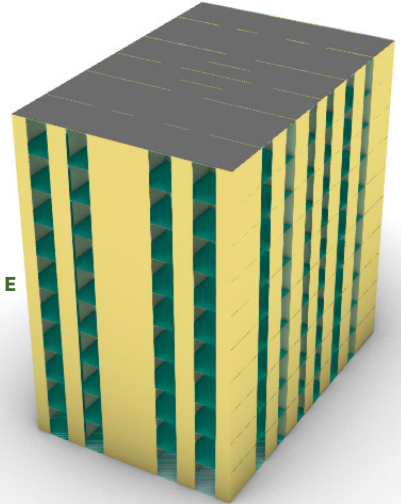
$$T_{EW} = 0.0673 \text{ sec}$$

$$V_{EW} = 3489 \text{ kN}$$

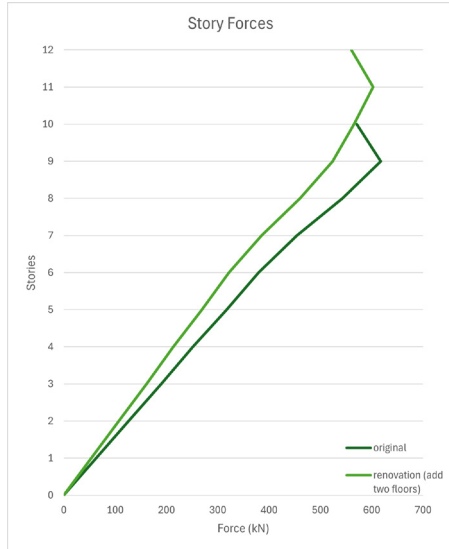


$$T_{NS} = 0.0829 \text{ sec}$$

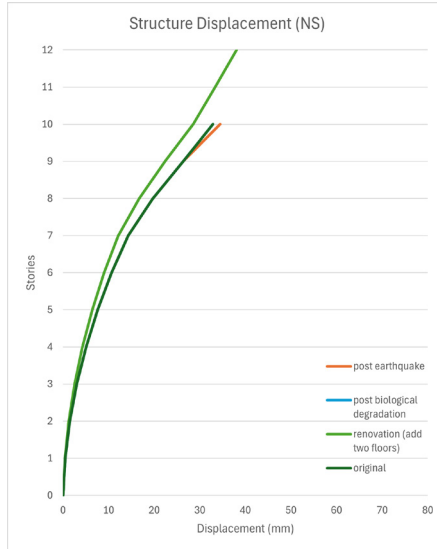
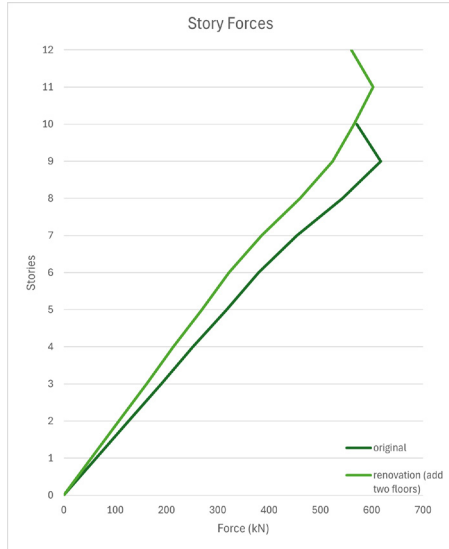
$$V_{NS} = 3514 \text{ kN}$$



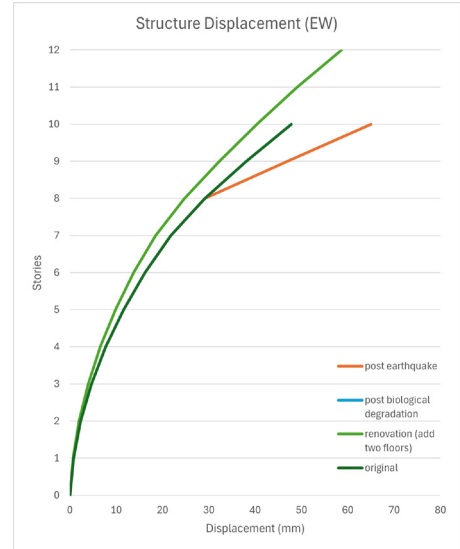
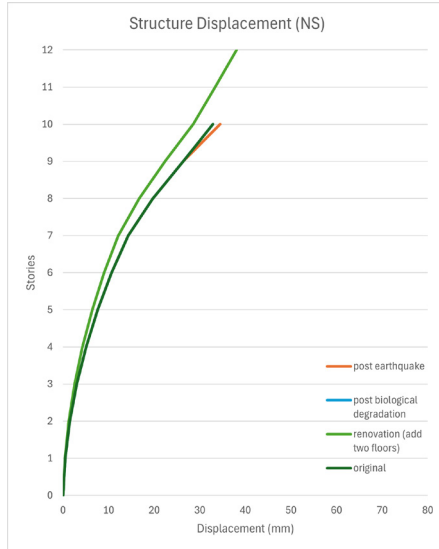
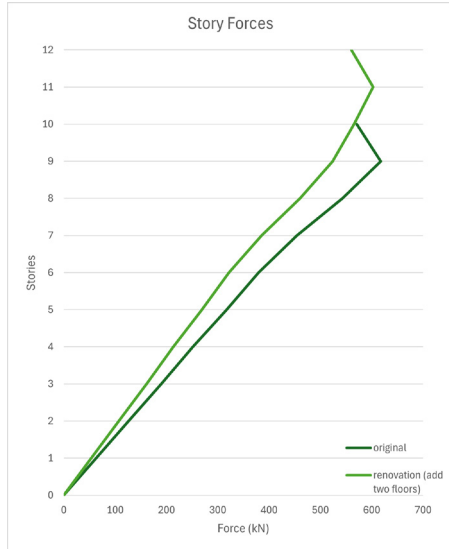
Results Comparisons



Results Comparisons



Results Comparisons

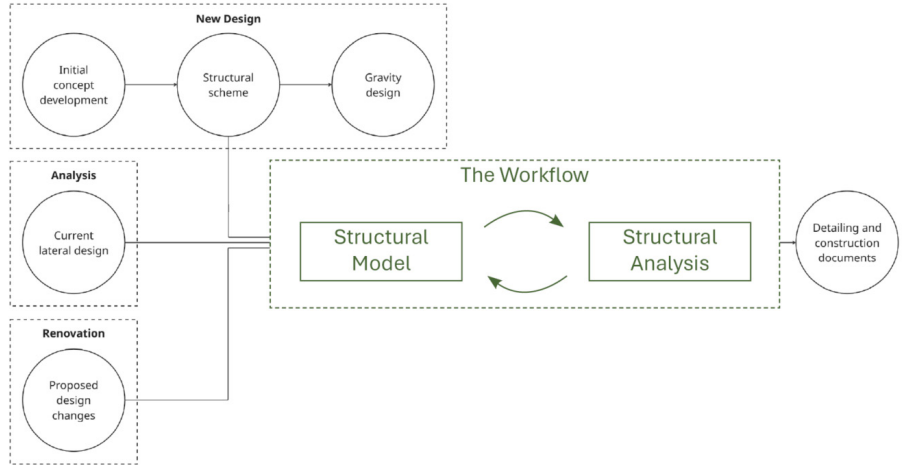


Conclusions

Conclusions

Goals

- Integrated into the main workflow



Conclusions

Goals

- Integrated into the main workflow
- Applicable for common tall timber construction

Form and Function



Rectilinear

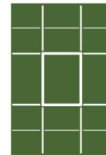


8+ stories

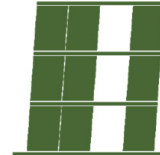


Residential

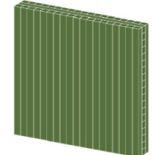
Structural System



Central Core



Shear Walls

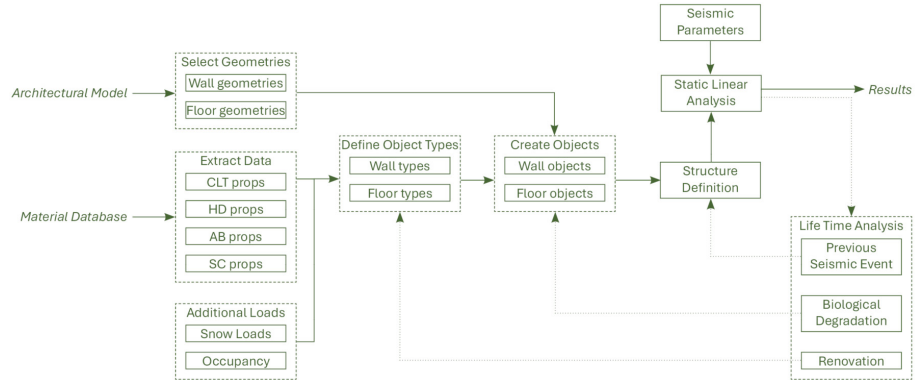


CLT Walls

Conclusions

Goals

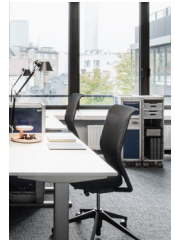
- Integrated into the main workflow
- Applicable for common tall timber construction
- Simple to implement and adaptable



Conclusions

Goals

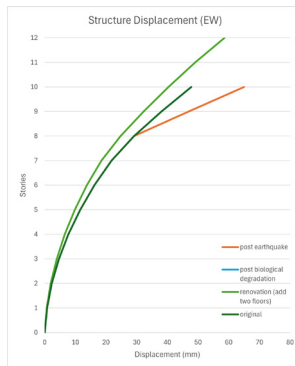
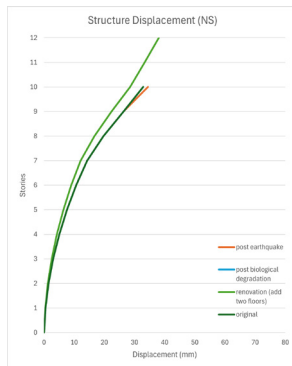
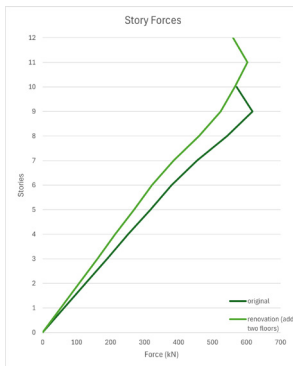
- Integrated into the main workflow
- Applicable for common tall timber construction
- Simple to implement and adaptable
- Includes lifetime analysis



Conclusions

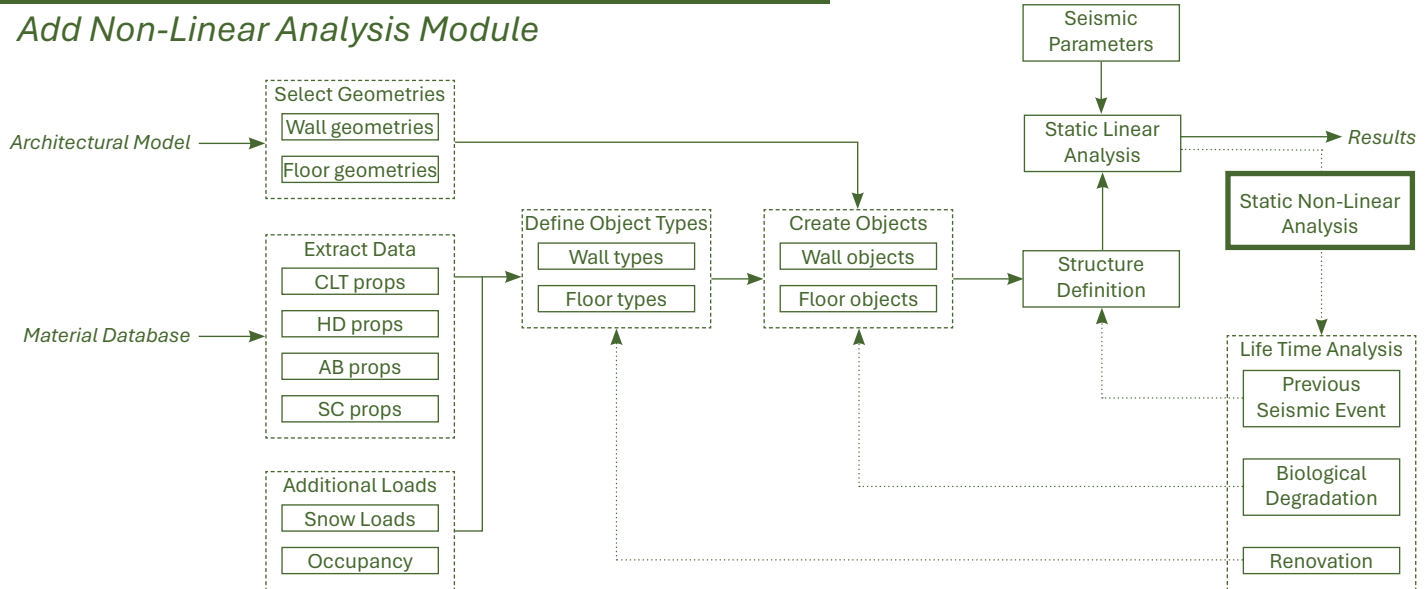
Goals

- Integrated into the main workflow
- Applicable for common tall timber construction
- Simple to implement and adaptable
- Includes lifetime analysis
- Output format is good for post-processing



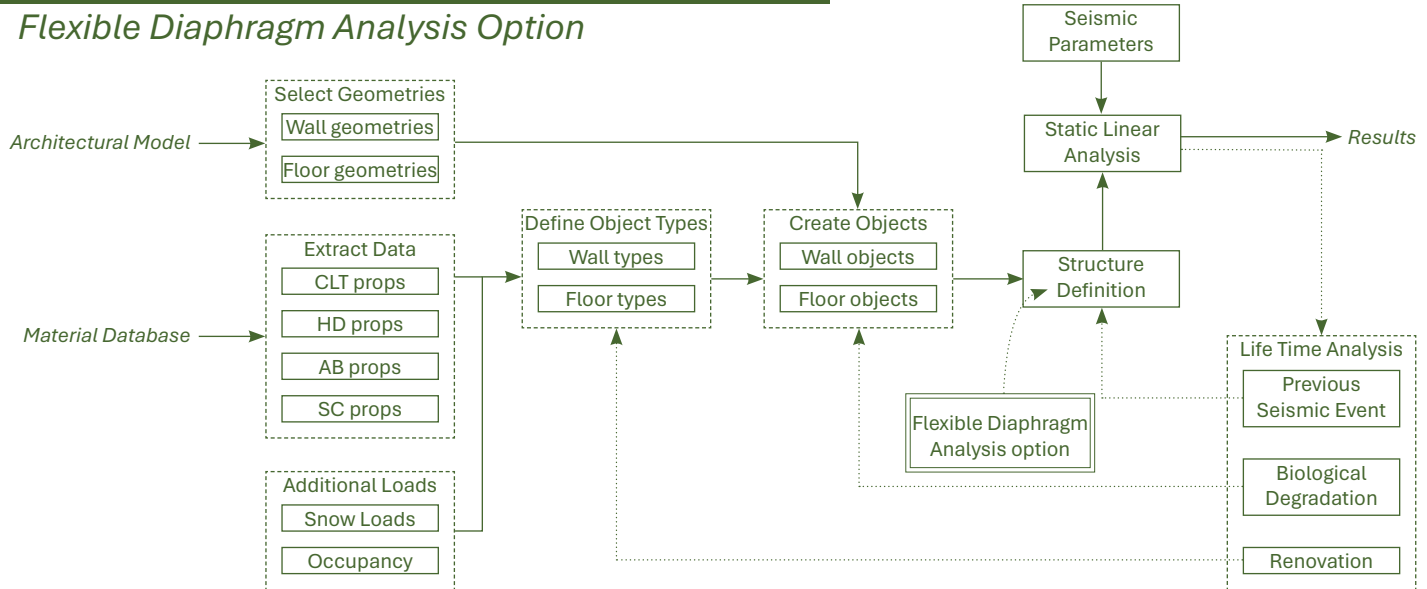
Future Development

Add Non-Linear Analysis Module



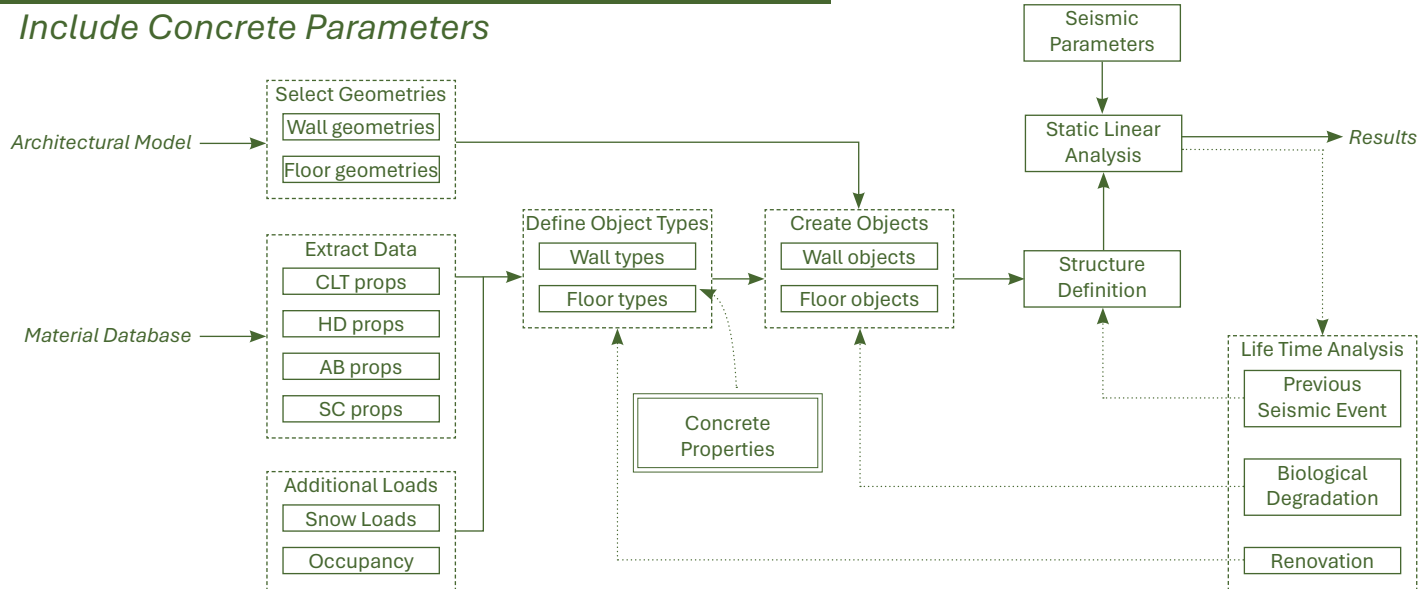
Future Development

Flexible Diaphragm Analysis Option



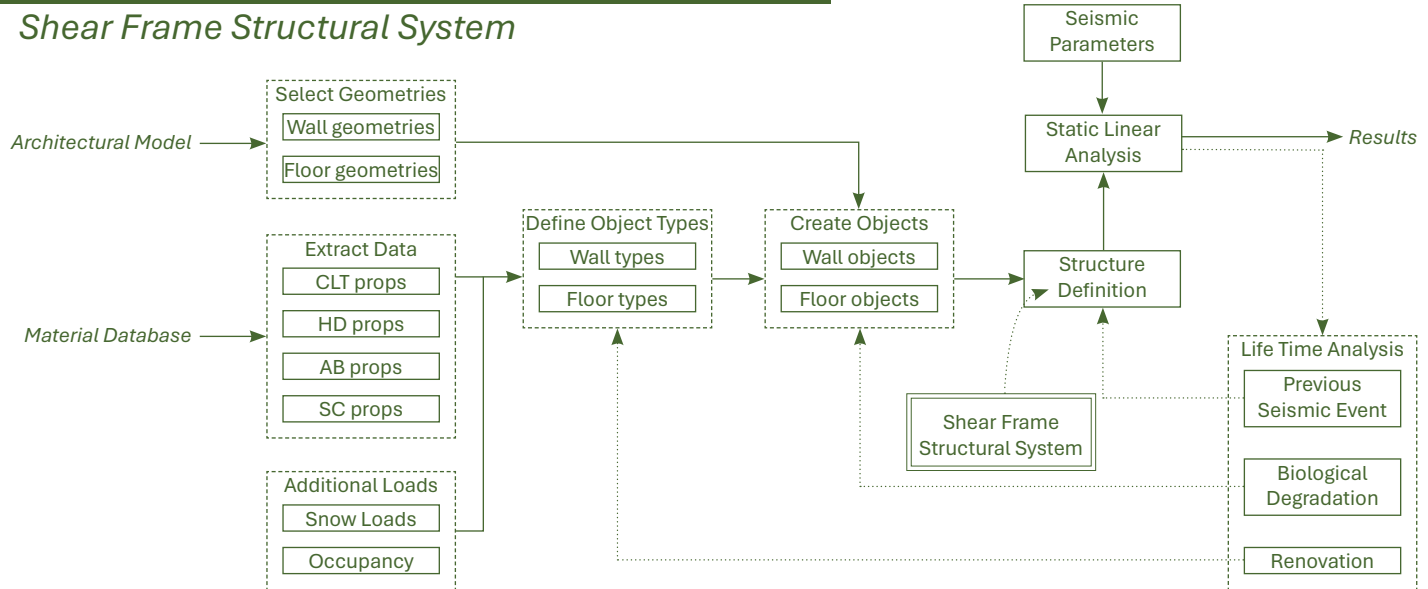
Future Development

Include Concrete Parameters



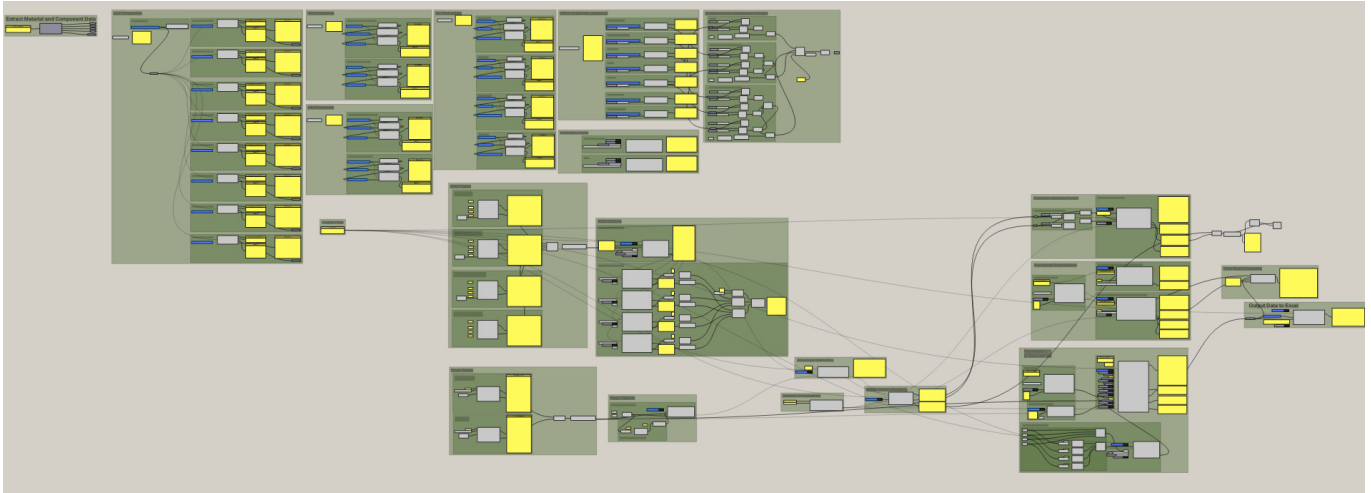
Future Development

Shear Frame Structural System



Future Development

User Interface



Thank you!

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