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## Issues in the Prediction of the Mechanical Properties of Open Graded Mixes

Hong Zhang<sup>1</sup>, Kumar Anupam<sup>1</sup>, Athanasios Scarpas<sup>1,2</sup>,  
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### Abstract

Within the pavement engineering community, open graded mixes (OGM) are regarded as mixes capable of reducing noise and improving wet skid resistance. However, during their design life, these asphalt mixes are known to suffer from a particular distress type known as raveling. This results in a premature failure of a road network. In order to study the propensity of OGM to raveling, homogenization-based approaches are considered to be accessible and effective. One of the most widely accepted homogenization models for asphalt concrete is proposed by Christensen et al. Several studies related to homogenization techniques have been conducted in the past; however, to the best of the authors' knowledge not a lot of attention has been paid to the study of OGM by means of homogenization models. The other limitation of the Christensen model is that some parameters are difficult to physically understand. Under the above realization, the objective of the paper is twofold: (1) to propose a modification of the Christensen model for OGM; and (2) to verify the modified model's capability in predicting the mechanical properties of OGM. In general, it was found that once the proposed factor is calibrated for a given OGM by laboratory tests the obtained results are accurate.

Open graded mixes (OGM) are increasingly used because of their effectiveness in reducing noise caused by tire–pavement interaction and improving skid resistance, especially during wet seasons (1). However, because of the high air voids characteristic, OGM always suffer from a particular distress known as raveling, which occurs as individual aggregate particles dislodge from the pavement surface (2). Normally, there are two types of damage modes for raveling distress: cohesive damage and adhesive damage (Figure 1) (2). Cohesive damage is the failure of mastic properties, whereas adhesive damage is the loss of bond between aggregate particles and mastic.

The methods proposed by researchers (3) at the Delft University of Technology to evaluate the characteristics of cohesive damage and adhesive damage in the laboratory are shown in Figure 1). A mastic column, the size of which is 6 mm in diameter and 12 mm in height, and a stone column, which consists of two stone columns and a mastic sample (8 mm in diameter and 2 mm in thickness) between them, are used to perform dynamic shear stress or strain to obtain fatigue life curves for analyzing cohesive damage and adhesive damage, respectively.

Although the characteristics of cohesive damage and adhesive damage can be evaluated by laboratory tests, the occurrences of these two damage modes in the field

are affected by various factors, such as repetition of loads, moisture, temperature, and so forth (4). Therefore, it is a big challenge to analyze the propensity of these two phenomena during the service life of OGM pavement.

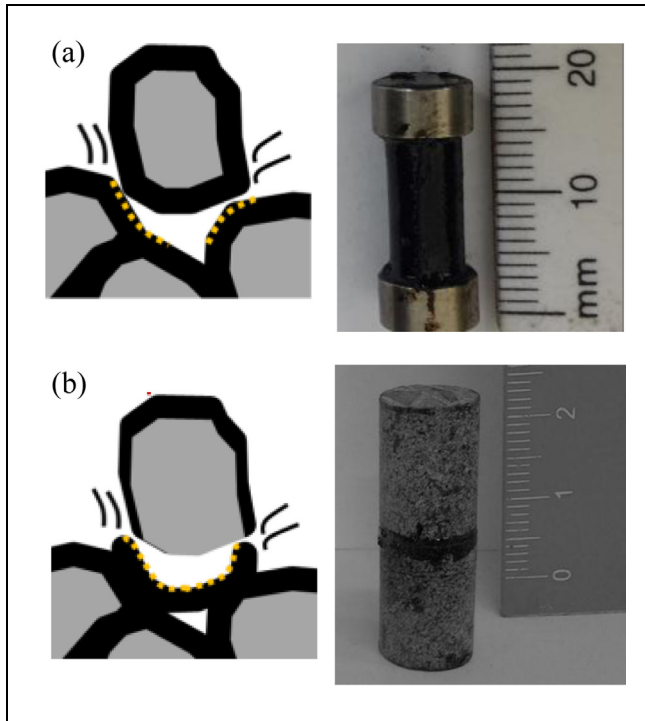
In the recent past, computational models based on finite element methods (FEM) and/or discrete element methods (DEM) have been proposed as a mean to obtain the stresses and/or strains at mix component level of OGM, which are further used to analyze the raveling damage (5, 6). Although FEM/DEM-based models are able to handle complex compositions and almost realistic mix component material properties, they require the development of very large FEM meshes (usually by means of post-processing of the results of CT scans) and very large-scale computational facilities. For example, for a specimen of 60 mm in height and 150 mm in diameter, about 6.8 million elements are needed to carry out a reliable analysis and a running time of several days (Figure 2a) (6). Computational tools and facilities of such scale are not typically available in engineering practice.

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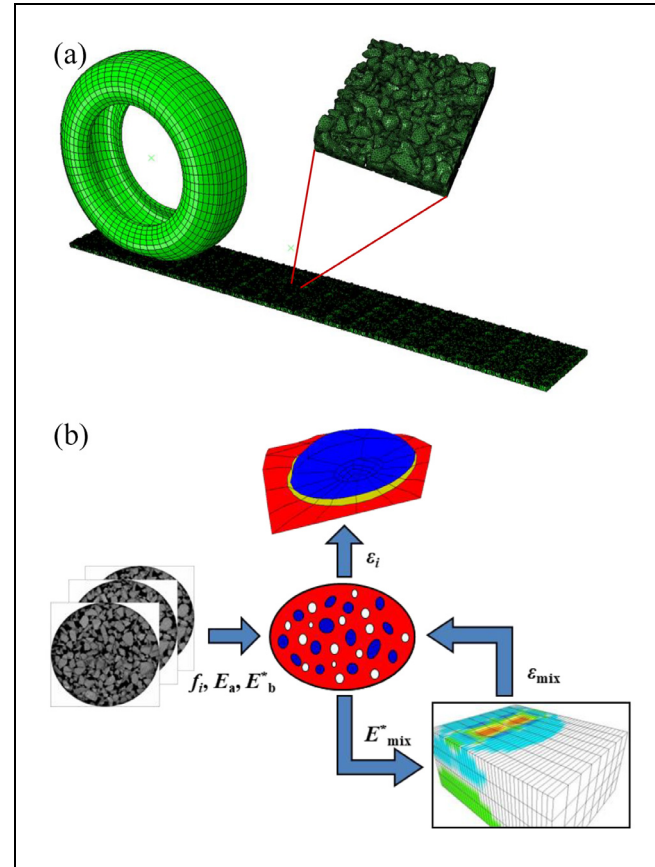
Address correspondence to Hong Zhang: h.zhang-4@tudelft.nl



**Figure 1.** Two types of damage modes of raveling distress. (a) Cohesive damage; (b) Adhesive damage.

Homogenization methods (7) offer an attractive alternative to the above issues (Figure 2b). By means of a homogenization technique, the effective micromechanical properties of the composite are determined from the properties of its different phases, which further allows the calculation of the stress/strain field of the different phases based on the applied loading conditions. This information, in combination with the laboratory tests mentioned above for evaluating the characteristics of cohesive damage and adhesive damage, can be utilized to investigate the propensity of a given OGM for raveling.

On the basis of the homogenization theory, many models have been developed for estimating the mechanical properties of asphalt mixes (8). Among these models, the Hirsch model is one of the most commonly used semi-empirical models because of its similarity and requirement of few constituent properties (8). It was originally developed by Hirsch (9) to investigate the modulus of concrete as determined by the moduli of cement and aggregate particles. In the Hirsch model, different phases are assumed to be in a combination of parallel and series arrangements. The modulus of the composite is controlled by the volume fractions and the moduli of all its phases. Further, Christensen et al. (10) modified the original Hirsch model to make it applicable for asphalt concrete, and the Christensen model has been independently evaluated by several researchers (11, 12) with generally good results.



**Figure 2.** Different approaches for OGM analysis. (a) FEM approach; (b) Homogenization approach.

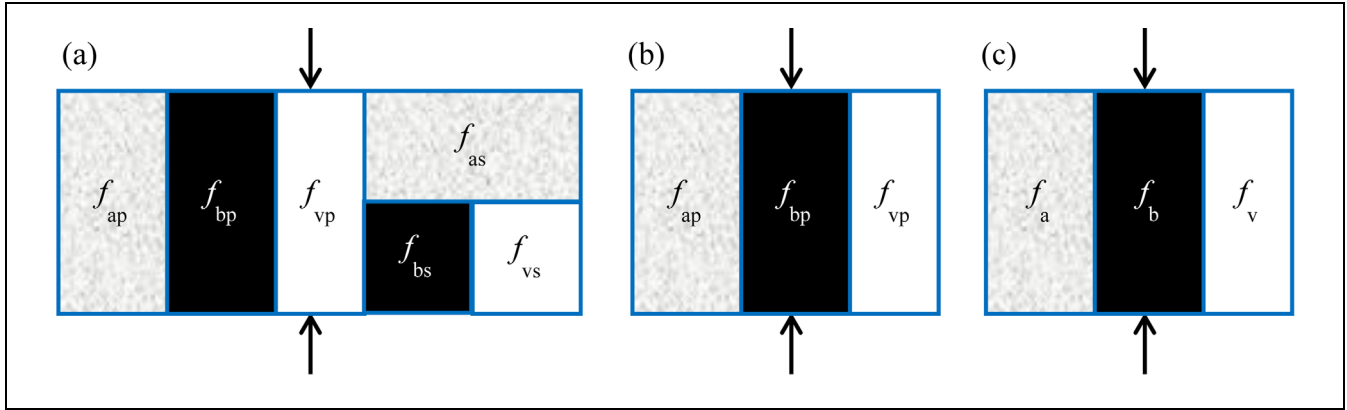
From the above analysis, it can be concluded that in comparison with the FEM and DEM methods, the homogenization approach is a more accessible and effective way for obtaining the mechanical properties of OGM. It can also be summarized that the Hirsch model, which relates the properties of each phase to the overall properties of mixes, is a simple but effective homogenization model. Based on this realization, the objective and research scopes will be proposed in the following section.

### Objective and Research Scopes

The objective of this paper is to outline the homogenization methodology for estimating the mechanical properties of OGM via the Hirsch model.

The scopes of the proposed study include:

- To modify the expression of the Christensen model and propose an aggregates organization factor to describe the frequency/temperature-dependent contribution of the aggregate phase to the overall behavior of OGM.



**Figure 3.** Arrangement of the Christensen model for asphalt mixes. (a) Original arrangement (10); (b) Simplified arrangement (13); (c) Proposed revised arrangement.

- To determine and validate the expression for the aggregates organization factor from laboratory tests of OGM.
- To analyze the contribution made by each phase to the modulus of OGM based on the modified expression of the Christensen model.

## Background Knowledge

### Application of the Hirsch Model on Asphalt Mixes

On the basis of the research needs described earlier, descriptions about some of the aspects related to the Hirsch model will be presented in this section. In the early research of Christensen et al. (10), various phase arrangements were proposed to modify the original Hirsch model to make it applicable for asphalt mixes. After thorough investigation of several proposed versions of the modified Hirsch model, the researchers came to a conclusion that the version in which the parallel and series sub-units composed of asphalt binder, aggregates, and air voids are arranged in parallel provided the most accurate predicted results of the modulus of asphalt concrete (Figure 3a).

In this version, the dynamic Young's modulus of an asphalt mix  $|E^*|_{\text{mix}}$  is obtained from the volume fractions and moduli of asphalt binder, aggregates and air voids, Equation 1.

$$|E^*|_{\text{mix}}(f) = f_{\text{ap}}E_a + 3f_{\text{bp}}|G^*|_{\text{b}}(f) + (f_{\text{as}} + f_{\text{bs}} + f_{\text{vs}})^2 \left[ \frac{f_{\text{as}}}{E_a} + \frac{(f_{\text{bs}} + f_{\text{vs}})^2}{3f_{\text{bs}}|G^*|_{\text{b}}(f)} \right]^{-1} \quad (1)$$

where the subscripts p and s represent the parallel portion and the series portion, respectively;  $f_a$  and  $E_a$  are the

volume fraction and Young's modulus of the aggregate phase, respectively;  $f_b$  and  $|G^*|_{\text{b}}$  are the volume fraction and the dynamic shear modulus of the asphalt binder phase, respectively; and  $f_v$  is the volume fraction of the air voids phase.

However, it was observed that the effects of the series sub-unit on the estimated modulus of an asphalt mix are much less significant than the effects of the parallel sub-unit (13), which indicates that the characteristics of an asphalt mix are similar to a parallel arrangement of individual phases. Under this realization, the authors (13) further simplified the original Christensen model to a simple parallel arrangement, as shown in Figure 3b, Equation 2.

$$|E^*|_{\text{mix}}(f) = f_{\text{ap}}E_a + 3f_{\text{bp}}|G^*|_{\text{b}}(f) \quad (2)$$

where  $f_{\text{ap}}$ ,  $f_{\text{bp}}$ ,  $E_a$  and  $|G^*|_{\text{b}}$  are described in Equation 1.

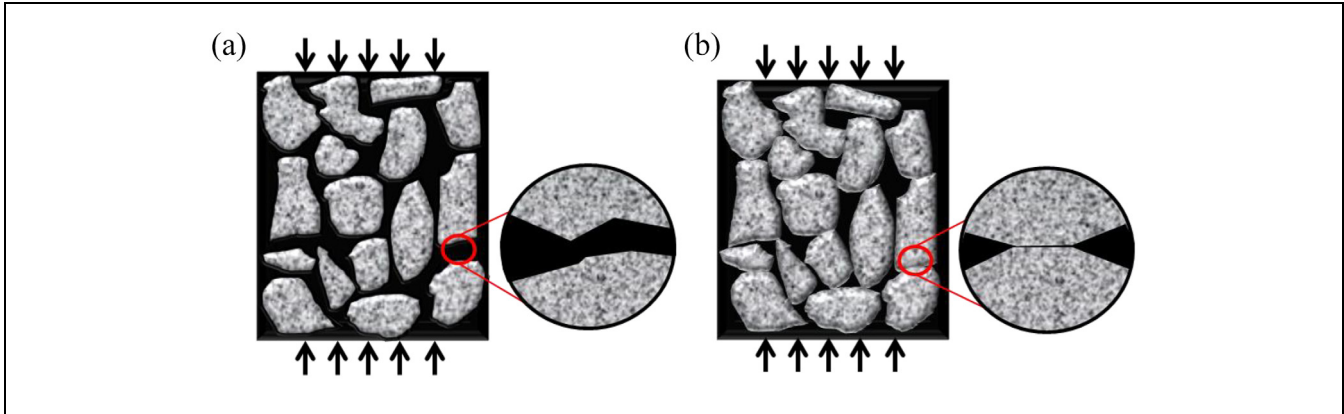
Equation 2 has two unknown parameters  $f_{\text{ap}}$ ,  $f_{\text{bp}}$ , which makes it difficult to be determined by using laboratory/field tests. Researchers (10) have proposed further simplification to solve this issue by introducing factors such as contact factor  $P_c$ .

### Contact Factor $P_c$

$P_c$  is the share of the parallel component of the total volume of the composite and therefore varies between 0 and 1. As can be seen from Equation 2, if the parallel part occupies the same proportion in each phase, Equation 2 can be further reduced to the following equation, Equation 3.

$$|E^*|_{\text{mix}}(f) = P_c(f)(f_a E_a + 3f_b |G^*|_{\text{b}}(f)) \quad (3)$$

where  $f_a$ ,  $f_b$ ,  $E_a$  and  $|G^*|_{\text{b}}$  are described in Equation 1. However, it is highlighted here that the underlying



**Figure 4.** Aggregate contacts at different frequencies/temperatures. (a) High frequency/low temperature; (b) Low frequency/high temperature.

assumption is that the value of  $P_c$  does not change with different phases but remains the same for all the phases of the composite.

It is obvious that  $P_c$  is one of the critical parameters in the calculation of  $|E^*|_{\text{mix}}$ . As the behavior of asphalt concrete depends on frequency/temperature, the relative proportions of the series part and parallel part are also frequency/temperature dependent. As proposed by the researchers (10), the values of  $P_c$  can be determined from the laboratory tests. The researchers concluded that the proposed factor  $P_c$  is frequency/temperature sensitive, as expected.

In line with the above research work, researchers (10, 13) showed that in general a good agreement exists between the predicted results of the Christensen model and the laboratory tests. However, to the best of the authors' knowledge, it can be stated that the physical representation of  $P_c$  is difficult. In their pioneering studies, researchers (10) hypothesized that  $P_c$  represents the aggregate contact factor and interpreted it as the contribution from the portion of aggregate particles in intimate contact with each other. They also noted that high values of  $P_c$  at high frequencies/low temperatures indicate more contact among aggregate particles. However, this interpretation does not comply with the physical situation where fewer aggregate particles are expected to be in intimate contact at high frequencies/low temperatures (Figure 4a). This behavior is expected because of the existence of stiff asphalt binder at high frequencies/low temperatures. Similarly, at low frequencies/high temperatures when the modulus of asphalt binder is soft, it is expected that the aggregate would find it easier to move, which will result in more pronounced contact (Figure 4b).

Based on the above analysis, it can be concluded that the interpretation of  $P_c$  does not consider the aggregate contact interaction aptly. Thus, in the following section, a modified expression for the Christensen model will be proposed.

### Modified Expression of the Christensen Model

As mentioned above, the effects of the series element in the Christensen model on the estimated modulus of asphalt concrete are negligible as compared with the effects of the parallel element. Therefore, in the revised arrangement, it is proposed that the total volume of asphalt binder, aggregates, and air voids are arranged in parallel (Figure 3c). This arrangement is the same as the arrangement proposed by the original researchers (13) except that in the revised arrangement the part of the volume is replaced by the total volume. The relationship between  $|E^*|_{\text{mix}}$  and the properties of individual phases is shown in Equation 4.

$$|E^*|_{\text{mix}}(f) = f_a E_a + 3f_b |G^*|_b(f) \quad (4)$$

where  $f_a, f_b, E_a$  and  $|G^*|_b$  are described in Equation 1.

The authors proposed to modify Equation 4 by introducing a factor  $P_a$ , which describes the contribution to  $|E^*|_{\text{mix}}$  from the arrangement of aggregate particles at different frequency/temperature conditions, Equation 5. A detailed description about  $P_a$  will be presented in the following subsection.

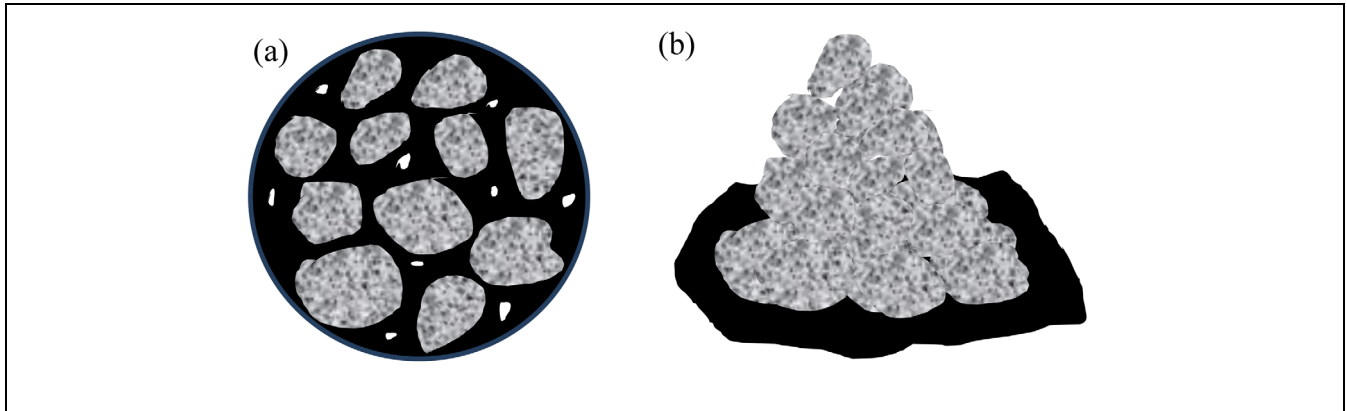
$$|E^*|_{\text{mix}}(f) = P_a(f) f_a E_a + 3f_b |G^*|_b(f) \quad (5)$$

where  $f_a, f_b, E_a$  and  $|G^*|_b$  are described in Equation 1.

### Aggregates Organization Factor, $P_a$

An asphalt mix has a higher value of  $f_a$  than  $f_b$ . Moreover, the value of  $E_a$  is also much higher than  $|G^*|_b$ . Therefore, the contributions made by the aggregate phase to the modulus of the asphalt mix are expected to be higher than other phases. If a non-frequency/temperature-dependent factor was introduced for evaluating the contribution from the aggregate phase to





**Figure 5.** Organization of aggregates at different frequencies/temperatures. (a) High frequency/low temperature; (b) Low frequency/high temperature

the overall response of the mix,  $|E^*|_{\text{mix}}$  would also be primarily frequency/temperature independent. This can be clearly deduced from Equation 5, where the aggregate phase contribution ( $P_a f_a E_a$ ) becomes temperature/frequency independent as a result of the temperature/frequency-independent  $P_a$ .

In order to physically understand the concept of  $P_a$ , if hypothetically two extreme conditions of low temperatures (or high frequencies) and high temperatures (or low frequencies) are considered, then in the case of low temperatures the asphalt binder would be stiff and it would be able to bond the particles together well (Figure 5a), whereas, at high temperatures, the binder would be too soft to bind the particles (Figure 5b). In the former case, the whole structure will act together in the load-bearing capacity; on the contrary, in the latter case, the asphalt binder will not take part in the load-bearing capacity and the mix would slowly collapse.

From the above discussions, it can be stated that the introduction of the factor,  $P_a$ , is logical and meaningful. It is also expected to capture the frequency/temperature-dependent contribution of the aggregate phase in predicting  $|E^*|_{\text{mix}}$ .  $P_a$ , which is termed the “aggregates organization factor,” has been determined and validated by the laboratory tests in this study. The description of the sample preparation, laboratory tests, test results and analysis will be presented in the later sections.

## Material Properties

Two OGM specimens and one dense asphalt concrete (DAC) specimen, denoted as “OGM-1,” “OGM-2,” and “DAC,” were prepared. Specimen OGM-1 was utilized for carrying out necessary tests to calibrate the values of  $P_a$ , whereas specimens OGM-2 and DAC were used for the subsequent validation of the proposed model. The properties of different used materials are shown in Table 1.

These specimens with a height of 150 mm and a diameter of 100 mm were prepared by using a gyratory compactor in accordance with the AASHTO T 342-11 standard method (14). The laboratory test methods to measure the mechanical properties of these specimens will be introduced in the next section.

## Laboratory Tests

### Dynamic Shear Rheology Test

The values of  $|G^*|_b$  was measured by using the parallel plate configuration of dynamic shear rheology equipment. Frequency sweep tests were conducted in a frequency range of 50–0.1 Hz, at four different temperatures,  $-10^\circ\text{C}$ ,  $4^\circ\text{C}$ ,  $21^\circ\text{C}$  and  $37^\circ\text{C}$ . Two geometrically different asphalt binder specimens 8 mm and 25 mm in diameter, and 2 mm and 1 mm in height, were used.

### Uniaxial Compression Test

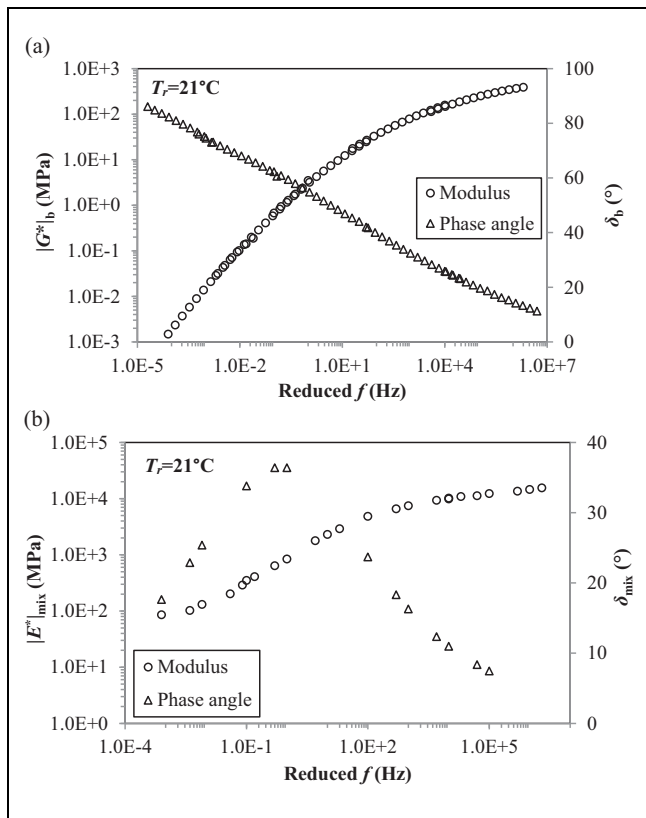
The values of  $|E^*|_{\text{mix}}$  were measured under cyclic sinusoidal uniaxial compression load by universal testing machine (UTM) at different frequencies and temperatures. In accordance with the AASHTO T 342-11 standard method (14), four temperatures,  $-10^\circ\text{C}$ ,  $4^\circ\text{C}$ ,  $21^\circ\text{C}$  and  $37^\circ\text{C}$ , were applied during the test and for each temperature, six different frequencies, which were 20 Hz, 10 Hz, 5 Hz, 1 Hz, 0.5 Hz, and 0.1 Hz, were performed. The load applied on the specimens was under stress-controlled mode and three linear variable differential transformer sensors were installed to measure their displacements.

## Results and Discussion

According to the time–temperature superposition principle, the measured results of  $|G^*|_b$  and the phase angle  $\delta_b$  of asphalt binder at different temperatures were shifted

**Table 1.** Properties of Materials

Type of materials	Gradation of OGM (% passing)	Gradation of DAC (% passing)	Density (kg/m <sup>3</sup> )	Volume fraction OGM-1	Volume fraction OGM-2	Volume fraction DAC
16	98.3	97	2686	0.735	0.777	0.831
11.2	77.3	85	2686			
8	43.8	74.5	2678			
5.6	22.1	57.5	2670			
2	15	40	2673			
0.063	4.1	7	2658			
Filler	0	0	2638			
Asphalt 70/100	4.5	6.4	1032	0.086	0.091	0.137
Air voids	-	-	-	0.179	0.132	0.032



**Figure 6.** Laboratory test results of asphalt binder and OGM-1. (a) Master curve of  $|G^*|_b$  and  $\delta_b$ ; (b) Master curve of  $|E^*|_{mix}$  and  $\delta_{mix}$ .

to obtain master curves over a large range of frequencies at a reference temperature ( $T_r$ ) of 21°C (Figure 6a). The plots tend to follow the power law curve in the test frequency range, which is consistent with the results from other research (15).

From the UTM tests, the master curves of  $|E^*|_{mix}$  and phase angle  $\delta_{mix}$  of OGM-1 specimen were obtained at the same reference temperature (Figure 6b). It can be

observed that the value of  $|E^*|_{mix}$  decreases from high frequencies to low frequencies. The plot of  $|E^*|_{mix}$  reaches a stable plateau at high/low frequencies beyond which there is only a slight change in the value of  $|E^*|_{mix}$ . This trend further solidifies the hypothesis made regarding  $P_a$  in the earlier section that weaker bond characteristics among aggregate particles are expected with decreasing frequencies.

The value of  $\delta_{mix}$  reaches a peak and starts to further decline as a result of the decrease in frequency. This indicates that the mix behaves elastically both at higher and lower frequencies. This behavior can also be explained in terms of  $P_a$  where at low temperatures the binder itself starts to behave elastically thus the overall mix response is elastic; on the other hand, at high temperatures  $P_a$  is too low to contribute in the load-bearing capacity of the mix, and that is why the bulk of loading is taken over by the aggregates themselves, which are elastic in nature. A laboratory-based approach to determine  $P_a$  will be presented in the following paragraphs.

#### Determination of $P_a$ from Laboratory Tests

Equation 5 can be rearranged in the form of the following equation, Equation 6:

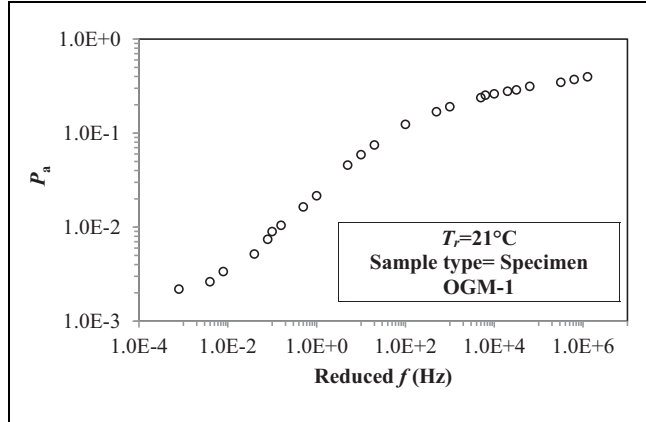
$$P_a(f) = \frac{|E^*|_{mix}(f) - 3f_b|G^*|_b(f)}{f_a E_a} \quad (6)$$

Where  $f_a$ ,  $f_b$ ,  $E_a$  and  $|G^*|_b$  are described in Equation 1; the values of  $f_a$  and  $f_b$  can be found in Table 1 and the value of  $E_a$  is assumed as 53000 MPa.

The values of  $P_a$  can be calculated with the mix properties and Equation 6. A typical curve as obtained for specimen OGM-1 is shown in Figure 7. The shape of the plot shows that  $P_a$  is indeed a frequency-dependent factor as discussed before. It can also be observed that  $P_a$  increases with the increase in frequency and more or less follows a sigmoidal curve for the tested specimen. This in

**Table 2.** Fitting Results of Constant Parameters

Parameters	$a$	$b$	$c$	$d$
Values	0.0017	0.62	0.72	-0.17

**Figure 7.** Calculation results of  $P_a$ .

terms of physical meaning implies that the contribution made by the aggregate phase to the modulus of OGM increases with the frequencies.

By assuming  $P_a$  to follow a sigmoidal curve, Equation 7 can be deduced. It is noted here that Equation 7 is similar in nature to the  $P_c$  function as described elsewhere (10). After fitting the test results in Equation 7, constant parameters of the equation can be obtained, as shown in Table 2.

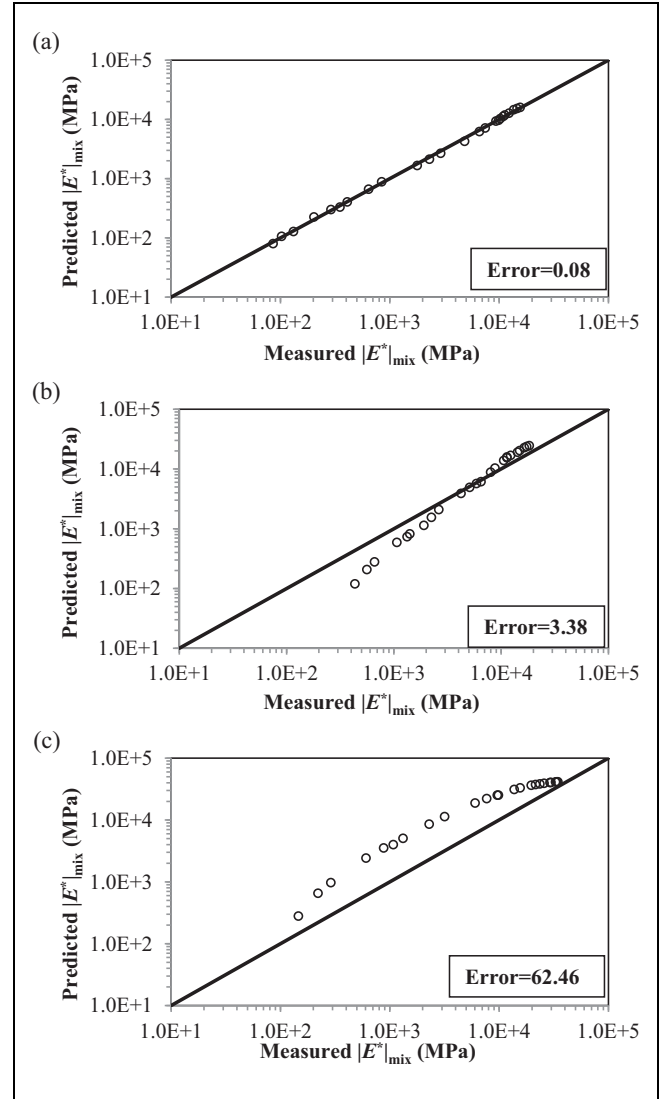
$$P_a = a + (1 - a) \frac{\exp(b + c \ln(f_b/(f_b + f_v)) \times |G^*|_b) + d(f_b + f_v)}{1 + \exp(b + c \ln(f_b/(f_b + f_v)) \times |G^*|_b) + d(f_b + f_v)} \quad (7)$$

where  $f_a, f_b, f_v$  and  $|G^*|_b$  are described in Equation 1.

It is important that the proposed model is validated. In order to judge the suitability of  $P_a$ , a two-step validation approach was adopted. In the first step the overall response of the specimen OGM-1 was compared against the predicted results, and in the second step, predictions were made for the specimen OGM-2 and the specimen DAC on the basis of parameters obtained in Table 2.

### Validation of the Modified Expression of the Christensen Model

Figure 8,  $a-c$ , shows the predicted results of  $|E^*|_{\text{mix}}$  for specimen OGM-1, specimen OGM-2 and specimen

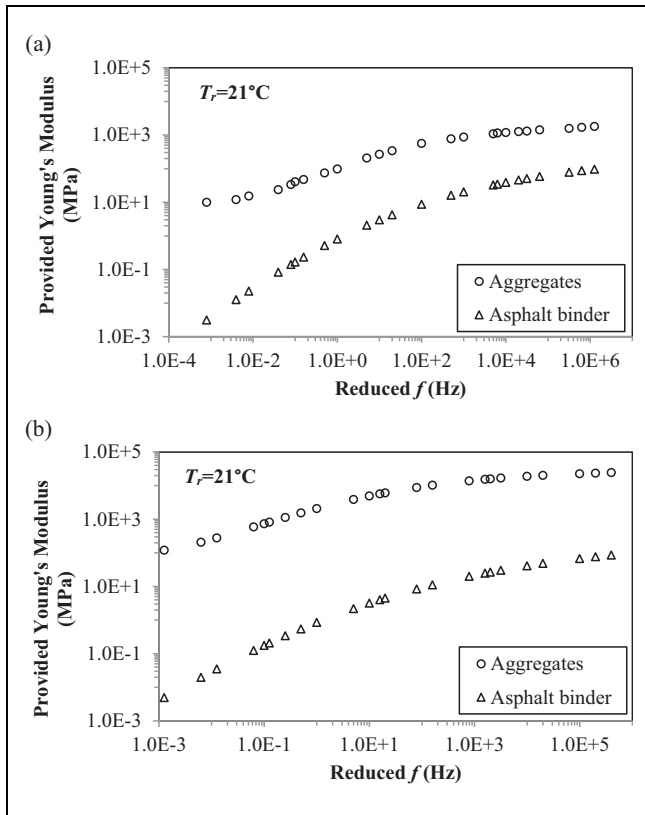
**Figure 8.** Comparison between predicted results of  $|E^*|_{\text{mix}}$  to the laboratory tests. (a) Specimen OGM-1; (b) Specimen OGM-2; (c) Specimen DAC.

DAC, respectively. It can be observed that the predicted results of  $|E^*|_{\text{mix}}$  for specimen OGM-1 are in a good agreement with the laboratory tests, which provides a good check for the calibration procedure.

The predicted results of  $|E^*|_{\text{mix}}$  for specimen OGM-2 on the basis of parameters obtained from specimen OGM-1 fit with reasonable accuracy. These differences were already expected because the parameters used for both OGM specimens in Equation 7, although with different properties (see Table 1), are the same. Despite these differences, the predicted and the test results for practical purposes match quite well for OGM.

A deliberate comparison between two altogether different types of mixes was further made to judge the suitability of the proposed model. As one would expect, it is





**Figure 9.** Modulus provided by aggregates and asphalt binder in OGM. (a) Specimen OGM-1; (b) Specimen OGM-2.

clear that the differences between these two results are substantial (Figure 8c).

Overall, it can be concluded that if the parameters are obtained for one type of mix, the prediction could be made with fair accuracy, and if the type of mix is completely changed then new parameters would need to be calibrated.

### Contributions of Each Phase in the $|E^*|_{mix}$ Prediction

According to Equation 5, the contribution of the aggregate phase to the total mix modulus is obtained by  $P_a f_a E_a$ , whereas the contribution of the asphalt binder phase could be obtained by  $3f_b |G^*|_b$ . Figure 9, a and b, shows the plots of the relative contribution of each phase in the overall response of the OGM specimens. It can be seen that both phases are frequency dependent as explained earlier.

As expected it is found that the aggregate phase makes a more significant contribution to the modulus of OGM. It can be further deduced that in OGM the aggregate phase provides the load-bearing capacity while the role of the asphalt binder phase is to bind the aggregate particles together ensuring the structural organization of the composite material.

## Conclusions

Homogenization methodology makes the analysis of OGM accessible for determining the propensity of raveling. In order to describe the frequency/temperature-dependent contribution of each phase to the overall response of OGM, the expression of the Christensen model was modified and a temperature/frequency-dependent factor  $P_a$  was proposed. A function for calculating  $P_a$  was determined and verified against test results. Furthermore, the contributions made by each phase to the modulus of OGM were analyzed. The following conclusions were drawn:

- By means of  $P_a$ , the effects of the frequency/temperature-dependent contribution of the aggregate phase on the overall mix response can be accounted for, and the modified expression of the Christensen model can produce the shape and values of the frequency/temperature-dependent modulus of asphalt mixes.
- The calibrated function for describing  $P_a$  can only obtain accurately predicted results of the material which it is calibrated for.
- In the whole range of frequencies/temperatures, the contribution made by the aggregate phase to the overall modulus of OGM is much more significant than the contribution of the asphalt binder phase.

## Current Research

The basic limitation of the presented work is the need to experimentally determine  $P_a$ . In future research, the current work will be extended to enable the prediction of asphalt mix properties exclusively on the basis of the mechanical properties of its constituents.

## Author Contributions

The authors confirm contribution to the paper as follows: study conception and design: Athanasios Scarpas, Kumar Anupam, Hong Zhang; data collection: Hong Zhang; analysis and interpretation of results: Hong Zhang, Kumar Anupam, Athanasios Scarpas, Cor Kasbergen; draft manuscript preparation: Kumar Anupam, Hong Zhang. All authors reviewed and the results and approved the final version of the manuscript.

## References

1. Hagos, E. T. *The Effect of Aging on Binder Properties of Porous Asphalt Concrete*. Delft University of Technology, Delft, 2008.
2. Mo, L., M. Huurman, S. Wu, and A. A. A. Molenaar. Ravelling Investigation of Porous Asphalt Concrete Based on Fatigue Characteristics of Bitumen–Stone Adhesion

- and Mortar. *Materials & Design*, Vol. 30, No. 1, 2009, pp. 170–179.
3. Khedoe, R. N., and J. Moraal. *Lifetime Optimisation Tool: Sample Preparation and Laboratory Testing for the LOT*. Delft University of Technology, Delft, 2007.
  4. Kringos, N., and A. Scarpas. Raveling of Asphaltic Mixes Due to Water Damage: Computational Identification of Controlling Parameters. *Transportation Research Record: Journal of the Transportation Research Board*, 2005. 1929: 79–87.
  5. Kringos, N., and A. Scarpas. Development of a Finite Element Tool for Simulation of Ravelling of Asphaltic Mixes. *Proc., International Conference on Computational and Experimental Engineering and Sciences (ICCES)*, Madeira, 2004, pp. 1211–1216.
  6. Anupam, K., S. K. Srirangam, A. Varveri, C. Kasbergen, and A. Scarpas. Microstructural Analysis of Porous Asphalt Concrete Mix Subjected to Rolling Truck Tire Loads. *Transportation Research Record: Journal of the Transportation Research Board*, 2016. 2575: 113–122.
  7. Ghossein, E. *Numerical Validation of Analytical Homogenization Models for the Case of Randomly Distributed and Oriented Ellipsoidal Fibers Reinforced Composites*. Université de Montréal, Montreal, 2014.
  8. Eberhardsteiner, L. *Extension of a Multiscale Model for Hot Mix Asphalt Stiffness Prediction to Introduce Binder Aging into Pavement Design*. Vienna University of Technology, Wien, 2014.
  9. Hirsch, T. J. Modulus of Elasticity of Concrete Affected by Elastic Moduli of Cement Paste Matrix and Aggregate. *Proceeding of the American Concrete Institute*, Vol. 59, No. 3, 1962, pp. 427–452.
  10. Christensen, D. W., J. T. Pellinen, and R. F. Bonaquist. Hirsch Model for Estimating the Modulus of Asphalt Concrete. *Journal of the Association of Asphalt Paving Technologists*, 2 Vol. 72, 2003, pp. 97–121.
  11. Dongre, R., L. Myers, J. D'Angelo, C. Paugh, and J. Gudimettla. Field Evaluation of Witczak and Hirsch Models for Predicting Dynamic Modulus of Hot-Mix Asphalt. *Journal of the Association of Asphalt Paving Technologists*, Vol. 74, 2005, pp. 381–442.
  12. Pellinen, T., A. Zofka, M. Marasteanu, and N. Funk. Asphalt Mix Stiffness Predictive Models. *Journal of the Association of Asphalt Paving Technologists*, Vol. 76, 2007, pp. 575–625.
  13. Christensen, D. W., and R. Bonaquist. Improved Hirsch Model for Estimating the Modulus of Hot-Mix Asphalt. *Road Materials and Pavement Design*, Vol. 16, 2015, pp. 254–274.
  14. American Association of State Highway Transportation Officials. *Determining Dynamic Modulus of Hot-Mix Asphalt Concrete Mixes*. Washington, D.C.: AASHTO T342-11, 2011.
  15. Underwood, B. S. *Multiscale Constitutive Modelling of Asphalt Concrete*. ProQuest Dissertations and Theses, Raleigh, N.C., 2011.
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