

Technical report

Maeslant barrier

Alternative solution for the upgrading of the Maeslant barrier



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Cover

Closed Maeslant Barrier, Bird's eye view

Source: Beeldbank Rijkswaterstaat

https://beeldbank.rws.nl/MediaObject/Details/Luchtfoto_van_de_gesloten_Maeslantkering_in_de_nieuwe_Waterweg_nabij_Hoek_van_Holland_158813

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Preface

I am an exchange student from the Swiss Federal Institute of Technology in Zürich (ETH). Part of the master program at the ETH is writing an additional thesis in each study direction you are following. This is therefore the report to my project for hydraulic engineering. The goal of this thesis is to demonstrate the ability to use all the knowledge gathered in the last years and to perform research at an academic level.

The aim of this research is to investigate alternative solutions for the upgrading of the Maeslant barrier. Based on the experience about the Ramspol barrier an alternative design with an inflatable rubber barrier is worked out.

I like to use the opportunity to thank my supervisor ir. Wilfred Molenaar for his support.

Jonas Riteco,

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Abstract

The Maeslant barrier was the final piece of the Delta works which protects a large area of the Netherlands. However the present barrier seems to fulfil his function, several Dutch experts have doubts about the system. However the discussion on the Maeslant barrier not being safe enough refers mostly to future scenarios. Experts expect that the barrier already will not provide enough protection from storm surges as of 2070.

The goal of this research is to investigate an alternative solution for the upgrading of the existing Maeslant barrier in order to make it more reliable for the future. The two functional requirements that all future solutions must fulfil are assuring enough safety against storm surges and providing enough navigation space for ships. The new waterway has to be accessible at any time during normal conditions. For the hydraulic boundary conditions future water levels are taken into account by implying the most extreme Deltascenario steem.

In a variant study four different designs are worked out. Design A introduces a solution with navigation locks. This solution closes of the new waterway entirely which can be an advantage for safety reasons but also a disadvantage in terms of navigation. The second design consist of an inflatable rubber barrier like the Ramspol barrier. The mode of operation and in terms of maintenance the inflatable barrier has a lot of advantages, two of the biggest disadvantages are however the tension forces and stresses in the membrane and the fact that the dimensions exceed the largest inflatable rubber barrier by the triple. Design C consist of a new barrier with tumble gates either hydraulic or pneumatic. This design was already introduced as one of the alternatives for the existing Maeslant barrier. However these kind of gates are straightforward and simple the disadvantage is that large movable parts are placed under water which makes maintenance work laboured and expensive. The last design consists of several rising sector gates as it has already been built at the Thames in the UK. The dimensions would be in the same range and therefore a lot of the experience could be used for the design. This design however leads to fixed structures within the water way which are a hindrance for navigation. The inflatable barrier is worked out in more detail.

After taking a closer look at the biggest inflatable barrier in the world, the Ramspol barrier in the Netherlands, working out general information on inflatable barriers provides the basics for the design of the new Maeslant barrier. Because of the positive experience at the Ramspol barrier a combination of water and air was chosen to be the filling material. A purely air filled barrier leads to high tension forces in the membrane that exceed the bearing capacity. By using factors between the crest-height and the base width of the clamping lines and the crest-height and the circumferential length of the membrane based on the Ramspol barrier, the initial parameters for the new Maeslant barrier could be calculated using a crest-height of 22 meters. From that an iterative calculation process using a excel spread sheet calculates the shape, the internal air pressure and tension force in the membrane. First results than lead to an inflatable rubber barrier with a crest-height of 22.55 m, an internal air pressure of 0.6 bar and a tension force of 730 kN/m^2 . Optimizing the structure leady to a final design with a smaller base width, slightly longer circumferential length, lower tension force and internal air pressure and a higher crest-height.

Using the calculated tension force the horizontal and vertical clamping forces are calculated and can be used to design the clamping lines and the foundation. The foundation also includes the bottom recess where the membrane is stored in the deflated situation. The membrane fits exactly over the ribs created by conveyors to facilitate the horizontal transport. The membrane will then be held in place by applying an under-pressure in the membrane.

Contents

1 Introduction	1
1.1 Problem description	1
1.2 Objectives	2
1.3 Project outline	2
2 Analysis of existing Maeslant barrier	3
2.1 Classification	3
2.2 Alternative gate design	3
2.3 Performance	5
2.4 What is wrong with the barrier?	5
3 Variant study	6
3.1 Introduction	6
3.2 Boundary conditions	6
3.2.1 General boundary conditions	6
3.2.2 Functional requirements	6
3.2.3 Hydraulic boundary conditions	7
3.2.4 Navigation boundary conditions	7
3.3 Design A - navigation locks	8
3.3.1 Introduction	8
3.3.2 Mode of operation	8
3.3.3 Specific boundary conditions	9
3.3.4 Advantages and disadvantages	9
3.4 Design B - inflatable rubber barrier	10
3.4.1 Introduction	10
3.4.2 Mode of operation	10
3.4.3 Specific boundary conditions	11
3.4.4 Advantages and disadvantages	11
3.5 Design C - tumble gate	12
3.5.1 Introduction	12
3.5.2 Mode of operation	12
3.5.3 Specific boundary conditions	13
3.5.4 Advantages and disadvantages	13
3.6 Design D - rising sector gate	14
3.6.1 Introduction	14
3.6.2 Mode of operation	14
3.6.3 Specific boundary conditions	15
3.6.4 Advantages and disadvantages	15
3.7 Evaluation	15
4 Inflatable rubber barrier	17
4.1 Case study Ramspol	17
4.1.1 Substructures	17
4.1.2 Hydrodynamic and load conditions	20
4.1.3 Experience with the Ramspol barrier	21

4.2 General theory for inflatable barriers	21
4.2.1 Crest height	21
4.2.2 Filler and filling mechanism	22
4.2.3 Force transfer	24
4.2.4 Pressure gradient of an inflatable rubber barrier	26
4.2.5 Hydrodynamic and load conditions	27
4.3 Upscaling Maeslant barrier	31
4.3.1 Filler	31
4.3.2 Loads	31
4.3.3 Membrane force calculations	32
4.3.4 Crest-height	33
4.3.5 Fabric	40
4.3.6 Abutments	41
4.3.7 Bottom recess structure	41
4.3.8 Foundation	43
4.3.9 Bed protection	44
4.3.10 Maintenance	44
5 Conclusion and recommendations	47
5.1 Conclusion	47
5.1.1 Advantages	47
5.1.2 Disadvantages	47
5.2 Recommendations	48
List of Figures	49
List of Tables	50
Bibliography	51
Appendix	55

1 Introduction

1.1 Problem description

The Netherlands are protected against flooding by a sophisticated system of barriers, dams and other hydraulic structures. To protect Nord Holland and the city of Amsterdam in 1927 the start of Holland's first extreme engineering project of the modern era began, the Afsluitdijk. The north was now protected but in the south the North Sea could still flood a very large part of the country. After the major flood of 1953 the Delta Works was initiated. The Delta Works is the largest flood protection project in the world which consists of a number of surge barriers and a considerable length of dams. This project is updated each year in which the strategy for the future is stated. The current strategy for the Dordrecht and the Rhine mouth is to maintain the current situation with the Maeslant barrier and strengthening of the existing dikes.

The Maeslant barrier is a storm surge barrier in the province South Holland (Zuid-Holland). The barrier is located in the new waterway as shown below in figure 1 and consists of two movable arms that close of the waterway in case of a storm surge. The whole system is fully autonomous. The Maeslant barrier was the final piece of the Delta works and is part of the Europortbarrier.

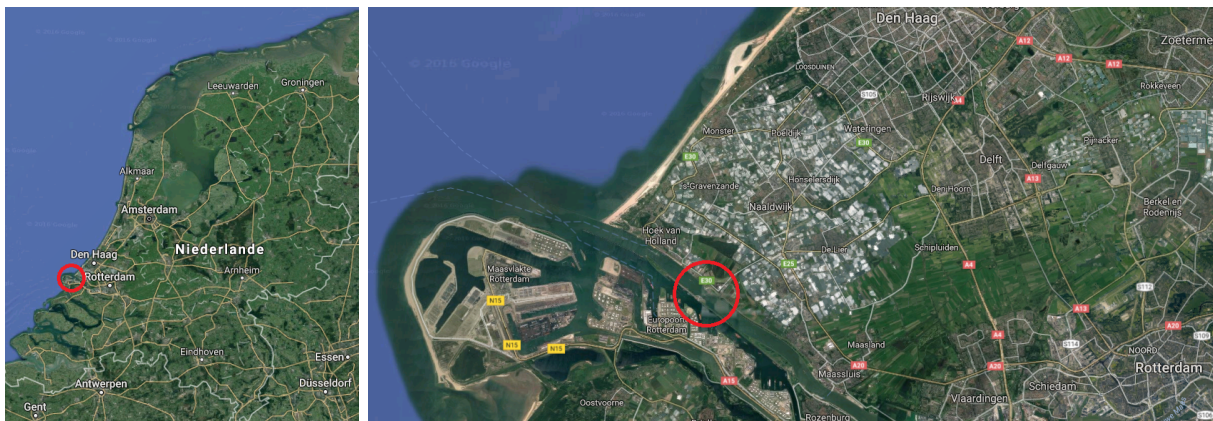


Figure 1: Location of existing Maeslant barrier

However the present barrier seems to fulfil his function, several Dutch experts put this strategy in question for different reasons:

- The closure reliability of the Maeslant barrier is not high enough. Its probability of failure is 1/100 per closure
- The barrier is not resistant against a negative head of more than 2.5 m. Problems will be caused in a situation of a closed barrier and a high water discharge from the Rhine and the Meuse.
- It is expected that the barrier will have to close more often in the near future due to the sea level rise. This is primarily a problem for navigation on the new waterway.
- The storm surge barrier is not suitable to manage the problems regarding the irrigation and drinking water. In case of low water in the river, the intrusion of saltwater could endanger the fresh water supply.

1.2 Objectives

The goal of this research project is to investigate an alternative solution for the upgrading of the Maeslant barrier, in order to make it more reliable for the future. The main tasks of this research project are to investigate the current situation with the Maeslant barrier and work out the shortcomings. Based on that information the requirements of an alternative concept are defined including the main dimensions. This project will include the following aspects:

- Introduction into the existing Maeslant barrier with working principles and shortcomings
- Determination of the requirements of the alternative concepts and functions
- Work out different designs
- Investigate one design in more detail

1.3 Project outline

Preliminary investigation aspects on the existing Maeslant barrier will be presented in chapter 2. That chapter is divided into the classification of the barrier, the alternative designs and the performance of the existing barrier and the last section explains what is wrong with the current design.

Based on the aspects explained in chapter 2 in chapter 3 will list four different alternative designs. First the specific functional requirements and boundary conditions for navigation and hydraulics are given, before presenting designs A to D. At the end of this chapter an evaluation of the four designs is presented including the election of the option further to be elaborated.

The design chosen in chapter 2 is worked out in more detail in chapter 4. First a case study on the biggest inflatable barrier in the world, the Ramspol barrier, is conducted. After that more general information about the design of inflatable barriers concerning loads and dimensions is determined. This knowledge can then be used for the upscaling of the new Maeslant barrier.

This report ends with a conclusion as well as several recommendations for further research on inflatable barriers and the improvement of the Maeslant barrier.

2 Analysis of existing Maeslant barrier

2.1 Classification

The Maeslant barrier is a storm surge barrier located in the province of South Holland. The barrier consists of two movable parts that can be placed into the new waterway in case of a threatening storm surge. It is a fully automated system that works on behalf of the Decision and Support System (BOS – Beslis en Ondersteunend Systeem). That system decides on itself if it will be necessary to close the barrier or not [28] [29]. The barrier is therefore to be allocated to the movable barriers and within that it belongs to the surge barriers. Storm surge barriers have two opposing primary functions, namely retaining water and allowing passing of ships and/or water. The functions are generally not fulfilled at the same time. The need to maintain the tidal movement in the estuary for environmental reasons led to the construction of a movable storm surge barrier instead of a closed dam and navigation locks.

The Maeslant barrier is part of the greater project called Delta Works, which is a series of hydraulic projects in the south-west of the Netherlands to protect a large area including the port of Rotterdam from the North Sea. The area protected by the barrier includes around 1 million people. Figure 2 shows a cross section of the existing Maeslant barrier including the dimensions of the retaining wall.

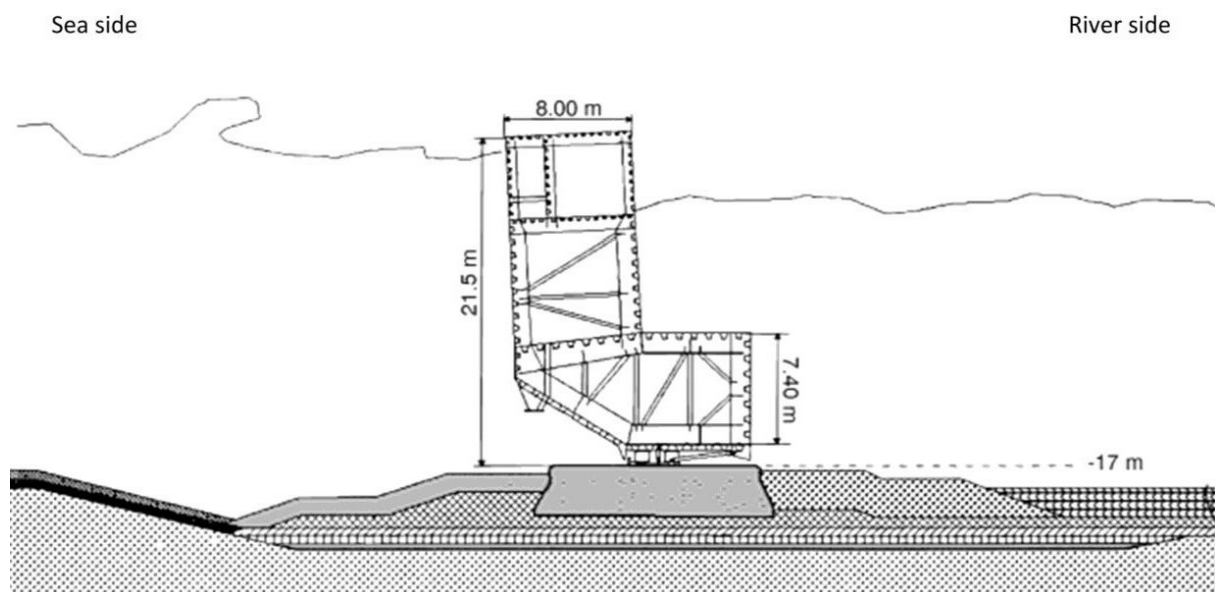


Figure 2: Cross-section of existing Maeslant barrier

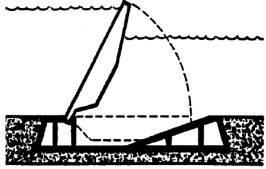
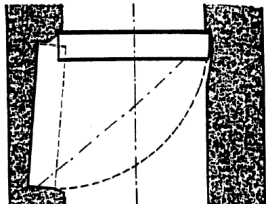
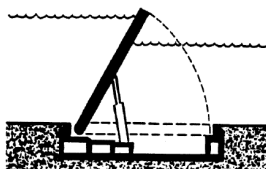
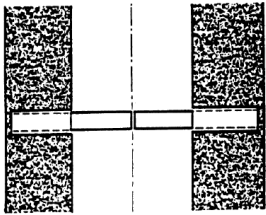
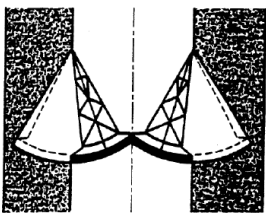
2.2 Alternative gate design

In 1987 an investigation was ordered to determine whether a storm surge barrier in combination with only minimal raising of dikes would be a possible alternative for continued strengthening of dikes [28]. Results showed that it was possible and in the same year six possible designs were presented [36] as shown in table 1 below.

All six designs had different advantages and disadvantages. Due to the easy inspection and

maintenance as well as for the safety of the gates the floating gates were chosen to be the best option.

Table 1: Alternative gate designs

Design	Schematics	Advantages	Disadvantages
Pneumatic tumble gate		- Straightforward - Reliable parallel systems (14 valves) - simple construction	- Sensitive to “negative head” - Silting-up of valves and buoyancy chambers - underwater maintenance
Segment gate		- Barrier can be closed even in extreme, flood current - Simple construction systems using, proven hydraulic and offshore engineering	- Opened gates vulnerable to collision. - Silting-up of gate buoyancy chambers - Chambers cannot be emptied for, maintenance - Gate has to plough through sediment, deposits
Boat-gate		- Insensitive to silting-up - No hinged, sliding, rotating etc., parts under water (reliability and ease of maintenance)	- Process control of gate closure not easy - Design loads difficult to define (has, direct impact on closing process control)
Hydraulic tumble gate		- 24 independent valves (risk of failure, very small) - Requires little space	- Valves, caissons and bottom protection, underdimensioned - Complex foundation technology required - Awkward maintenance - Silting-up of valves
Sliding gate		- Gates well protected when open - gates easily accessible in drained, docks - simple construction system, technology, well proven in practice	- Interference with navigation during construction - Over-dimensioned - The most expensive solution
Floating sector gate		- Gates well protected when open - Easily inspection and maintenance in, drained gate docks - Well balanced design	- Entire reaction to be passed point, wise on the hinge foundation

2.3 Performance

The failure probability of closure of the barrier is 1/109 according to official numbers (determined in October 2011 [28]) however there are several publications that emanate from a higher probability of failure [16]. In 2006 several news agencies reported that the barrier is less safe than expected. In response to the problems identified several measures were determined to bring the barrier up to the required safety level. Until 2009 all necessary measures were conducted and it was reported that the probability of failure is now higher than 1/100 per closure. The measures included a series of improvements of technology, organization and staffing [28]. The Maeslant barrier was first closed in (near-) storm conditions on 8 and 9 November 2007. The barrier normally closes when the predicted water level in Rotterdam is 3.00 m above NAP. Since the probability of closure is low (once every seven to ten years on average), the decision was made to lower the closure level to 2.60 m above NAP for 2007 only. That level was reached that year and the barrier had to come in action. After that the closure level was again set back to 3.00 m above NAP. Since then the barrier never closed although critical situations appeared in 2012, 2013 and 2014. This closure at lower closure level is called verification closure to test the barrier in an actual storm situation [28].

On 1 October 2016, Rijkswaterstaat lowered the official closing levels of the Europoort barrier again. The barrier will close at 2.60 m above NAP at Rotterdam and/or 2.30 m above NAP at Dordrecht. This means that it is likely that Rijkswaterstaat will carry out the verification closure for storm conditions either this year or the next. After the trial closure has been conducted, the official closing levels will be reinstated [26].

2.4 What is wrong with the barrier?

What is wrong with the Maeslant barrier is not an easy question to answer. To start it must be said that at the moment there is nothing wrong with the Maeslant barrier as the yearly functioning closures show. The failure probability is lower than 1 in 100 closures which is sufficient according to the Water Act (water wet)[15]. At the moment there are no convincing arguments that justify changes of the barrier on a large scale.

However the discussions on the Maeslant barrier not being safe enough refers mostly to future scenarios. The barrier was put in operation in 1997 and designed for 100 years. Experts expect that the barrier already will not provide enough protection from storm surges in 2070. This is based on a reports from van Waveren et al. [9] and Spaargaren et al. [8]

In 2015 the probability that the barrier has to close was one time within five years. For 2100 the expected rise in sea level is around 0.85 m which has a normative influence on the water level in Rotterdam and Dordrecht. Therefore it is expected that then the probability of closure will on average be five times per year. This also means that the probability of failure per closure is 25 times higher than nowadays and the influence on navigation on the New Waterway is too much [15].

Furthermore there is one more problem to consider. The intrusion of saltwater in the New Waterway and the waterbodies behind the Maeslant barrier. In times that the river discharge of the Rhine and the Meuse is very low the pressuring North Sea pushes the salt water inland and endangers the drinking water supply and increases the salinization of the land.

In the current Water Act there are no standards defined for b-barriers however there is a maximum probability of failure defined in the test obligations (WTI – Wettelijk Toetsinstrumentarium). Currently there is a revision of the Water Act going on which will include the b-barriers. This will lead to the fact that then the Maeslant barrier will not be safe enough anymore and improvement needs to be conducted [25].

3 Variant study

3.1 Introduction

To be able to meet the higher safety standards in the future it is absolutely necessary to start planning an alternative solution sufficiently early. The Maeslant barrier was planned to defy the storm surges for a service life of 100 years. However people already talk to decommission the world's largest robot between 2050 and 2070. At that time the Maeslant barrier either needs to be updated or, more likely, an alternative solution will be built. In the first subsection first the general boundary conditions which are indispensable for all alternatives are presented. After that, subsections 3.3 till 3.6 show four different alternatives.

The assumption was made that closing of the New Waterway with the Maeslant barrier is still undesired in the future because of the enormous economic effects on the port of Rotterdam and the surroundings. However, with the construction of the second “Maasvlakte” many port activities are placed at the sea side of the Maeslant barrier. The economic effect of closure of this barrier will decrease. Therefore also an alternative with a full closing of the new waterway is presented in subsection 3.3.

3.2 Boundary conditions

The following general boundary conditions are applicable for all of the following alternatives. This subsection not only considers the general conditions but also explains the functional requirements, hydraulic boundary conditions and boundary conditions for navigation.

3.2.1 General boundary conditions

There are four general conditions that apply to all future designs of the storm surge barrier replacing the Maeslant barrier. At first it is necessary that the capacity in navigation of the New Waterway during construction of the new barrier remains basically the same. There should be no or very little impact on navigation during that stage. Furthermore it is indispensable to define a minimum safety level and a maximum probability of failure for the new barrier. This however might be given by the new Water Act which is still in revision. All irreplaceable parts of the new barrier besides need to be designed for a minimal life-time of 100 years and therefore the safety-level needs to be guaranteed for that span of time. All elements that are replaceable are to be designed in such a way that replacing them can easily be conducted.

3.2.2 Functional requirements

Storm surge barriers have two opposing primary functions as already mentioned above. The functions are generally not fulfilled at the same time. The most important function is to assure enough safety against storm surges. In the Netherlands this means that they have to withstand floods that appear once in 10'000 years. The second primary function is allowing passing of ships. The New Waterway has to be accessible at any time or when the movable barriers are open for the ascribed CEMT and Panamax class explained later on. Also vessels with exceptional sizes have to be able to pass the waterway occasionally. Furthermore the guaranteed

draught on the waterway should not change and stay at 17 meters. Two more functions need to be considered to the functional requirements which are the discharge of water, ice and sediments and the indemnification of saltwater not infiltrating more into the inland waterways. The latter requirement does not apply for all designs since this function is mostly not available for movable barriers.

3.2.3 Hydraulic boundary conditions

For the hydraulic boundary conditions it is necessary to distinguish between the water level seaside and the water level inland side. For the water level at the seaside several effects as storm surge, waves, tide, seiches and sea level rise need to be taken into account. At this moment the governing water level for a storm surge has a probability of exceedance of 1 in 10'000 years for the existing Maeslant barrier. The wave height is 6.95 meter for the mouth of the new waterway, which is located more westwards. The wave conditions are considerably smaller at the structure itself and are therefore not taken into account in the further design. The tidal effects are included in the storm surge levels. Seiches are oscillatory water level rises which can have a period from a few minutes to a few hours and can have a large influence on the hydraulic loads. One important effect that needs to be taken into account especially for designs in the future and structures with life-times of 100 years is the sea level rise. The estimated sea level rise until the year 2100 can be found in the Deltasenarios where the most extreme scenario is called steem (stoom) [30]. For this scenario a sea level rise of 85 cm is taken into account.

The inland water level mainly depends on the discharges of the rivers Rhine and Meuse. The discharges can be looked up in the report "Variant Landelijke Samenhang" [19]. For the Rhine the design value is 18'000 m^3/s and for the Meuse the discharge is 4'600 m^3/s . This leads to a water level on the inland side of the barrier of 2.90 m + NAP.

3.2.4 Navigation boundary conditions

For the navigation boundary conditions it is necessary to distinguish between seagoing and inland vessels. For seagoing vessels the New Panamax class applies since there are cruise ships which fall under that category. The New Panamax class defines ships that can pass through the Panama Canal. The dimensions of the New Panamax class are defined as follows [35]:

$$L * W * D = 366m * 49m * 15.2m$$

For inland vessels the New Waterway is classified within the CEMT-Class VIb [31]. The dimensions of the CEMT-Class are listed below in table 2. Since the New Waterway is also used for recreational navigation for which there is also a classification. The waterway is classified with AZM and the dimensions for that class are shown below in table 3.

Table 2: CEMT-classification

CEMT-Class	Description	Length [m]	Beam [m]	Depth [m]	Weight [ton]	Minimal bridge Height [m]
VI b	Push towing 2 x 2 convoy	185-195	22.8	2.50-4.50	6'400- 12'000	7.0-9.1

Table 3: Recreatievaart-classification

Recreatievaart-Class	Length [m]	Beam [m]	Draught [m]	Ship height [m]	Bridge height [m]
AZM	15.0	4.25	2.10	30.0	30.0

3.3 Design A - navigation locks

3.3.1 Introduction

A navigation lock (or just lock) is a civil structure used for raising and lowering boats, ships and other watercraft between stretches of water of different levels on waterways, canals and rivers. Down below the navigation lock complex of the North Sea Canal is shown in figure 3. Since these structures close of the waterway entirely, most of the times it is necessary to install a dewatering system. The locks need to be able to give access to the port of Rotterdam even for the big container ships of the new Panamax class (dimensions of ship that can pass the new Panama Canal).

Not all ships that pass the New Waterway are considered to belong to the new Panamax class. And since it is more efficient to transport smaller ships with smaller locks it is necessary to design multiple locks with different dimensions. Inland going vessels in Europe are classified with the CEMT classification. The New Waterway belongs to the CEMT-class VIb, as mentioned above in the boundary conditions for navigation. Lock dimensions should meet the dimensions of that CEMT-class.

To be able to evaluate the number of locks needed for each class, research needs to be done on current navigation on the new waterway. This data than can be upscaled to possible future navigation demand since the alternative solution for the Maeslant barrier will need to be realized way in the future.

3.3.2 Mode of operation

There is no specific mode of operation that differs from the usual locks already working. The basic principle is very easy. For a downstream traveling ship the boat enters the lock chambers, the doors get shut behind the boat. Once the doors are closed the sluice gates are raised opening pipes between the lock and the canal. The water than empties out of lock due to the difference in water pressure. While emptying, the difference in water level across the upstream and downstream gates holds the upstream gates shut. After the water level in the lock is equal to the water level downstream, the lock doors can be opened and the ship can resume his travels.

The special thing about these locks is the frequency they will have to deal with. Every six minutes a ship passes through the new waterway and needs to use one of the locks accordingly. This means that all locks have to work as fast as possible but should still be efficient. The research on this issue goes hand in hand with the research for the number of locks needed.

3.3.3 Specific boundary conditions

- At least 4 fields
- Approximately 25 m deep
- The depth of the navigation channel should not change and is set to 17 m below NAP
- All irreplaceable parts of the locks need to be designed for a minimal life-time of 100 years
- All hydraulic boundary conditions mentioned above need to be considered
- Functional requirements:
 - Making navigation possible
 - The waterway has to be accessible at any time or when the movable barriers are opened, for the ascribed CEMT class
 - Also vessels with exceptional sizes have to be able to pass the waterway
 - The guaranteed draught on the waterway should not change and stay at 17 meters below Amsterdam Ordnance Datum (NAP)
 - Assuring enough safety against flooding
 - Withstand floods that appear 1:10'000 years
 - Discharge of water, ice and sediment
 - Ensure that saltwater is not infiltrating more into the inland waterways
 - Preserve the irrigation and drinking water upstream from the barrier

3.3.4 Advantages and disadvantages

- + Low maintenance costs
- + The waterway is permanently closed off → the safety of the port is guaranteed
- + Ensure that saltwater is not infiltrating more into the inland waterways
- Waterway is completely closed off
- Navigation is slowed down because of locks, ships need to stop and accelerate again
- Dewatering system is needed to be able to pump water from one to the other side



Figure 3: The navigation lock complex at the North Sea Canal [6]

3.4 Design B - inflatable rubber barrier

3.4.1 Introduction

An inflatable rubber barrier or weir consists of some kind of foundation structure, abutments, a rubber membrane and a filler e.g. air or/and water. The rubber membrane is clamped to the foundation structure on the bottom and on both ends to the abutments. In case of a threatening storm surge, the rubber membrane can be filled with water or/and air and therefore closes of the waterway.

The biggest realized inflatable rubber barrier is the Storm Surge Barrier Ramspol in Holland. There the barrier consists of three sections, each 75 m long, 13 m wide and with a design height of 8.35 m. The New Waterway however needs a considerably longer and higher barrier. The biggest issue in designing a rubber membrane with these dimensions is the peak stresses that occur because of folds in the membrane in the inflated state. The challenge is now to come up with a new design to reduce peak stresses to make an inflatable rubber barrier possible with the now existing materials.

In the following table 4 an upscaling from the Ramspol barrier is done to the dimensions of the Maeslant barrier. According to a previous report of Marjolein van Breukelen, a master student at the TU Delft, it is already possible to handle peak stresses of 2850 kN/m with today's materials. However this is not cost efficient.

Table 4: Upscaling of forces inflatable barriers

Parameter	Ramspol	Maeslant	Scaling factor
Length [m]	75	500	6.67
Height [m]	8.35	25	3.0
Tension [kN/m]	200	600 (200*3.0)	
Peak stresses [kN/m]	950	2850 (600*4.75)	
Factor design forces	4.75	4.75	

The report of van Breukelen also gives a rough cost estimation. According to her calculations it should be possible to build a rubber barrier at the site of the Maeslant barrier for 900 million Euro. The biggest issue here is the life span of the rubber membrane.

3.4.2 Mode of operation

In the introduction a brief explanation of the mode of operation was already made, here we are going a little more into detail. As mentioned above the rubber membrane is clamped to the foundation structure, this can be done either on one side only or on two sides. For a system that needs to withstand both forces from a storm surge and from the discharge of the rivers Rhine and Meuse it is more convenient to use the two-sided method. The rubber membrane can be filled with air, water or a combination of both. However the air filled dam is by far the most commonly used. A large part of the deformation capacity and force transfer is related to the filler. In case of designing an inflatable storm surge barrier research needs to be done to find the most suitable filler. Therefore the mode of operation is also strongly related to the filler. The general concept however is to start filling the rubber membrane in case of a possible storm surge with the elected filler. The membrane starts to rise and eventually closes of the

waterway since it is clamped to the foundation structure and on both sides to the abutments. During normal water conditions the rubber membrane is stored in the bottom recess.

Once the rubber barrier is filled and the waterway is closed of, navigation will not be possible any more. Only after deflating the barrier ships can again pass through the New Waterway. This type of barrier therefore is part of the "usually open, occasionally closed" concept.

3.4.3 Specific boundary conditions

- Approximately 500 m wide
- Approximately 25 m deep
- The depth of the navigation channel should not change and is set to 17 m below NAP
- Life expectancy of the rubber membrane should be at least 25 years
- All irreplaceable parts of the barrier need to be designed for a minimal life-time of 100 years
- All hydraulic boundary conditions mentioned in section 3.2.3 need to be considered
- Functional requirements:
 - Making navigation possible
 - The waterway has to be accessible at any time or when the movable barriers are opened, for the ascribed CEMT class
 - Also vessels with exceptional sizes have to be able to pass the waterway
 - The guaranteed draught on the waterway should not change and stay at 17 meters below Amsterdam Ordnance Datum (NAP)
 - Assuring enough safety against flooding
 - Withstand floods that appear 1:10'000 years
 - Discharge of water, ice and sediment

3.4.4 Advantages and disadvantages

- + Easy to realize
- + Nearly no maintenance necessary
- + Relatively low costs
- + Convenient barring concept
- + No direct movable parts
- + Part of "usually open occasionally closed" concept
- + Opening will be wider than it is today
- + It is probably easy to adjust this system to changed safety-levels and weights
- High tension forces and stresses in the membrane
- Complicated storage of the membrane
- Still needs a lot of research
- Vulnerable to vandalism in inflated state
- Vulnerable to damage due to floating objects
- Part of "usually open occasionally closed" concept (no holding back of saltwater at state of low water in the rivers)
- If maintenance is necessary it needs to be conducted under water



Figure 4: The inflatable rubber barrier at Ramspol [5]

3.5 Design C - tumble gate

3.5.1 Introduction

The design of a hydraulic or pneumatic tumble gate was already presented as one of the alternative designs for the existing Maeslant barrier. Tumble gate barriers consist of a container that forms the liftable gate, a foundation that can store the gate under water when there is no immediate threat of a storm surge, and a system that can lift the gate from the foundation to a closed position.

These kind of gates are in general straightforward and simple constructions and therefore very reliable. However there are two main aspects that need to be carefully considered during the design stage. The first aspect is the underwater maintenance that can have a major impact on the total cost of the project. The second reason is the silting up of the valves in both closed and open stage.

3.5.2 Mode of operation

As already mentioned above it is possible to rise the gates either pneumatic or with a hydraulic system. The pneumatic system uses pressurized air to lift the gate doors and the hydraulic system uses a hydraulic press to push the gate from the foundation in its nearly upright position. For the hydraulic system even more components need to be installed under water which increases maintenance work and therefore also the costs.

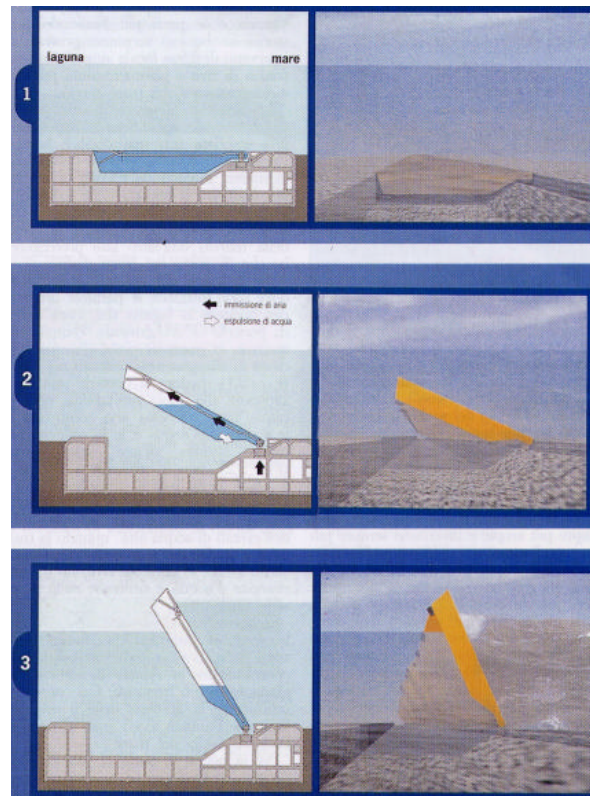


Figure 5: The MO.S.E. system for the defence of Venice [38]

3.5.3 Specific boundary conditions

- Approximately 500 m wide
- Approximately 25 m deep
- The depth of the navigation channel should not change and is set to 17 m below NAP
- All irreplaceable parts of the barrier need to be designed for a minimal life-time of 100 years
- All hydraulic boundary conditions mentioned above need to be considered
- Functional requirements:
 - Making navigation possible
 - The waterway has to be accessible at any time or when the movable barriers are opened, for the ascribed CEMT class
 - Also vessels with exceptional sizes have to be able to pass the waterway
 - The guaranteed draught on the waterway should not change and stay at 17 meters below Amsterdam Ordnance Datum (NAP)
 - Assuring enough safety against flooding
 - Withstand floods that appear 1:10'000 years
 - Discharge of water, ice and sediment

3.5.4 Advantages and disadvantages

- + Straightforward
- + Reliable parallel systems (multiple gates)
- + Simple construction
- + Construction has little impact on navigation

- + Part of “usually open occasionally closed” concept
- + The opening of the New Waterway could be wider than today
- Sensitive to negative head
- Silting up of the valves and buoyancy chambers
- Underwater maintenance
- Part of “usually open occasionally closed” concept
- Gates have a width of around 20 m which leads to 25 gates for a waterway with of 500 m

3.6 Design D - rising sector gate

3.6.1 Introduction

Basically, the Barrier is a series of separate movable gates positioned end-to-end across the river. Each gate is pivoted and supported between concrete piers that house the operating machinery and control equipment. Closing the Barrier seals off the new waterway from the sea. When not in use, the rising sector gates rest, out of sight, in curved recessed concrete cills in the riverbed, allowing free passage of river traffic through the openings between the piers. If a dangerously high storm surge threatens, the rising sector gates are moved up through about 90° from their riverbed position. The gates thus form a continuous steel wall facing the sea ready to stem the storm surge. Further rotation of the gates to the horizontal maintenance position renders them accessible for routine maintenance.

3.6.2 Mode of operation

The introduction already explained the basic concept of the rising sector gate. The closing and opening mechanism mostly consists of hydraulic pressures that are situated inside the piers in the water. Like this, every sector gate has two pressures which gives a redundancy but also allows to smaller design of the hydraulic system.



Figure 6: The Thames Barrier flood defence London [39]

3.6.3 Specific boundary conditions

- Approximately 500 m wide
- At least 4 fields
- Approximately 25 m deep
- The depth of the navigation channel should not change and is set to 17 m below NAP
- All irreplaceable parts of the barrier need to be designed for a minimal life-time of 100 years
- All hydraulic boundary conditions mentioned above need to be considered
- Functional requirements:
 - Making navigation possible
 - The waterway has to be accessible at any time or when the movable barriers are opened, for the ascribed CEMT class
 - Also vessels with exceptional sizes have to be able to pass the waterway
 - The guaranteed draught on the waterway should not change and stay at 17 meters below Amsterdam Ordnance Datum (NAP)
 - Assuring enough safety against flooding
 - Withstand floods that appear 1:10'000 years
 - Discharge of water, ice and sediment
 - Ensure that saltwater is not infiltrating more into the inland waterways
 - Preserve the irrigation and drinking water upstream from the barrier

3.6.4 Advantages and disadvantages

- + Quick operation as they move in an arc
- + The entire water load is transferred radially through the arms to the pivots which gives reduced hoist capacity
- + Gives an unobstructed passage for movement of river traffic
- + Torsionally rigid to allow operation from one side
- + Easy to build without influencing navigation too much
- + Part of “usually open occasionally closed” concept
- + Opening will be wider than it is today
- A recess is required in the floor for the gate to occupy which could be a problem where there is high silt content
- Fixed structures within the waterway
- Internal confined space for maintenance
- Curved fabrication requires an assembly jig
- Negative head could be a problem
- Maintenance closes off the field for navigation

3.7 Evaluation

Apart from upgrading the existing barrier the four options given above are all possible future solutions. However only the navigation lock design takes all functional requirements into account given in subsection 3.2.2. All other options are not suitable for the functional requirement of not letting salt water infiltrating the inland waterways in case of low discharge levels from the rivers Rhine and Meuse. For this project this requirement is set to nice-to-have and can therefore be neglected in the evaluation.

Taking previous studies at the TU Delft into account the navigation lock has already been introduced several times and therefore no new solution can be presented within this report. The tumble gate design has three major disadvantages with silting up of the valves, sensitivity to negative heads and the underwater maintenance. The advantages of the structure don't make up for the disadvantages and therefore also design C is not considered to be the best option. The rising sector gate seems to be a very good solution with a lot of advantages compared to the other solutions. Since silting up of the bottom recess is a real problem in the New Waterway a solution with storage of the main retaining structure at the bottom of the waterway might not be the best solution. Furthermore fixed structures are needed within the waterway which decreases safety on the New Waterway. The inflatable rubber barrier also is stored at a bottom recess but since it is closed structure no silting up of the storage can occur. Maintenance work of the membrane can be done from the inside so there are no high costs for under water work. The biggest point that needs to be considered is that the dimensions of an inflatable barrier in the New Waterway are around three times bigger than ever realised. The following table 5 shows a simple multi criteria analysis of the four designs according to aspects discussed before.

Table 5: Multi Criteria Analysis for the four designs

Criteria:	Design A:	Design B:	Design C:	Design D:
Maintenance	+	+	- -	-
Hindrance during construction	0	0	0	0
Adaptability	-	+	-	-
Knowledge	0	-	0	0
Sensitivity to negative head	+	+	-	-
Complexity	+	+	+	-
Vulnerability to external impacts	+	-	0	0
Hindrance during operation	-	+	+	+
Silting up of the structure	0	0	-	0
Result	+2	+3	-3	-3

This table leads to the solution with an inflatable rubber barrier which is chosen to be analysed on a higher level in this report.

4 Inflatable rubber barrier

In the last chapter the inflatable rubber barrier was chosen to be analysed in more detail. To do so it is advisable to take a look at rubber barriers that are already in service to get to know the difficulties they had to deal with. The biggest inflatable barrier is the storm surge barrier Ramspol in the Netherlands. In this chapter therefore first a case study of the Ramspol barrier is conducted and after gathering some general information in section 4.2, section 4.3 presents an upscaling from the Ramspol barrier to the dimensions of the New Waterway is.

4.1 Case study Ramspol

The reason for the design of a new barrier to protect the province Overijssel in the east of the Netherlands were the extreme high water levels in 1995. These height water levels were fed by two sources: first a high discharge of the river “Vecht” and second because of wind set-up from the “IJsselmeer” (lake IJssel). Therefore a new type of storm surge barrier was designed to protect the hinterland from flooding, namely a rubber inflatable barrier. Inflatable rubber structures already have been designed before but mainly as weirs and not in the scale needed for the Ramspol barrier. The barrier consist of three inflatable, nylon-reinforced rubber dams, each 80 m long, 13 m wide and with a design height of 8.35 m. These dimensions made the Ramspol barrier the largest inflatable barrier in the world.

The most important reasons for building an inflatable barrier were that the barrier had to be able to withstand hydraulic loads in both directions, navigation is only obstructed in closed conditions, the barrier integrates well into the surroundings and it is relatively low-cost [11]. The barrier closes fully automatic if the water level is 0.50 m above NAP, however there is always personnel on site to monitor the process and to be able to intervene in case of error [23]. The pumping stations are located in the abutments

4.1.1 Substructures

As mentioned in the introduction, the barrier consists of three inflatable rubber parts. These are connected to the surrounding structures by means of abutments and clamped to the bottom of the waterway. In case of high water levels the barrier must be closed which is done by powering the pumps to fill the tube. After the storm surge has passed, the barrier needs to be deflated again and stored in the recess structure on the bottom of the waterway. The structure can be categorised into four main subsystems, namely:

1. Filling mechanisms and material
2. Rubber membrane
3. Recess structure
4. Foundation

Filling mechanisms and material

Inflatable barriers can be filled with either air or water or, like the barrier of Ramspol, a combination of air and water. The main advantages of this system are that for closure only air

compressors and for opening only water pumps are required. The water flows in without using pumps because of the head difference. The water inside remains in open condition with the upstream water at any time which makes the barrier a self-regulating system. Like this a relatively high crest height can be reached. This however results in larger tension forces in the membrane compared to only water filled barriers which is the main disadvantage of the combined filling material [3].

The most important difference between the two systems is the internal pressure gradient. The air filled barrier has a constant internal pressure, the water filled system however experiences hydrostatic pressures. Figure 7 illustrates the difference with a barrier surrounded by stationary water. Therefore the water filled barrier will have a circular and the air filled barrier a more upright shape.

As the water level reaches +0.50 m NAP the barrier is closed. The closure procedure is then started by opening the water pipes, which stand in open connection with the upstream water level. The water flows into the space under the membrane and starts to lift it. At the same time air is blown in from the top of both abutments. The increased air pressure causes the membrane to lift further. Water keeps flowing inside because of the difference in water levels. The barrier behaves as a horizontal closure (a water filled barrier would close vertically) [3]. The pressure will increase automatically to the required level of 0.44 bar due to the increase of the water level inside the membrane [11]. During deflation the air valves are immediately opened after starting the water pumps. The structure slowly starts to deflate and in the middle a V-notch is formed. Water then starts flowing over the barrier again and after a while the membrane is stored in the bottom recess [11].



Figure 7: Difference between water filled (L) and air filled (R) system[4]

Rubber membrane

The membrane of the barrier must have a high enough resistance against static and dynamic forces as well as a sufficiently large fatigue resistance level. In addition the membrane also needs to be sufficiently flexible in order to clamp it to the foundation and the abutments and to store the membrane in a bottom recess [3].

The membrane forces of the Ramspol barrier are larger than the forces on most existing rubber weirs. Therefore no ordinary proven material was applicable. After scale model tests with different materials the material for the membrane was chosen to be a so called PA6 (Polyamid 6, Nylon) in combination with rubber as matrix material. This material is widely used and has a relatively large strength. The final design consisted of a rubber body reinforced with Nylon fabrics. The rubber body consists of two thick layers of Nylon fabric and four smaller layers of Nylon fabric for the forces in warp and longitudinal direction.

The average initial strength of the membrane in the direction of the circumferential is approximately 1'900 kN/m. After fatigue loading, ageing and relaxation from prestresses, the strength

of the membrane was circa 970 kN/m. The material properties in warp and longitudinal direction differ quite a lot. The initial strength in longitudinal direction is 450 kN/m. Also the strain stiffness EA of the membrane in longitudinal direction of the barrier is considerably lower (3'200 kN/m) than the strain stiffness in circumference direction (5'700 kN/m). The lifetime of the membrane is minimal 25 years [21].

Recess structure

The forces consisting of hydraulic loads due to the hydraulic load need to be transferred from the membrane to the concrete structure of the foundation. A clamping system is introduced to attach the membrane onto the concrete bedding. This clamping system has a second function namely to make the membrane water and air tight. The basic principle is that the membrane is clutched between two plates, mostly steel, and connected to the concrete with bolts [27]. The bottom part of the clamp has ribs on top to increase the friction between the upper part of the clamp and the membrane. This also prevents the membrane of slipping in case of great tensile forces [24]. A different clamp is needed for the abutment part to reduce stress concentrations in the membrane; a clamp which is waved in two directions. The first wave direction is in the load direction, the other wave is than in longitudinal direction of the membrane [21].

After deflation of the rubber barrier, the fabric lays horizontally on the river bed and needs to be hidden away into the bottom recess. In order to facilitate the horizontal transport of the membrane roller conveyors can be arranged in the bottom recess [4]. In its deflated position, the membrane lies over the ribs and can be stabilized with a negative pressure under the membrane. The membrane must be stored in a way that there is no working surface for currents.

The membrane also has to be connected to the abutments. The requirements for this connection are very high since the membrane must be able to deform freely and leakage is unacceptable. The membrane at the Ramspol barrier was therefore connected to an inclined plane of 45° [3]. This shape of the abutment leads to enlargement of the membrane in longitudinal direction. This enlargement will cause folds in the membrane in the deflated state. Folds increase the probability of a leakage and can also be the cause for deflation problems of the barrier [21].

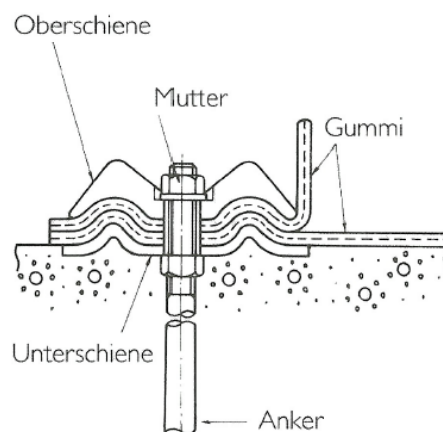


Figure 8: Waveclamp [27]

Foundation

The membrane needs to be clamped onto a concrete floor, which is in connection to the foundation. Due to the weak soft soil, a pile foundation was needed to prevent settlements. The dimensions and the configuration of the foundation depends on the load analysis.

4.1.2 Hydrodynamic and load conditions

Because of its great capacity of deformation and the way of transferring the forces down to the foundation an inflatable barrier is a special construction. Not only does the shape change with different loads acting on it, also the stiffness changes depending on the internal pressure and external loads [4]. Therefore it is necessary to distinguish the load conditions. Considering the static loads will give an insight in the equilibrium form of the structure. The behaviour of the barrier furthermore depends on various dynamic loads that need to be considered [2].

Static loads

The static loads are best explained in the situation of a closed barrier with stationary water on both sides. The response of the barrier heavily depends on the fill material and conditions. The phenomena are illustrated in figure 9 below. Starting point for a broader statically analysis is the horizontal equilibrium the hydrostatic water pressure on the outside and the internal air and/or water pressure. Where the water filled part of the barrier is in contact with the external water the barrier forms a circular arc. This also accounts for an air filled barrier in contact with the external air pressure. For all other static conditions the barrier forms a transition arc. To be able to upscale the static loads a more detailed statically analysis will be conducted later on.

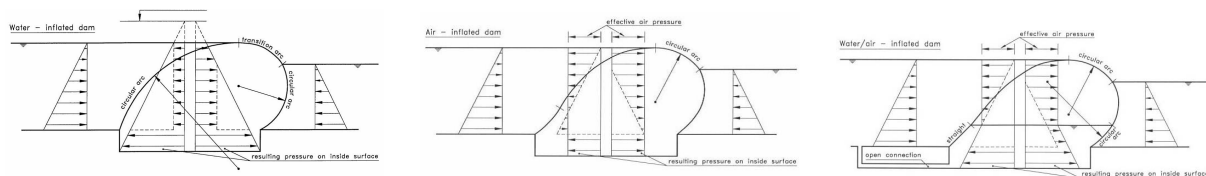


Figure 9: Different pressure curves for water filled (L), air filled (M) and air-water filled (R) [4].

Dynamic loads

Whenever the external load condition changes, the internal pressure, the shape of the barrier, the membrane forces and the contour of the barrier will adapt to it. This means the barrier deforms in function of time when it is dynamically loaded. The dynamic forces are due to wind waves, sloshing and currents. The factors which influence the dynamic behaviour of the barrier are [4]:

- The dead weight of the membrane
- The stiffness of the fabric
- The mass of the internal water
- The internal pressure head
- The incoming waves

Overflow

In case of overflow due to extreme wave heights in combination with the storm surge the barrier will behave as a weir. The overtopping water follows the circular shape of the barrier till deep into the downstream water level. The downward faced flow causes turbulences and can lead to the need of installation of a bed protection or a stilling basin in extreme conditions [4].

Wave loads

During storm conditions, when the barrier is closed, the wind generates waves which cause a dynamic load on the structure. Since wave loads are time dependent and bound to the local wave climate the barrier will deform along the pressure variation [2].

Sloshing

The water filled barrier has an internal free water surface. Dynamic loads on the structure can lead to internal vibrations with the possibility of creating an internal standing wave. This phenomenon is called sloshing of the barrier and should be avoided. The oscillation occurs when the eigenfrequency of the water inside the barrier is close to the eigenfrequency of the external dynamic wave loading [2]

4.1.3 Experience with the Ramspol barrier

In general it can be said that the barrier works according to the requirements made at the beginning of the project. During the filling and the emptying of the barrier no overpressure occurs and therefore the stiffness is very low. The low stiffness did until this day (2005) not lead to any unintentional responses such as rattling and vibrating in the membrane.

In the closed condition the barrier has to withstand great wind wave loads however this was accounted for and the barrier reacts as designed.

After deflation of the rubber barrier, the fabric lays horizontally on the river bed and needs to be hidden away into the bottom recess. In order to facilitate the horizontal transport of the membrane roller conveyors can be arranged in the bottom recess. Several deflations of the barrier have shown that the system works as designed however ships should only pass over the recess when the membrane is fully stored. Another point is that the friction in the rollers should not increase much over time otherwise the correct storing of the membrane will not work properly.

The regulation of the barrier is fully automated which lead to a couple of problems in the past. The biggest problem is the water level variation around +0.50 m NAP in which the barrier has to close and open several times in a short time period. It is possible that the barrier is opening and the water level reaches +0.50 m NAP. In this case the barrier first needs to fully open before it can close again and manual intervention is not possible.

Other positive aspects are that the lifetime of the membrane is probably more than the designed 25 years and that maintenance cost have been very low so far [4].

4.2 General theory for inflatable barriers

To be able to use the design values from the Ramspol barrier and upscale them for the new Maeslant barrier it is necessary to take a more detailed look at filling material, crest height, and force transfer as well as the different load conditions and their reactions.

4.2.1 Crest height

According to the master thesis of Marjolein van Breukelen it is hard to find any general guidelines for the design of inflatable barriers [21]. Since the cost of the membrane has a major impact on the overall cost of the project the goal is to minimize the dimensions of the membrane. A good indicator for the amount of membrane needed per unit width of the barrier is the product of the membrane force T (see subsection 4.2.5) and the circumferential length L . The membrane force determines the thickness and the amount of reinforcement needed for the membrane. The product of those factors results in the amount of membrane needed.

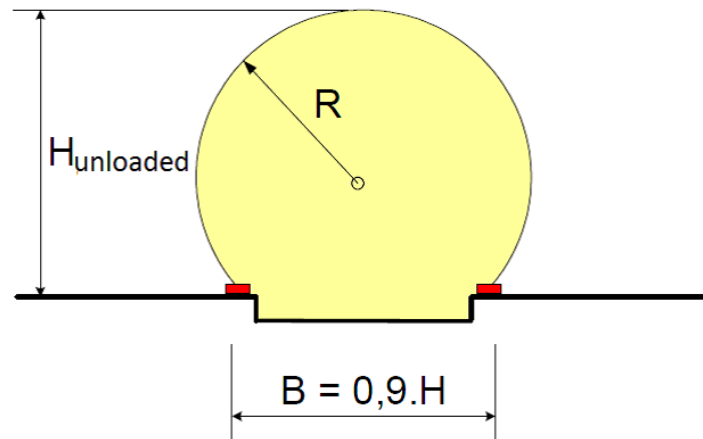


Figure 10: Two sided clamped inflatable dam [4]

The circumferential length of the membrane depends on the shape of the barrier and therefore also on the filling material. For an air filled barrier the minimal circumferential length for a certain unloaded crest height is achieved by applying 0.9 times the crest height as base width. With external loading of the air filled barrier, the crest height will increase, this means that the ignition crest height H_{unloaded} can be chosen lower than the design crest height H . Water filled or water-air filled barriers require a larger circumferential length of the membrane to reach the same crest height. The circumferential length L can be calculated by multiplying the unloaded height with a factor of 2.76 [4].

4.2.2 Filler and filling mechanism

As already briefly explained in subsection 4.1.1 an inflatable barrier can be filled with air, water or a combination of both. The air filled dam is the most common used rubber barrier and covers almost 90 % of inflatable dams in Europe [21]. The force transfer and the deformation capacity of the barrier are strongly related to the filler. The stiffness of the inflatable barrier has a direct correlation with the differential pressure which again depends on the filler. The design of the barrier is significantly intertwined with the choice of the filler. The selection of the filler is often accompanied with the following aspects [4]:

1. The speed of opening and closing of the barrier in relation to the required pump capacity
2. The desired crest height of the barrier, the circumferential length and the required internal pressure to reach this crest height
3. The magnitude of forces that occur in the membrane and on the foundation depending on internal pressure, external load and dead weight of the membrane
4. The degree of stability of the barrier
5. The influence of the compressibility of the filler on the stiffness and the dynamic behaviour of the inflatable dam
6. Are there effects on the barrier as a result of moving of the internal water
7. Specific weather conditions like freezing
8. Durability of the membrane due to the filler
9. Degree of fluctuating loads (including wave and tidal loading)

Related to the aspects above the following table compares the three different filling materials air, water and the combination of both. According to this table the combined system turns out to be the best solution.

Table 6: Advantages and disadvantages of filling material [4][21]

Aspect:	Filler: Water	Filler: Air	Filler: Water & air	Explanation
1	–	+	0	Air is relatively easy to pump into the barrier. Water however requires way more energy. For a combined system the water can flow into the barrier due to gravity force after first pumping a little air in it. For deflation however the water needs to be pumped out.
2	-	–	+	For air filled barriers the internal pressure has to be increased to withstand the external hydraulic pressure. For a water filled barrier the internal pressure can be lower. However a water filled barrier needs a larger circumferential length. The combined filler allows to keep the differential pressure to a minimum.
3	-	–	+	For an air filled barrier the tension is high due to high internal pressure, the dead weight however is low. The tensions for a water filled barrier are lower but the dead weight much higher. The combined system is in between.
4	-	–	-	A water filled dam could move along with waves. An air filled dam has the tendency to V-notching. The combined system lies in between the two singular fillers.
5	+	-	++	For the combination of water and air this is adjustable due to the ability to increase the internal air pressure
6	0	0	-	No water no water movement. The water filled system is full of water so no internal movement can occur. The combined filler can lead to internal wave creating, the system is however damped and the effects are therefore minor.
7	-	+	-	Theoretically water could freeze which would lead to loss of flexibility of the barrier. Due to a great rise in temperature the air pressure might increase.
8	0	0	0	Aggressive salts can be a problem for the membrane. The effect is the same for all fillers.
9	0	0	+	A water filled dam could move along with waves. The combined system is more stable and can withstand several load combinations.
Result:	-4	-2	+2	

4.2.3 Force transfer

In case the inflatable barrier is uniformly loaded in longitudinal direction and the characteristics are constant the bearing of loads stays in plane of the cross section. There is no transfer of loads in longitudinal direction to a neighbouring cross section. The load will be transferred in circumferential direction of the barrier to the clamping lines at the foundation. In case of non-uniform loading the loads will also be transferred in longitudinal direction. An in plane load transfer is shown in figure 11.

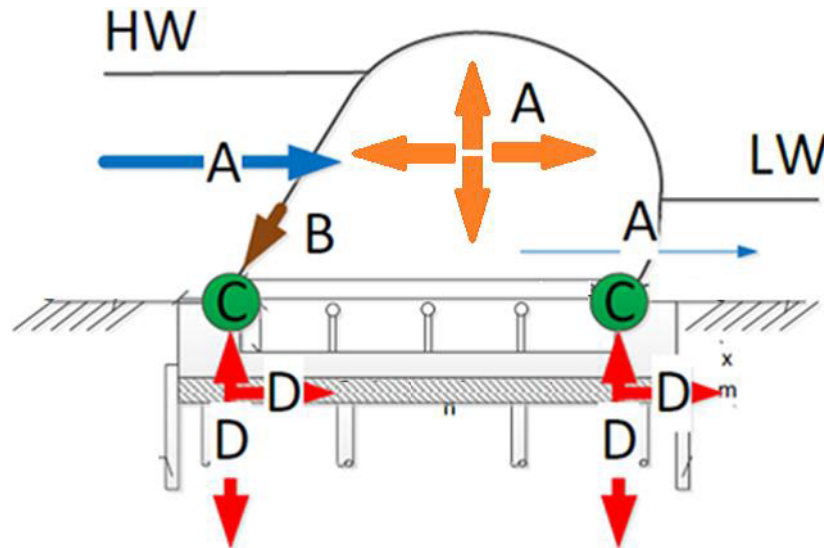


Figure 11: Load transfer inflatable rubber barrier clamped at two sides[21]

- A: Load due to the difference in water levels (head) and to the internal pressure of the filler
- B: The load due to the pressure difference (p) is transferred to the two clamping lines as a tension force in the membrane
- C: The clamping line transfers the membrane force to the foundation
- D: The forces in the clamping lines are distributed over the construction via the foundation floor and if present transferred to the pile foundation

For the transfer of the forces into the ground a couple of aspects related to the filler need to be considered. With a water filled barrier a great pressure is exerted on the foundation floor and the foundation piles due to the weight of the water. This can be an advantage in case of uplift forces under the foundation but also could be considered a disadvantage in case of a soil with weak ground layers. For the latter an air filled barrier might be better which however might then need tension piles.

The membrane force T is the result of the external load and the internal pressure. To keep the membrane force small the pressure difference p needs to be minimized. For a water-air filled barrier figure 12 shows internal pressures and external loads as well as the resulting pressure difference p .

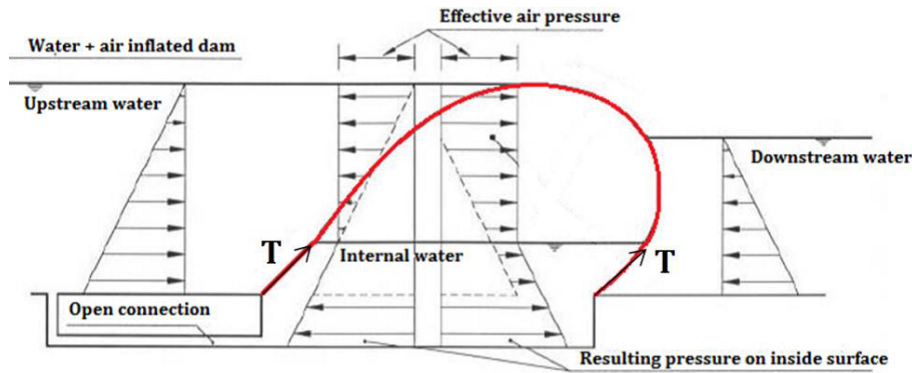


Figure 12: Acting forces on water-air filled rubber barrier [21]

For a two-sided clamped inflatable rubber barrier, the distribution of the horizontal load due to the difference in water levels ($H = 1/2 \cdot \rho \cdot g \cdot (h_1^2 - h_2^2)$) over both clamping lines is determined by the angle ϕ between the membrane and the horizontal. This situation is displayed below in figure 13.

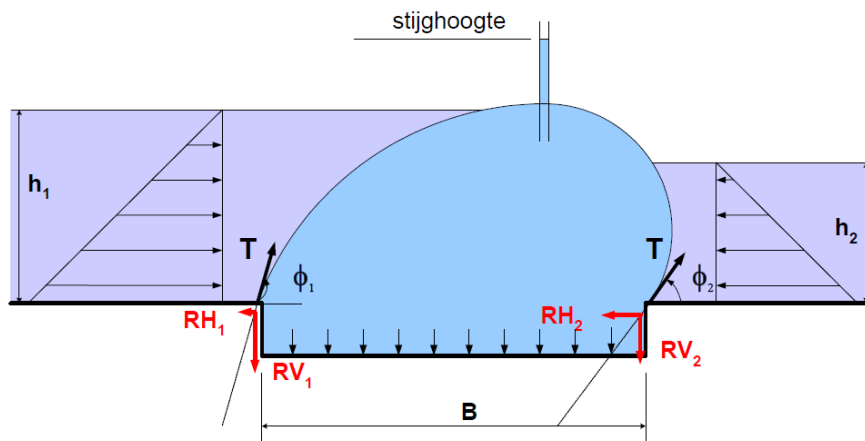


Figure 13: Load distribution over both clamping lines [4]

The horizontal and vertical component of the clamping forces as indicated in figure 13 can be calculated as follows:

$$RH = -T \cdot \cos(\phi)$$

$$RV = -T \cdot \sin(\phi)$$

The main part of the horizontal load H is transferred to the clamping line with the smallest angle ϕ . Depending on the shape of the barrier this can be on either side of the barrier. In an air filled dam with not too much internal pressure, the angle ϕ_1 is smaller than the angle ϕ_2 and so the main part of the horizontal load is transferred to the upstream clamping line. For a water filled dam the angle ϕ_1 is larger than the angle ϕ_2 .

The horizontal equilibrium leads to:

$$H = -(RH_1 + RH_2) = T(\cos\phi_1 + \cos\phi_2)$$

Because of the over pressure inside the barrier the resulting forces at the clamping are always tension forces. The distribution of the horizontal load as prescribed above can be influenced by the internal pressure: the higher the internal pressure is set, the stiffer the barrier and the less the barrier is tilted by the given loads. Therefore the angles get greater and the distribution variates [4].

4.2.4 Pressure gradient of an inflatable rubber barrier

In subsection 4.1.2 it was already briefly discussed that the pressure gradient is related to the filling material used. This section will discuss the pressure gradient for the three main fillers.

Water filled inflatable rubber barrier

The piezometric head in the barrier is a constant quantity and the internal pressure is hydrostatic. Figure 14 shows that there is a constant resulting pressure at the place where there is internal and external water. This leads to a circular shape of the barrier in case the dead weight is neglected. The sections not in contact with the outside water have a non-circular transition arc shape.

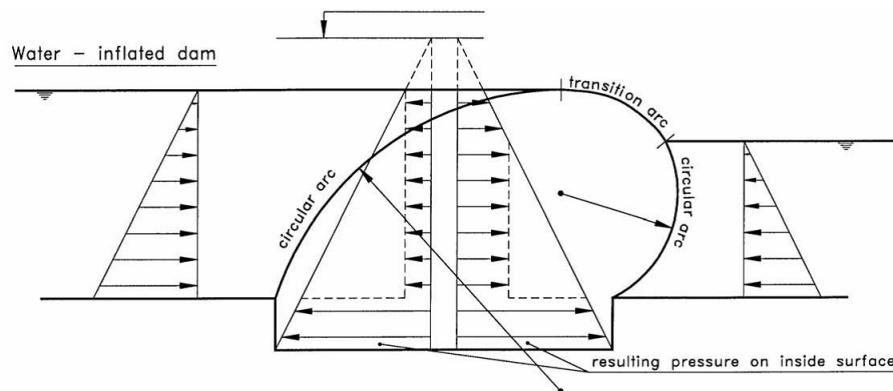


Figure 14: Pressure water filled inflatable rubber barrier [4].

Air filled inflatable rubber barrier

The internal pressure is constant in a air filled rubber barrier. The resulting pressure on the membrane now is constant on the section of the barrier in contact with the external air. This is shown below in figure 15. This again leads to a circular shape of the barrier. At the sections of the barrier in contact with water the resulting pressure is linear and the shape is non-circular again. The curvature of the dam is directed inwards at the sections where the outside water pressure is greater than the effective internal pressure.

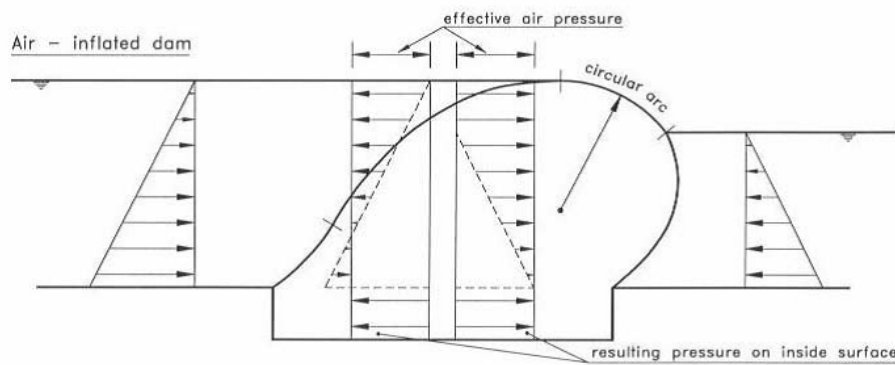


Figure 15: Pressure air filled inflatable rubber barrier [4].

Water-air filled inflatable rubber barrier

The pressure gradient for a water-air filled barrier is more complicated (see figure 16). At the section of the barrier where no resulting pressure exists (see left bottom corner in figure 16) the barrier has a linear shape in case the dead weight is neglected. Circular shapes result at sections where either there is both, internal and external, air or water. For a water-air filled barrier the constant air pressure needs to be added to the hydrostatic pressure of the water. The internal water can be in open connection with the water upstream. The internal water level is limited by the minimum air pressure needed so no sagging will occur in the membrane.

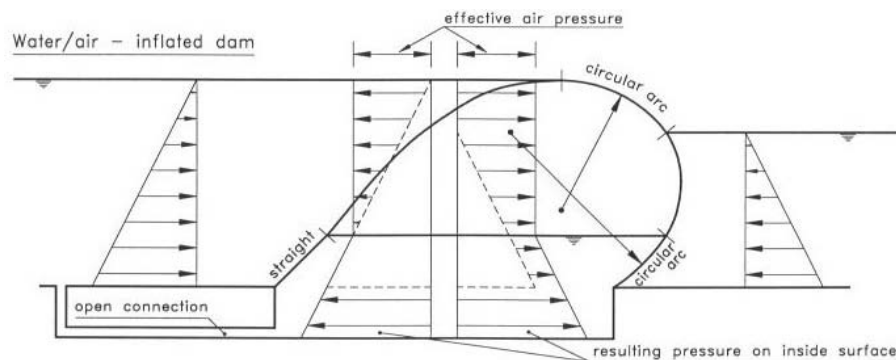


Figure 16: Pressure water-air filled inflatable rubber barrier [4].

4.2.5 Hydrodynamic and load conditions

Static Analysis

To be able to use the Information about the Ramspol barrier to upscale for the Maeslant barrier it is important to understand how the membrane forces behave. Because the force in longitudinal direction is very small in comparison to the force transfer in normal direction only forces in circumferential direction are analysed. Inflatable rubber barriers can be analysed by the “Classical Form Finding” technique according to the book on Structural Design-Special structures by J.L. Coenders [18]. This technique is based on the “form follows force” principle. This means that the shape of the inflatable barrier depends on the forces acting on it. The internal and external forces acting on a cylindrical and on two sides clamped barrier are shown in figure 17.

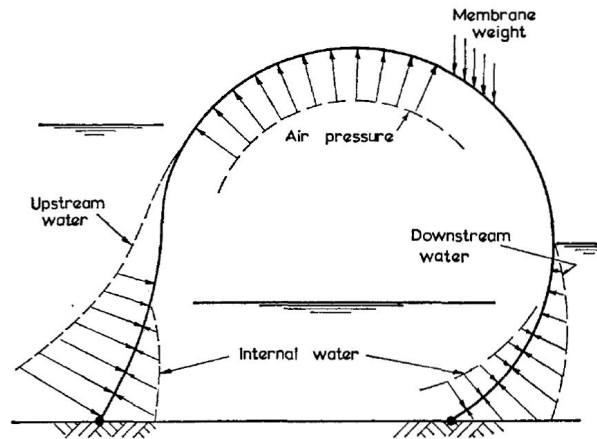
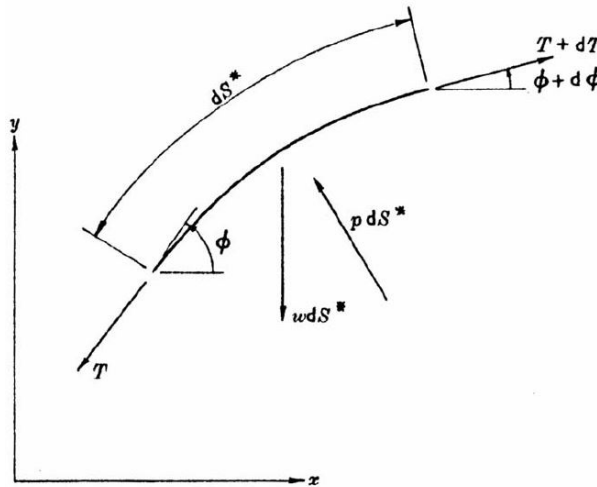


Figure 17: Internal and external loading on a membrane dam [10]

The main theory behind the numerical analysis of an inflatable rubber barrier has been published by various experts; in particular H. B. Harrison, 1970 [10] and R. D. Parbery, 1978 [32]. Looking at a cross-sectional element of the membrane with a linear load in longitudinal direction the equilibrium can be shown. This can be done because the membrane transfers the tension force over the entire length of the two clamping lines. Each element immediately receives a deformation with an equilibrium between internal pressure, external loads, the dead weight of the membrane and the axial membrane forces [4]. The principle is shown in figure 18.

Figure 18: Forces on an element with length dS^* [4]

The static equilibrium in one element with an unloaded length dS and a loaded length dS^* (S = coordinate in circumferential direction of the membrane) is described for the limiting case of $dS^* \rightarrow 0$ [4].

Tangential equilibrium: $(T + dT) \cdot \cos(d\phi) = T + w \cdot dS^* \cdot \sin(\phi)$

Since $d\phi$ is very small $\rightarrow \cos(d\phi) \approx 1$.

This leads to $dT = w \cdot dS^* \cdot (\phi)$

Radial equilibrium: $p \cdot dS^* + (T + dT) \cdot \sin(d\phi) = w \cdot dS^* \cdot \cos(\phi)$

Since $d\phi$ is very small $\rightarrow \sin(d\phi) \approx d\phi$ and $d\phi \cdot dT \approx 0$.

This leads to $p \cdot dS^* - w \cdot dS^* \cdot \cos(\phi) = -T \cdot d\phi$

With:

T = membrane force in axial direction (N)

w = force due to dead weight of the membrane (N/m)

p = resulting pressure on the membrane (N/m)

From the tangential equilibrium (circumferential direction) of the element it follows that by neglecting the force due to dead weight of the membrane w , dT becomes zero. Therefore the membrane force T in circumferential direction is constant. From that the radial equilibrium becomes:

$$p \cdot dS^* = -T \cdot d\phi$$

In case of uniform material the extension of the membrane in circumferential direction is given by the stress-strain relation according to Hooke's law:

$$\frac{dS^*}{dS} = \frac{1}{E} \cdot \left(\frac{T}{t}\right) + 1$$

Where t is the thickness of the membrane and E the young's modulus in circumferential direction.

The curvature κ of the membrane is given by the following equation:

$$\kappa = \frac{1}{R} = -\frac{d\phi}{dS^*} = \frac{(p-w \cdot \cos\phi)}{T}$$

(A positive curvature means a deformation of the membrane in outward direction)

Furthermore:

$$dx/dS^* = \cos\phi$$

$$dy/dS^* = \sin\phi$$

The geometrical boundary conditions are:

$$S = 0: \rightarrow x = 0, y = 0$$

$$S = L: \rightarrow x = B, y = 0$$

(L = circumferential length of membrane in unloaded condition, B = horizontal distance between the clamping lines)

With the above set of equations and conditions, the shape and the membrane force T of an inflatable dam can be calculated, provided that the resulting pressure p as a function of S is known and also the dead weight w of the membrane. The calculation is bound to an iterative process [4].

For the scale enlargement of an inflatable dam it is important to know what the influence is of a larger dam (higher crest) on the membrane forces. Dorreman [17] has studied what the influence is on the membrane force is due to the variation of internal pressure, water levels external and internal, base width and membrane length. The conclusion of that study can be summarized as follows:

- A larger width of the base increases the membrane force
- A larger length of the membrane increases the membrane force
- A larger internal pressure causes larger membrane forces
- A larger external load causes smaller membrane forces

In the study of Dorreman the parameters were variated individually, what happens if two or

more parameters are changed at the same time needs further analysis.

Dynamic Analysis

In case of dynamic loads both external loads and internal pressures of the inflatable rubber barrier change as a function of time. As a result, the membrane moves and deforms as a function of time and place. Also the membrane force is subject to variation in time and place in circumferential direction and does not remain constant [21].

The dynamical analysis primary deals with the dynamical equilibrium between the loads varying in time, the inert forces, damping and elastic forces. Static and dead weight loads do not play a role in the dynamic analysis however they are still responsible for the shape of the barrier at the moment that a dynamic load acts.

The membrane force T_0 as a result of a static pressure p (p can be variable along the circumference of the membrane) is constant in circumferential direction when dead-weight is neglected. The membrane force T as a result of the dynamic pressure q varies, as well as the dynamic pressure q itself as a function of time and position along the circumference of the membrane. The dynamic pressure q includes both external loads as hydrodynamic forces (interaction forces) [4].

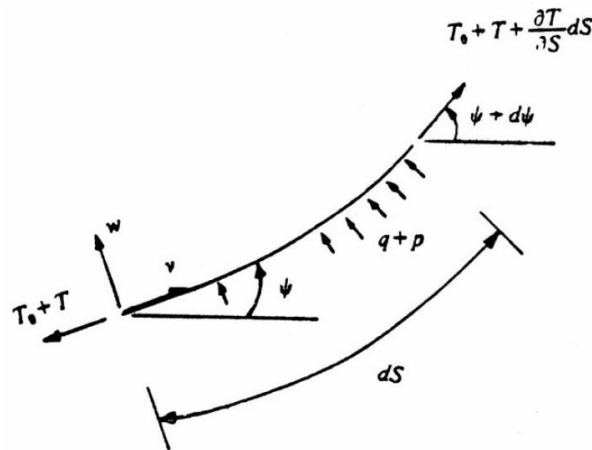


Figure 19: Dynamic equilibrium of an element of a cylindrical membrane [4]

According to figure 19 the tangential and radial equilibrium can be written as:

Tangential equilibrium: $\mu \cdot dS \cdot \frac{\partial^2 v}{\partial t^2} = (T_0 + T + \frac{\partial T}{\partial S} \cdot dS) \cdot \cos(d\phi) - (T_0 + T)$

Since $d\phi$ is very small $\rightarrow \cos(d\phi) \approx 1$.

This leads to $\mu \cdot \frac{\partial^2 v}{\partial t^2} = \frac{\partial T}{\partial S}$

Radial equilibrium: $\mu \cdot dS \cdot \frac{\partial^2 w}{\partial t^2} = (T_0 + T + \frac{\partial T}{\partial S} \cdot dS) \cdot \sin(\frac{d\phi}{2}) + (T_0 + T) \cdot \sin(\frac{d\phi}{2}) + (q + p) \cdot dS$

Since $d\phi$ is very small $\rightarrow \sin(\frac{d\phi}{2}) \approx \frac{d\phi}{2}$

This leads to $\mu \cdot \frac{\partial^2 w}{\partial t^2} = T_0 \cdot \frac{\partial \phi}{\partial S} + T \cdot \frac{\partial \phi}{\partial S} + (q + p)$

With:

T_0 = membrane force for static load p (N)

T = membrane force for dynamic load q (N)

q = resulting dynamic pressure on the membrane (N/m)

p = resulting static pressure on the membrane (N/m)

μ = mass of the membrane (kg/m)

4.3 Upscaling Maeslant barrier

In this section the information and knowledge gathered above is used to make a design for the dimensions of the Maeslant barrier. First the filler and the crest-height will be discussed, taking the chosen values aspects about the fabric will be discussed in subsection 4.3.5. With known forces that need to be transferred the abutments and after that the foundation can be designed. At the end of this section also other aspect to be considered are explained and designed. The Ramspol barrier is still taken as a reference object but due to different boundary conditions several parameters of the new Maeslant barrier deviate. The new Maeslant barrier has a greater crest-height, a larger horizontal distance between the two clamping lines and therefore also a greater circumferential length of the membrane. Also the internal pressure is a little higher. The following two tables give the difference in boundary conditions and dimensions in relation to the Ramspol barrier.

4.3.1 Filler

In subsection 4.2.2 the water-air filled inflatable barrier was considered the best system. The filler is chosen to be a combination of water and air because due to the high internal pressure and the following high tension force in the membrane an air filled barrier was not possible. For the Ramspol barrier this system was applied with success and therefore the water-air filled barrier will be adopted in the same way for the new Maeslant barrier.

The advantages of a the water-air filled barrier are:

- The inflatable barrier is during wave loads stiffer than a completely air filled barrier
- During retaining the present water in the barrier has a positive effect on the weight of the foundation since the tension forces are smaller
- The closing procedure only needs air compressors since the water is flowing in naturally inward
- Since the interior of the barrier is in open connection with the upstream water, the shape and the pressure of the inflatable barrier fits itself to the changing water levels
- Deflating the barrier only needs water pumps, the air is pressed out of the barrier due to the external water pressure

4.3.2 Loads

The different external loads acting on the inflatable membrane for the new Maeslant barrier will be discussed here. To be considered are the hydrostatic load, wave load and wind load.

Hydrostatic water pressure

The hydrostatic water pressure can be calculated with the following formula:

$$1/2 \cdot \rho \cdot g \cdot h^2$$

Table 7: Upstream and downstream water pressure

Parameter	Ramspol barrier	New Maeslant Barrier
Upstream water pressure	$1/2 \cdot \rho \cdot g \cdot 8.2^2 = 330 \text{ kN/m}$	$1/2 \cdot \rho \cdot g \cdot 22^2 = 2'374 \text{ kN/m}$
Downstream water pressure	$1/2 \cdot \rho \cdot g \cdot 3.6^2 = 64 \text{ kN/m}$	$1/2 \cdot \rho \cdot g \cdot 18^2 = 1'589 \text{ kN/m}$

Wave load

Waves cause an additional pressure on the membrane which is variable over time. This pressure depends on the wave height and the wave number of incoming waves. For the first design of the new Maeslant barrier the wave loads are neglected.

Wind load

Van Breukelen calculated the wind load for the Ramspol barrier to be 2 kN/m^2 and 8.8 kN/m [21]. These loads are negligible in comparison to the static water loads acting on the membrane. From this the assumption is made that wind load is also negligible for the new Maeslant barrier. For this reason the wind loads are not included during the design of the new Maeslant barrier.

Negative head

As the barrier is symmetrical, with two clamping lines, it is able to retain water from both sides. This means that the barrier can retain the same negative head as the head calculated from the storm surge on the sea side of the barrier. The only difference is that the inside water level is no longer connected to the upstream water anymore, but has a connection to the downstream water level at the North Sea. Since the internal air pressure is adjustable within certain boundaries this causes no problems.

4.3.3 Membrane force calculations

In subsection 4.2.1 formulas were given to make a first calculation of the needed dimensions of the barrier. These formulas however are for air filled barriers only and cannot be applied here. The responding proportions for a water-air filled barrier can be derived from the Ramspol barrier. The following relations are applicable [4]:

$$L = 2.96 \cdot H$$

$$B = 1.59 \cdot H$$

In which H is the crest height of the barrier, L the circumferential length of the membrane and B the width between the two clamping lines. This leads to the following dimensions for the new Maeslant barrier:

$$L = 2.96 \cdot 22 \text{ m} = 65.1 \text{ m}$$

$$B = 1.59 \cdot 22 \text{ m} = 35.0 \text{ m}$$

With the information given in subsection 4.2.3 it is possible to get a good first estimation of the membrane force by using the horizontal equilibrium.

$$H = 1/2 \cdot \rho \cdot g \cdot (h_1^2 - h_2^2)$$

$$H = -(RH_1 + RH_2) = T(\cos\phi_1 + \cos\phi_2)$$

These two equations lead to:

$$T = \frac{1/2 \cdot \rho \cdot g \cdot (h_1^2 - h_2^2)}{\cos\phi_1 + \cos\phi_2}$$

By assuming that the angles of the membrane are the same for the new Maeslant barrier and the Ramspol barrier one can find multiplication factor for the membrane force with the following relations [20]:

$$T_{new\ Maeslant} = \nu \cdot T_{Ramspol}$$

$$\nu = \frac{h_{1,new\ Maeslant}^2 - h_{2,new\ Maeslant}^2}{h_{1,Ramspol}^2 - h_{2,Ramspol}^2}$$

Based on this theory the following table gives the membrane force of the Ramspol barrier [4] and the expected membrane force for the new Measlant barrier.

Table 8: First estimation of membrane force

Parameter	Ramspol barrier	New Maeslant Barrier
h_1	8.2 m	22.0 m
h_2	3.6 m	18.0 m
$h_1^2 - h_2^2$	54.28 m^2	160 m^2
ν	-	2.95
T	200 kN	590 kN

4.3.4 Crest-height

With the initial parameters for the width between the clamping lines and the circumferential length of the membrane made in subsection 4.3.3 and the spread sheet made by Arjan Dirkmaat a first calculation of the crest-height can be made [1]. The spread sheet was validated by using the parameters of the Ramspol barrier given in table 9 below. For comparison the initial conditions for the Bolivar Roads barrier, designed by Marjolein van Breukelen, are included in table 9 as well. The membrane force calculated by the spread sheet was 201.33 kN/m and the actual calculations of the Ramspol barrier showed a membrane force of 200 kN/m. Therefore, it is concluded that the spread sheet approach is valid to use for the membrane force calculation.

Table 9: Parameters of the Ramspol and initial parameters of the new Maeslant barrier [21]

Parameter	Ramspol barrier	New Maeslant Barrier	Bolivar Roads
Inflatable barrier height [H]	8.2 m	22.0 m	19.0 m
Base width [B]	13 m	35 m	31.8 m
Circumferential membrane length [L]	24.3 m	65.1 m	59.2 m
Internal air pressure (P_{air})	37.3 kN/m^2	59.7 kN/m^2	58.0 kN/m^2
Internal water level [h]	3.77 m	17.0 m	13.2 m
Upstream water pressure [P_{ext}]	281 kN/m	2'374 kN/m	1'815 kN/m
Downstream water pressure [P_{ext}]	90 kN/m	1'589 kN/m	724 kN/m

To be able to calculate the equilibrium per element length dS a distinction has to be made between the different loadings of the elements. Figure 20 shows the different loading cases that can occur in a water-air filled inflatable rubber barrier, whereas table 10 gives the pressure formulas for the different locations.

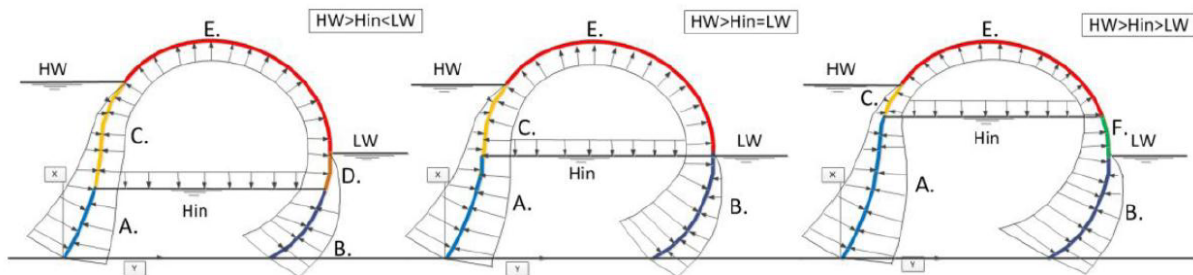


Figure 20: Occurring load situation along the length of the membrane [21]

Table 10: Applying pressure equations along length of membrane [17]

Location	External pressure	Internal pressure	Resulting pressure p
A + B	water	water	$\rho_w \cdot g \cdot (H + h - H_w)$
C + D	water	air	$\rho_w \cdot g \cdot (H - H_w + y_i + \Delta s \cdot \sin(\alpha/2))$
E	air	air	$\rho_w \cdot g \cdot H$
F	air	water	$\rho_w \cdot g \cdot (H + h - y_i - \Delta s \cdot \sin(\alpha/2))$

Using the given formulas above the spread sheet first calculates the angle of the membrane using the resulting pressure for each element depending on its location and dividing it by the initial tension force. That equation is derived from the radial equilibrium given in subsection 4.2.5

$$p \cdot dS^* = -T \cdot d\phi \quad \rightarrow \quad d\phi = \frac{p \cdot dS}{T}$$

The next step is to calculate the resulting pressure inside the barrier using the internal air and water pressures:

$$P_{in} = \frac{dP \cdot g}{10} + \rho_w \cdot g \cdot (H_{in} - Y_{coordinate})$$

Using the internal pressure and the hydrostatic water pressures up- and downstream the pressure difference ΔP for each element of length dS can be calculated depending on its position.

With the resulting pressure difference ΔP and the angle of each element the horizontal and vertical forces can be calculated using the following equations:

$$F_{horizontal} = \sin(\phi) \cdot (dS \cdot b) \cdot \Delta P$$

$$F_{vertical} = \cos(\phi) \cdot (dS \cdot b) \cdot \Delta P$$

In which b is the width of the barrier you are looking at, in this case b is set to 1 meter. From the horizontal and vertical equilibrium in each element the Tension force follows with:

$$T = \sqrt{F_{horizontal}^2 + F_{vertical}^2}$$

Finding the optimal shape of the rubber barrier and its resulting horizontal and vertical forces is

coupled to an iterative process which can be solved by the use of the solver in the spread sheet. The solution is found when the total deviation of the initial tension force and the calculated tension force is zero. For the initial calculation the internal water level is set to 17 meters. In this initial state the membrane force is calculated to be 723.5 kN/m'. The calculations can be seen below in figure 22. The total deviation is zero and the calculated crest-height is 22.55 meters. With the tension force it is now possible to calculate the horizontal and vertical reaction forces at the clamping lines.

Considering the start of the membrane in point A and the end in point B the reaction forces are calculated as follows:

$$R_{H,A} = T \cdot \cos(\phi_A) = 723.54 \text{ kN/m} \cdot \cos(54.7^\circ) = 423.5 \text{ kN/m}$$

$$R_{V,A} = T \cdot \sin(\phi_A) = 723.54 \text{ kN/m} \cdot \sin(54.7^\circ) = 586.6 \text{ kN/m}$$

$$R_{H,B} = -T \cdot \cos(\phi_B) = -723.54 \text{ kN/m} \cdot \cos(239.5^\circ) = 369.1 \text{ kN/m}$$

$$R_{V,B} = -T \cdot \sin(\phi_B) = -723.54 \text{ kN/m} \cdot \sin(239.5^\circ) = 622.3 \text{ kN/m}$$

Using these values it is now possible to design the clamps at the start and the end of the inflatable rubber barrier. A more detailed calculation for the last element of the membrane in point B for the initial conditions is given in appendix A.

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	35.00	m
Lengte membraan	Lmin	65.10	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.60	Bar
Druk balg in meter waterkolom	H	5.97	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te bereken waarden		Bandbreedte	
Trek in balg	70	1000	kN
Aanvangshoek	0.15	2.5	Rad
Luchtdruk (bar)	0.01	0.6	bar

Vaste gegevens			
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m3
Dichtheid water Goedschaloord	RG	1000	kg/m3
Dichtheid water	RW	1000	kg/m3
Afmeting deel	dS	0.07	m

Doek eigenschappen			
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m3
Elasticiteit doek		5000000.00	kN/m2
Gewicht ds		1.13	kg
		11.06	N

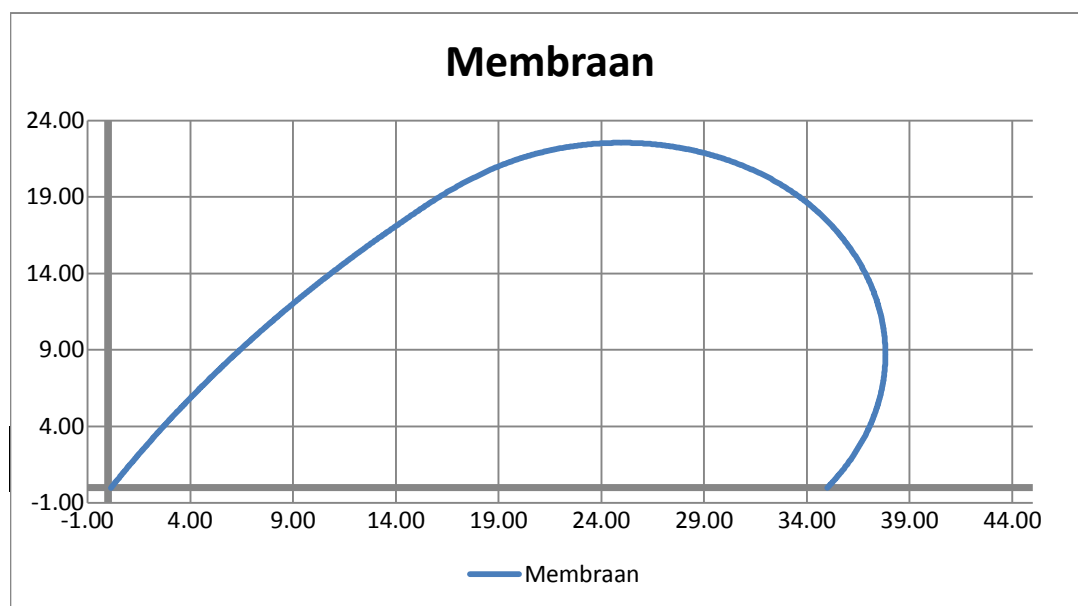


Figure 21: Crest-height calculations with initial parameters - Input

Belastingen			
Hoogwaterzijde		2`374	kN
Laagwaterzijde		1`589	kN
Hverval		785	kN
Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	369.1	423.5	792.6
RV	622.3	586.6	1`209.0
Verschil verval en horizontaal klemlijn		1%	7.78
			kN

Berekende waarden			
Tension		T	723.54
			723542
Eind hoek	ϕB	h1	4.18
			239.3
			59.3
Start hoek	ϕA	h2	0.95
			54.2
			Graden

Volumes		m3	%
Water		522.55	86.48%
Lucht		81.71	13.52%
Totaal		604.26	

Coördinaten		X	Y
Maxima		37.83	22.55
Vast punt 1		-	-
Vast punt 2		35.00	-
Onnauwkeurigheid		0.14	-0.02
		0.14	0.00
Onnauwkeurigheid		1.45	0.06
Totaal		0.06	
Factor			0.55
Ybij X max		-	
X bij Y max			24.96

Afwijkingen		m	%
Hbalg		1.45	
X		0.14	0.40%
Y		0.02	0.10%
Hverval		7.78	6.60%
Totaal			0.50%



Vaste input
 Vaste gegevens
 Berekende variabelen a.h.v. solver
 Berekende gegevens

Figure 22: Crest-height calculations with initial parameters - Output

After the initial parameter have been set and a possible solution was achieved an optimization can be conducted by changing parameters. The goal is to reach the lowest tension force in the membrane with a minimum crest height of 22 meters. The first optimization is conducted by decreasing the base width which leads to a higher crest-height.

Table 11: Membrane force and crest-height due to smaller base width

New Maeslant barrier	B= 34.5 m	B= 34.0 m	B= 33.0 m	B= 32.0 m	B= 31.0 m	B= 30.0 m
Crest-height [m]	22.62	22.72	22.91	23.07	23.10	23.28
Membrane force [kN/m']	711.30	710.07	709.87	708.04	652.12	669.40
Air pressure [kN/m ²]	0.59	0.59	0.59	0.60	0.57	0.58

From the table above it becomes clear that a base width of 31.0 meters is the best option. The second approach for optimization is to decrease the circumferential length of the membrane which will lead to a decrease in crest-height. For this optimization the base width is already set to 31.0 meters.

Table 12: Membrane force and crest-height due to smaller membrane length

New Maeslant barrier	L= 67.0 m	L= 66.0 m	L= 65.5 m	L= 65.0 m	L= 64.5 m	L= 64.0 m	L= 63.5 m
Crest-height [m]	24.23	23.57	23.33	23.12	22.89	22.62	22.38
Membrane force [kN/m']	748.8	664.6	667.6	680.5	689.2	691.4	694.13
Air pressure [kN/m ²]	0.6	0.57	0.57	0.59	0.60	0.59	0.60

This approach leads to a minimal tension force of 664.6 kN/m' with a base width B of 31 meters and a circumferential length of the membrane L of 66.0 meters. Using however the excel solver with variable internal air pressure, membrane length and base width, the optimized rubber barrier has the following parameters:

- Base width: $B = 30.58 \text{ m}$
- Membrane length: $L = 65.90 \text{ m}$
- Tension force: $T = 651.5 \text{ kN/m'}$
- Internal air pressure: $P_{in} = 56.3 \text{ kN/m}^2$
- Crest-height: $H = 23.54 \text{ m}$

The crest-height is higher than the required height of 22 meters which gives an additional safety for dynamic loads which are not included in the calculations. All calculations of the optimization can be found in the appendix B. Below in figures 23 and 24 the final design is shown with the design crest-height and the relevant water levels.

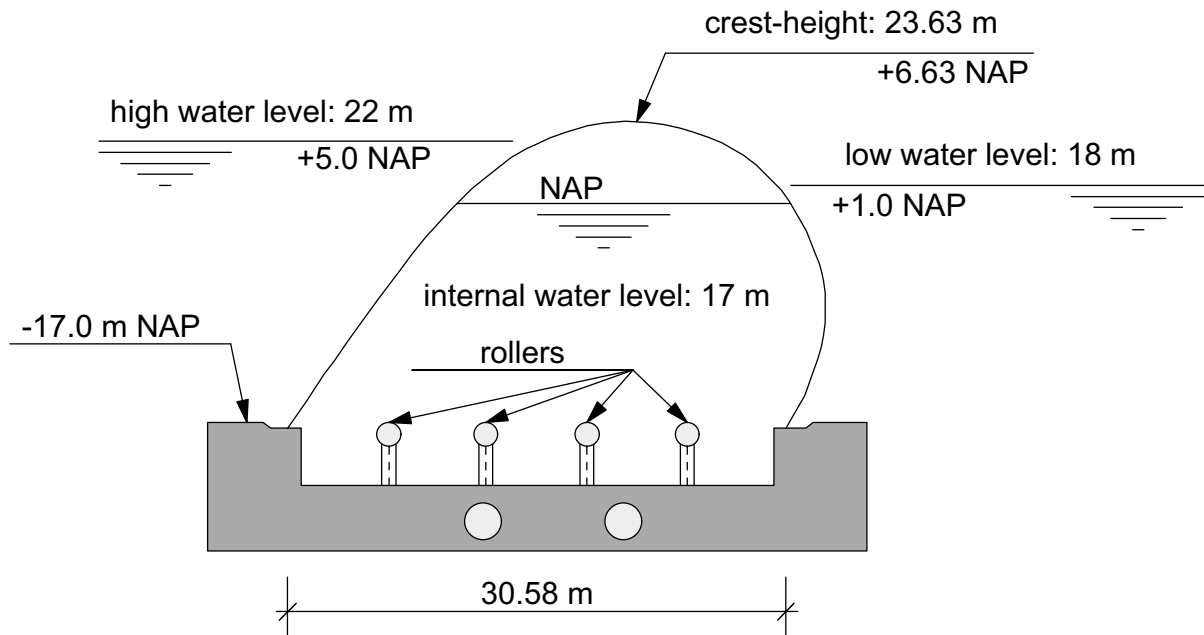


Figure 23: Final design of the inflatable rubber barrier

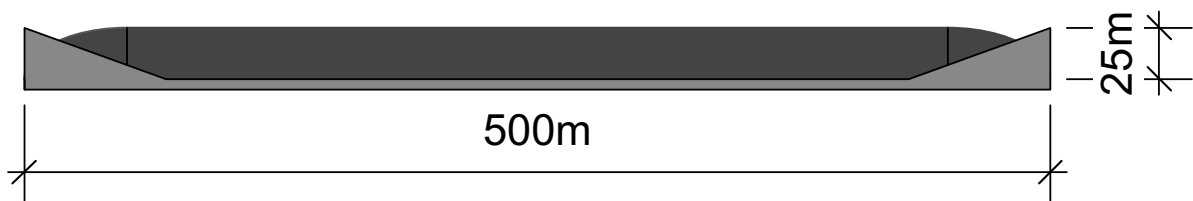


Figure 24: Possible inflatable rubber barrier in the new waterway

An additional advantage of the new inflatable barrier is the increase of cross-section of the new waterway which can be used for navigation. The additional cross-section is shown in dark blue in figure 25 below.

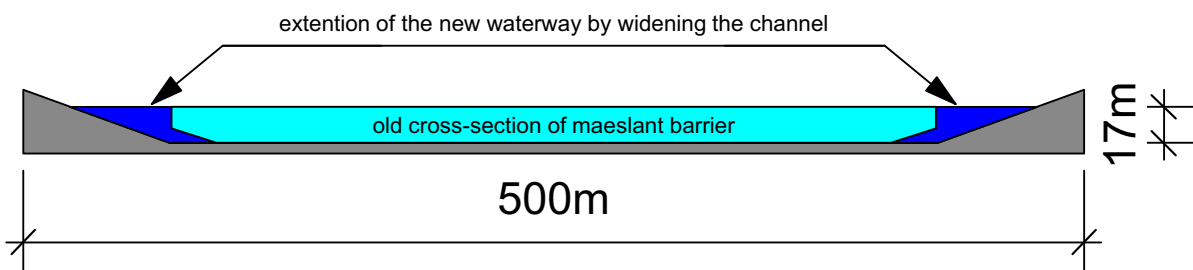


Figure 25: Cross-section comparison between old Maeslant and new inflatable rubber barrier

Marjolijn van Breukelen already made a sketch for an inflatable barrier on the site of the existing Maeslant barrier which she presented at the "waterbouwdag" in 2014. A possible design could look like shown below in figure 26

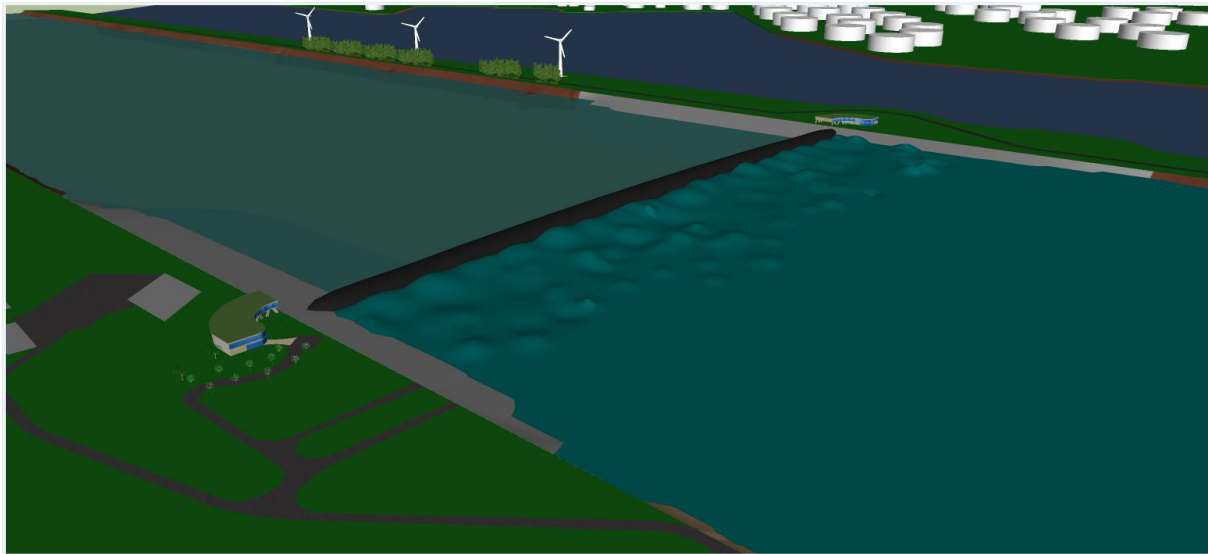


Figure 26: Possible design sketch of new inflatable rubber barrier [22]

4.3.5 Fabric

The membrane of the barrier should be strong enough to resist both static and dynamic forces and must have a sufficient fatigue resistance level. Additionally the membrane needs a sufficient flexibility for clamping and storing. In general inflatable barriers are made of a rubber coated fabric with a thickness between 10 and 20 mm with a high enough resistance against UV rays, water and oil. Normally the membrane is made of nylon reinforced rubber. However the reinforcement does not only make the membrane stronger but also stiffer. The main direction of reinforcement is in circumferential direction because of the main load transfer to the clamping lines [21].

For the Ramspol barrier the membrane material had to comply with a number of design requirements. These terms are most generally expressed as follows:

- It had to be water- and airtight under considerable pressures
- It had to be strong enough – statically and in terms of fatigue
- It had to be elastic and give a stable gate shape under all conditions
- It had to be durable, ensuring long term service

According to the Japanese design practice, the actual fabric strength should be eight times as high as the tension in the middle section of the barrier [37].

Due to increased forces in the rubber body, the design for the new Maeslant barrier cannot be based on the proven materials for existing dams or the Ramspol barrier. It is necessary to conduct tests and experiments with different types of materials and reinforcements.

Test on the fabric of the Ramspol barrier have shown that after fatigue loading, ageing and

relaxation from prestresses the strength of the membrane was nearly cut in half. However the strength was still at around 970 kN/m [21]. This means that it is possible to fabricate a membrane that will resist the forces of the new Maeslant barrier.

4.3.6 Abutments

To be able to connect the membrane of the inflatable barrier to the abutments an overlength of the membrane is necessary in two directions:

- The dimensions of the membrane must be sufficient to store the membrane properly in the recesses at the bottom and the abutments
- To prevent load transfer in longitudinal direction at the transitions to the abutments the membrane must have enough freedom of movement.

This overlength of the membrane causes folds and therefore peak stresses that are higher than the strength of the membrane. This is because of the uneven load distribution to the clamping lines. To overcome this problem a smaller slope angle of the abutments should be considered since this leads to a longer clamping length which decreases the overlength of the membrane. To reduce the overlength even more a circular clamping line could be applied at the top of the abutment. The connection of the membrane to the abutment then forms a half ellipsoid.

One big disadvantage of the ellipsoid clamping at the abutment is water leakage over the abutments due to the low connection point at the end face of the barrier. Since the existing Maeslant barrier also has leakage due to a gap between the two gates this should not be a problem here [21]. This means that it is even possible to design an inflatable barrier without any abutments which has the advantage that there will be none or just small folds in the membrane. In the case that the barrier is inactive and stored in the bottom recess it is completely invisible.

4.3.7 Bottom recess structure

In open stage when the inflatable barrier is deflated the membrane needs to either lie flat and stable on the river bed or be stored in a bottom recess structure. If the membrane is not stored properly danger in ship collision or flow blockage can occur. Collision occurred at the Ramspol barrier but lead to no damage of the membrane. In case of a collision at the new waterway the membrane will definitely be damaged due to the bigger ships passing. The flap that can occur is shown below in figure 27. Experience with the Ramspol barrier shows that flaps of around 30-60 cm occur. Since the bottom recess for the new Maeslant barrier is at -17 m NAP and the maximum draft of the design vessel is 15.2 m, the risk of collision with the flap is low but still existent.

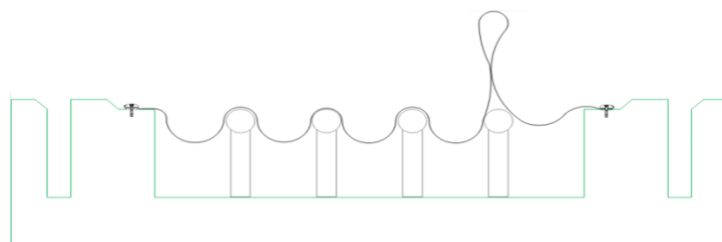


Figure 27: Cross-section of bottom recess with flap [21]

To facilitate the horizontal transport of the membrane roller conveyors can be arranged in the

bottom recess. In the deflated case the membrane than fits exactly over the ribs created by the conveyors and will be held in place by using an under-pressure. The storing process can be divided into three major phases as shown in figure 28. First the barrier is deflated and the membrane lies flat on the bottom. Than the sheet is sucked due to underpressure into the first trench at the downstream side. After that redistribution across the recess takes place until the membrane fits perfectly over the ribs.

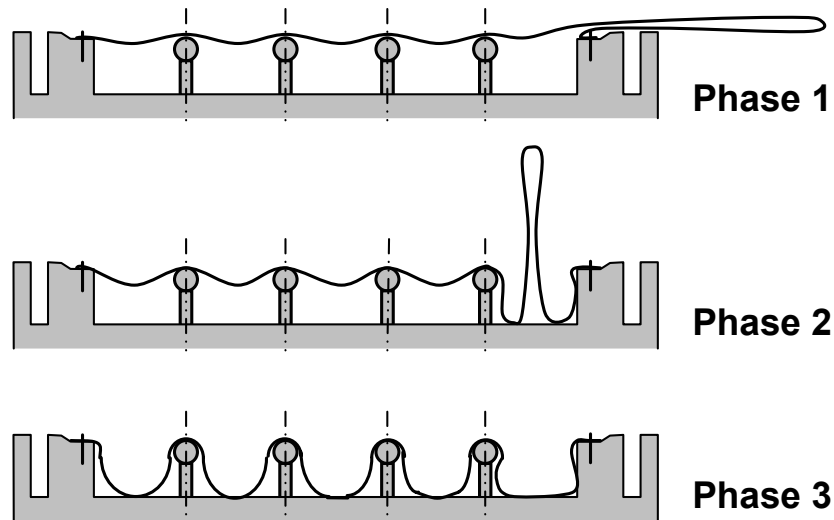


Figure 28: Cross-section of bottom recess phases 1 - 3 [37]

It is necessary to keep the friction between the bottom and the membrane as low as possible so moving in transversal direction is possible without strong friction forces. In practice it appears regularly that the membrane at the Ramspol barrier is unevenly distributed over the rollers and a flap occurs. In order to improve the storage it is possible to deflate the barrier in several stages called pressure management [21].

Figure 29 shows another possible storage concept consisting of a bottom recess with cover plates. This was already executed for the inflatable Weespertrekvaart barrier in Amsterdam. This solution however might not be suitable for the new Maeslant barrier because the cover plates with a length of around 15 meters would be too heavy to lift by inflation.

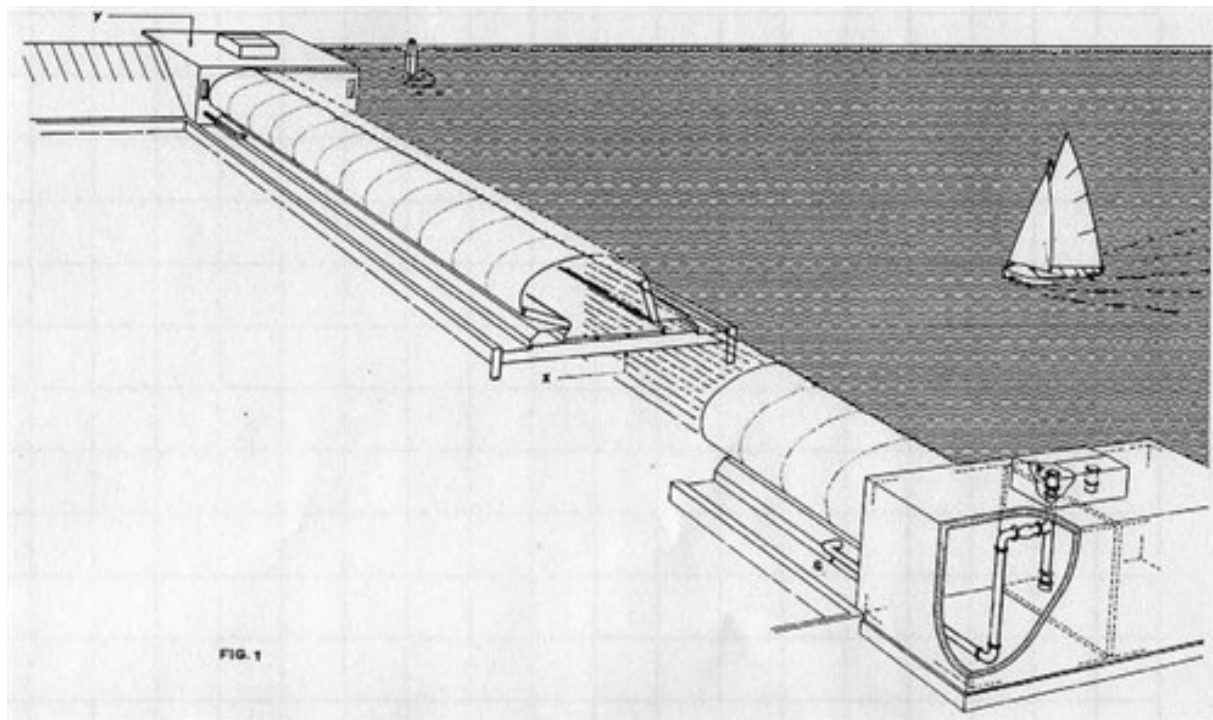


Figure 29: Inflatable rubber barrier Weespertrekvaart [34]

The conclusion is that the system with a bottom recess and rollers might be the best solution however it needs some improvement regarding the friction forces. Marjolein van Breukelen introduced a bottom recess with several small rollers. That might reduce the friction force however leads to more movable parts inside the barrier which is not preferred [21]. Also the friction in the rollers should not increase over time which also leads to the fact that a solution with more rollers is not feasible. To find a good solution to store the membrane and prevent the occurrence of flaps scale model tests with different solutions should be conducted.

4.3.8 Foundation

Normally an inflatable rubber barrier consists of a bottom recess discussed above and a foundation structure, either deep or shallow. In a first step a shallow foundation would be designed. This means that the bottom recess and a thin concrete floor would form the foundation. In case that structure is not stable enough it can be extended to a deep foundation with a pile structure as it was also applied for the Ramspol barrier. One possible solution with foundation piles is shown below in figure 30. The cases that need to be checked are static stability, dynamic stability due to waves from ship passing, settlement, buoyancy and geotechnical failure mechanisms like slip circles and piping. Piles will always be necessary in case of weak soil layers which cannot support the foundation structure. The piles will also be necessary in case buoyancy becomes a problem, however than the piles become tension piles.

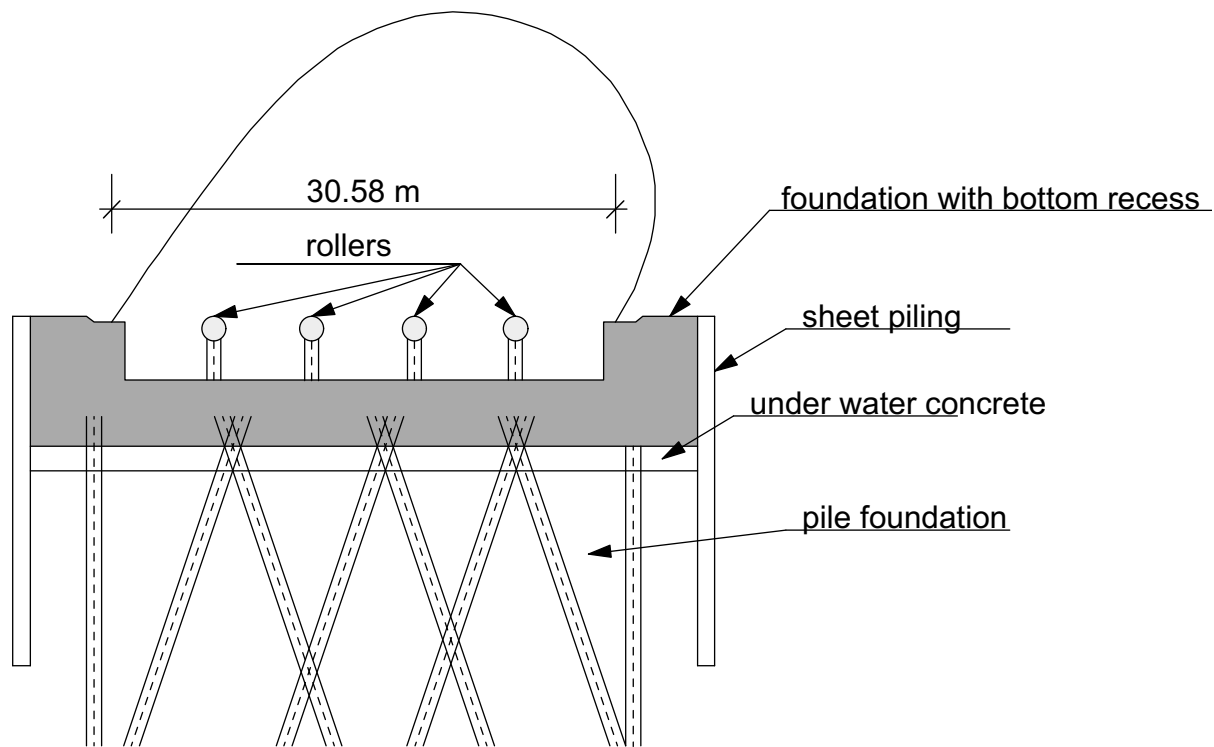


Figure 30: Possible foundation structure with piles

4.3.9 Bed protection

Because high flow velocities occur during closure at the existing Maeslant barrier a huge bed protection with several filter levels was constructed. Since the concept with an inflatable rubber barrier increases the flow area the flow velocities will decrease. For a first design it can be said that no bed protection is necessary but to in a next step flow velocity calculations will be necessary to review this statement.

4.3.10 Maintenance

Since rubber does not corrode and also does not need any painting the need for maintenance of rubber dams and barriers is minimal. However damage of the barrier can still occur due to vandalism or flood borne debris, especially sharp object. The maintenance will primary consist of periodic inspections and routine maintenance.

If maintenance becomes necessary, in principle maintenance work can be conducted from the in- and/or outside. Preferably the work is done from the inside of the barrier. To be able to do so the barrier has to be filled with air only. This causes several problems. Maintenance would be conducted during summer and normal conditions meaning that the water levels are the same on both sides of the barrier. With a water height of 18 meters the pressure on the membrane from the water load on top is 1.8 bar. This means that at least 2.8 bar is needed to lift the membrane from the floor considering the atmospheric pressure action on the water as well.

After the barrier has been totally filled with air the pressure can be reduced to the hydrostatic

pressure action at the bottom of the barrier. This is however still 1.8 bar meaning that the internal pressure can only be reduced to 1.8 bar to make sure the barrier stays inflated. The internal pressure exceeds the atmospheric pressure by 0.8 bar meaning that there is an overpressure present in which the work would have to be done. This is in general not a problem but some extra restrictions for the maintenance team apply like a maximum working time under overpressure. The entrance to the barrier consists of a room for pressure equalization. Since the barrier has a height up to 24 meters appropriate equipment like a lifting platform has to be able to fit through the entrance.

Another problem that might occur is buoyancy of the recess structure. This problem however can be contained by installing tension piles underneath the foundation.

Filling the barrier with air leads to large vertical forces on the clamping lines. These high tension forces lead to large shear stresses and bending moments in the recess structure. To reduce the bending moments a counteracting force should be present at the clamping lines. This can either be done again by the tension piles which are probably needed anyway for the buoyancy problem or by applying a load on top of the recess structure during maintenance.

If maintenance work is to be done from the outside this can either be done by divers which is expensive for greater maintenance work or by placing a sort of cofferdam around the barrier to work in a dry environment. The cofferdam could also be a good solution to load the recess structure to reduce the bending moments mentioned before. This however might also be very expensive and only be efficient if greater maintenance has to be done or for the replacement of the membrane.

Small punctures can be repaired using a sealing plug and small surface damage is to be repaired by using self-vulcanizing rubber. Small and large area damage however need a more sophisticated solution. Here the damage has to be covered with a reinforced patch that overlaps for a certain amount the damaged area. The possible damages and repair solutions for the rubber membrane are illustrated below in figure 31. In case there is such great damage that it is not possible anymore to inflate the barrier with air a temporary repair has to be conducted under water from the outside. After that it should again be possible to inflate the barrier and proper repair the rupture in the membrane. Even larger damage could ask for replacement of the whole membrane. Therefore a spare membrane should always be in stock and ready to use. For the replacement of the membrane it is probably the best solution to use the above mentioned cofferdam.

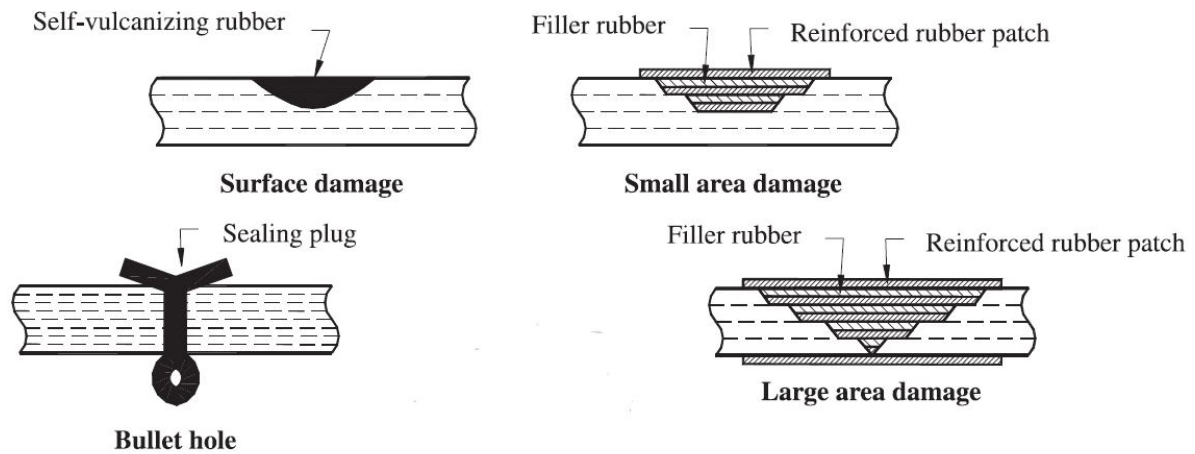


Figure 31: Possible repair techniques for different types of damage of the membrane [41]

After doing maintenance work from inside the rubber barrier it has to be made sure that really no tools and garbage is left behind because this would lead to malfunctioning of the barrier, the membrane storage and eventually lead again to damage. Also the behaviour of the membrane can be influenced by the repair patches, especially the storage behaviour of the membrane. Research has to be done on the effects of repair patches on the deflating and storage procedure.

As part of the maintenance plan periodic test closures should be conducted. It is advised to do this every year well before the storm season starts to make sure there is enough time for repair and adjustments before the barrier has to be ready for possible storm surges.

5 Conclusion and recommendations

This chapter is split in two. First the conclusions of this study are presented regarding the different designs and the inflatable barrier. The second section treats several recommendations on the topic of alternative solutions for the Maeslant barrier.

5.1 Conclusion

In this research project four alternative solutions for the upgrading of the existing Maeslant barrier were presented, a solution with a navigation lock, an inflatable barrier, a tumble gate and a rising sector gate. The design of an inflatable rubber barrier is investigated in more detail. An evaluation on the different designs was already given in section 3.7. All mentioned designs are possible future solutions for the upgrading of the Maeslant barrier. For the designs of navigation locks, tumble gate and rising sector gate reference objects can be found with similar dimensions. For the inflatable rubber barrier however the dimensions needed at the sight of the Maeslant barrier are around three times greater than ever realised with such a system. This however also leads to an interesting challenge and therefore the solution with an inflatable rubber barrier was chosen to be analysed on a higher level in this report. Following the most important advantages and disadvantages are listed.

5.1.1 Advantages

The advantages of the inflatable rubber barrier are manifold, some of the most important are given below.

- **Stiffness:** Due to the fact that inflatable rubber barriers move relatively strong with incoming waves the waves in front of the barrier are less high and lower loads appear as for rigid constructions.
- **Span:** In case of uniform loading in longitudinal direction and uniform geometric and strength properties the hydraulic load is evenly distributed leading to no restrictions to the possible span of an inflatable barrier
- **Maintenance:** Because no movable parts are placed under water and a durable rubber fabric less maintenance in comparison with traditional barriers is needed.
- **Environmental impact:** Visual pollution is low in the deflated situation and the disturbance of water flow is minime due to the fact that no constructions are placed within the pathway of the water.
- **Dead-weight:** The low dead-weight is advantageous in case of weak soil layers.
- **Fillers:** Because air and water are always present at a storm surge barrier these are the most efficient fillers however other fillers would be possible.
- **Adaptability:** In case the foundation is robust enough it might be possible just to exchange the membrane to achieve sufficient safety in the case of higher water levels.

5.1.2 Disadvantages

The inflatable barrier does however not only have advantages but also several disadvantages that need to be considered.

- **Lack of knowledge about dimensions:** There is less information and experience available on the design of inflatable rubber barriers. The largest inflatable barrier today (2016)

is still the Ramspol barrier in the Netherlands.

- **Fold formation:** The shape of the abutments can lead to folds in the membrane which increase the probability of leakage and can cause deflation problems. Folds also occurs by storing the membrane in the bottom recess. Flaps can than cause accidents with passing ships.
- **Peak stresses:** Peak stresses occur mostly around the abutments due to its design. The main problem is the uneven distribution of normal forces.
- **Resonance / sloshing:** It is possible that when the incoming wave frequency is close to the natural frequency of the barrier a strong resonance occurs. Sloshing can occur in air-water filled barriers where the internal water is brought into wave-like motion.
- **Position under water:** An inflatable rubber barrier is situated below water which can be a disadvantage for maintenance and inspection. However most of this can also be done from the inside of the barrier.

Following from this list of positive and negative points on inflatable barriers and the information worked out in this report it can be concluded that it is feasible to design a inflatable rubber barrier for the new waterway.

The first calculations on tension force in the membrane lead to the conclusion that these can be transferred to the foundation with existing rubber materials. The filler is chosen to be a combination of water and air because due to the high internal pressure and the following high tension force in the membrane an air filled barrier was not possible. The critical aspects in the design are than peak stresses in the membrane due to folds. Due to that the design membrane force can be way higher than the static force. Another problematic aspect is the storage of the membrane in the deflated situation. By incorrect storing it is possible that flaps appear which can lead to collision with ships.

5.2 Recommendations

Within this research no finite element modelling has been performed. For further analysis of an inflatable rubber barrier at the sight of the existing Maeslant barrier, it is important to perform finite element modelings in order to determine the exact forces appearing in the membrane.

Till this moment only 2D calculations have been performed, the load transfer however also appears in longitudinal direction of the barrier. Furthermore the dead weight of the membrane was neglected and also dynamic loads were not considered in the first calculations. To determine more accurate values for the forces in the membrane all the above mentioned aspects need to be included. In this report it was mentioned that peak stresses can occur, however these were not specified. Since peak stresses can determine the feasibility of such a barrier, in a next step they should be considered.

The next step would be to specify the conditions in more detail and higher accuracy to be able to take the design of the inflatable rubber barrier to the next level.

List of Figures

1	Location of existing Maeslant barrier	1
2	Cross-section of existing Maeslant barrier	3
3	The navigation lock complex at the North Sea Canal [6]	9
4	The inflatable rubber barrier at Ramspol [5]	12
5	The MO.S.E. system for the defence of Venice [38]	13
6	The Thames Barrier flood defence London [39]	14
7	Difference between water filled (L) and air filled (R) system[4]	18
8	Waveclamp [27]	19
9	Different pressure curves for water filled (L), air filled (M) and air-water filled (R) [4].	20
10	Two sided clamped inflatable dam [4]	22
11	Load transfer inflatable rubber barrier clamped at two sides[21]	24
12	Acting forces on water-air filled rubber barrier [21]	25
13	Load distribution over both clamping lines [4]	25
14	Pressure water filled inflatable rubber barrier [4].	26
15	Pressure air filled inflatable rubber barrier [4].	27
16	Pressure water-air filled inflatable rubber barrier [4].	27
17	Internal and external loading on a membrane dam [10]	28
18	Forces on an element with length dS^* [4]	28
19	Dynamic equilibrium of an element of a cylindrical membrane [4]	30
20	Occurring load situation along the length of the membrane [21]	34
21	Crest-height calculations with initial parameters - Input	36
22	Crest-height calculations with initial parameters - Output	37
23	Final design of the inflatable rubber barrier	39
24	Possible inflatable rubber barrier in the new waterway	39
25	Cross-section comparison between old Maeslant and new inflatable rubber barrier	39
26	Possible design sketch of new inflatable rubber barrier [22]	40
27	Cross-section of bottom recess with flap [21]	41
28	Cross-section of bottom recess phases 1 - 3 [37]	42
29	Inflatable rubber barrier Weespertrekvaart [34]	43
30	Possible foundation structure with piles	44
31	Possible repair techniques for different types of damage of the membrane [41] . .	46

List of Tables

1	Alternative gate designs	4
2	CEMT-classification	8
3	Recreatievaart-classification	8
4	Upscaling of forces inflatable barriers	10
5	Multi Criteria Analysis for the four designs	16
6	Advantages and disadvantages of filling material [4][21]	23
7	Upstream and downstream water pressure	32
8	First estimation of membrane force	33
9	Parameters of the Ramspol and initial parameters of the new Maeslant barrier [21]	33
10	Applying pressure equations along length of membrane [17]	34
11	Membrane force and crest-height due to smaller base width	38
12	Membrane force and crest-height due to smaller membrane length	38

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Technical report - Appendix

Maeslant Barrier

Alternative solution for the upgrading of the Maeslant barrier



Jonas Riteco (4167759)

16. January 2017

Cover

Closed Maeslant Barrier, Bird's eye view

Source: Beeldbank Rijkswaterstaat

https://beeldbank.rws.nl/MediaObject/Details/Luchtfoto_van_de_gesloten_Maeslantkering_in_de_nieuwe_Waterweg_nabij_Hoek_van_Holland_158813

Date: 17.10.2016

Contents appendix

Appendix A: Initial calculations	II
Appendix B: Optimization of membrane force	XIV
B.1. Variation of base width B	XIV
B.2. Variation of circumferential membrane length	XX
B.3. Total optimization	XXVII

Appendix A: Initial calculations

With the initial parameters for the width between the clamping lines and the circumferential length of the membrane made in subsection 4.3.3 and the spread sheet made by Arjan Dirkmaat a first calculation of the crest-height can be made. The initial conditions for the calculation are given below in table A.1.

Table A.1.: Parameters of the Ramspol and initial parameters of the new Maeslant barrier

Parameter	Ramspol barrier	New Maeslant Barrier
Inflatable barrier height [H]	8.2 m	22.0 m
Base width [B]	13 m	35 m
Circumferential membrane length [L]	24.3 m	65.1 m
Internal air pressure (P_{air})	37.3 kN/m^2	59.7 kN/m^2
Internal water level [h]	3.77 m	17.0 m
Upstream water pressure [P_{ext}]	281 kN/m	2'374 kN/m
Downstream water pressure [P_{ext}]	90 kN/m	1'589 kN/m

To be able to calculate the equilibrium per element length dS a distinction has to be made between the different loadings of the elements. Figure A.1 shows the different loading cases that can occur in a water-air filled inflatable rubber barrier, whereas table A.2 gives the pressure formulas for the different locations.

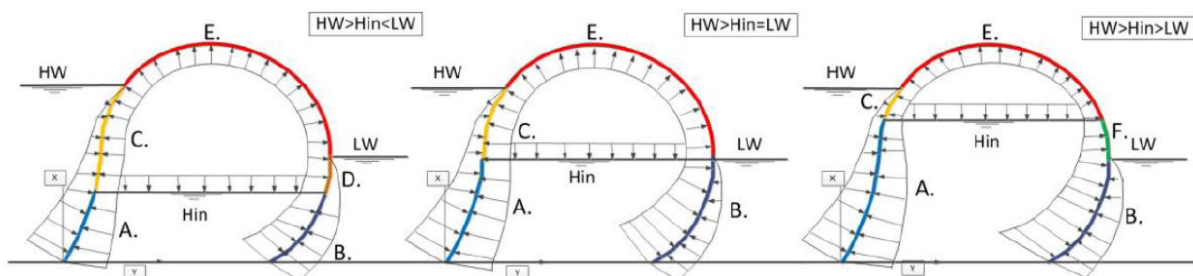


Figure A.1.: Occurring load situation along the length of the membrane

Table A.2.: Applying pressure equations along length of membrane

Location	External pressure	Internal pressure	Resulting pressure p
A + B	water	water	$\rho_w \cdot g \cdot (H + h - H_w)$
C + D	water	air	$\rho_w \cdot g \cdot (H - H_w + y_i + \Delta s \cdot \sin(\alpha/2))$
E	air	air	$\rho_w \cdot g \cdot H$
F	air	water	$\rho_w \cdot g \cdot (H + h - y_i - \Delta s \cdot \sin(\alpha/2))$

Using the given formulas above the spread sheet first calculates the angle of the membrane using the resulting pressure for each element depending on its location and dividing it by the initial

tension force. That equation is derived from the radial equilibrium given in subsection 4.2.5.

$$p \cdot dS = -T \cdot d\phi \rightarrow d\phi = \frac{p \cdot dS}{T}$$

$$dS = \frac{L_{min}}{n} = \frac{65.1 \text{ m}}{1000} = 0.0651 \text{ m}$$

$$T = 723542 \text{ N}$$

Taking the initial conditions, the pressure P and tension force T, the angle $d\phi_B$ in the last element (end of the membrane) results to:

$$\begin{aligned} d\phi_B &= \frac{p \cdot dS}{T} = \frac{\rho \cdot g \cdot (H + (H_{internal} - Y_{coordinate}) - (H_{lowwater} - Y_{coordinate})) \cdot dS}{T} \\ &= \frac{1000 \frac{\text{kg}}{\text{m}^3} \cdot 9.81 \frac{\text{m}}{\text{s}^2} \cdot (5.97 \text{ m} + (17 \text{ m} - 0) - (18 \text{ m} - 0)) \cdot 0.0651 \text{ m}}{723542} = 0.004384 \text{ rad} \rightarrow 0.25^\circ \end{aligned}$$

The next step is to calculate the resulting pressure inside the barrier using the internal air and water pressures:

$$P_{in,B} = \frac{dP \cdot g}{10} + \rho_w \cdot g \cdot (H_{in} - Y_{coordinate}) = \frac{0.59671 \text{ bar} \cdot 9.81 \frac{\text{m}}{\text{s}^2}}{10} + 1000 \frac{\text{kg}}{\text{m}^3} \cdot 9.81 \frac{\text{m}}{\text{s}^2} \cdot (17 \text{ m} - 0) \cdot \frac{1}{100'000}$$

$$P_{in,B} = 2.253 \text{ bar}$$

Using the internal pressure and the hydrostatic water pressures up- and downstream the pressure difference ΔP for each element of length dS can be calculated depending on its position. For the last element on the low water side the pressure difference is:

$$\Delta P_B = P_{in,B} - P_{LW,B} = 2.253 \text{ bar} - \rho \cdot g \cdot H_{LW,B} = 2.253 \text{ bar} - \frac{1000 \frac{\text{kg}}{\text{m}^3} \cdot 9.81 \frac{\text{m}}{\text{s}^2} \cdot 18 \text{ m}}{100'000} = 0.4872 \text{ bar}$$

With the resulting pressure difference ΔP_B and an initial angle of the last element of $\phi = 0.94 \text{ rad}$, $\phi = 54.17^\circ$ respectively the horizontal and vertical forces can be calculated using the following equations in which b is the width of the barrier you are looking at, in this case b is set to 1 meter:

$$F_{horizontal,B} = \sin(\phi) \cdot (dS \cdot b) \cdot \Delta P = \sin(54.17^\circ) \cdot (0.0651 \text{ m} \cdot 1 \text{ m}) \cdot 0.4872 \text{ bar} = 2.57 \text{ kN}$$

$$F_{vertical,B} = \cos(\phi) \cdot (dS \cdot b) \cdot \Delta P = \cos(54.17^\circ) \cdot (0.0651 \text{ m} \cdot 1 \text{ m}) \cdot 0.4872 \text{ bar} = 1.86 \text{ kN}$$

From the horizontal and vertical equilibrium in each element the Tension force follows with:

$$T_B = \sqrt{F_{horizontal,B}^2 + F_{vertical,B}^2} = \sqrt{(2.57 \text{ kN})^2 + (1.86 \text{ kN})^2} = 3.17 \text{ kN}$$

Doing these calculations for each small element of length dS by adding the difference in angle $d\phi_i$ to the angle ϕ results in horizontal and vertical forces for each element. The sum of these forces then lead to the final horizontal and vertical forces that need to be clamped. The sum of all tension forces from each element lead to the total tension force T.

Finding the optimal shape of the rubber barrier and its resulting horizontal and vertical forces is coupled to an iterative process which can be solved by the use of the solver in the spread sheet. The solution is found when the total deviation of the initial tension force and the calculated tension force is zero. For the initial calculation the internal water level is set to 17 meters. In

this initial state the membrane force is calculated to be 723.5 kN/m'. The calculations can be seen below in figure A.2. The total deviation is zero and the calculated crest-height is 22.55 meters. With the tension force it is now possible to calculate the horizontal and vertical reaction forces at the clamping lines.

Considering the start of the membrane in point A and the end in point A the reaction forces are calculated as follows:

$$R_{H,A} = T \cdot \cos(\phi_A) = 723.54 \text{ kN/m} \cdot \cos(54.7^\circ) = 423.5 \text{ kN/m}$$

$$R_{V,A} = T \cdot \sin(\phi_A) = 723.54 \text{ kN/m} \cdot \sin(54.7^\circ) = 586.6 \text{ kN/m}$$

$$R_{H,B} = -T \cdot \cos(\phi_B) = -723.54 \text{ kN/m} \cdot \cos(239.5^\circ) = 369.1 \text{ kN/m}$$

$$R_{V,B} = -T \cdot \sin(\phi_B) = -723.54 \text{ kN/m} \cdot \sin(239.5^\circ) = 622.3 \text{ kN/m}$$

$$T_{total,B} = \sqrt{R_{H,B}^2 + R_{V,B}^2} = \sqrt{(369.1 \text{ kN})^2 + (622.3 \text{ kN})^2} = 723.5 \text{ kN}$$

$$T_{total,A} = \sqrt{R_{H,A}^2 + R_{V,A}^2} = \sqrt{(423.5 \text{ kN})^2 + (586.6 \text{ kN})^2} = 723.5 \text{ kN}$$

The in- and output of the spread sheet is given below in figure A.2 and the table on the following pages.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
 0.596713 bar Luchtdruk
 35 m Breedte tussen klemlijnen
 65.1 m Lengte membraan

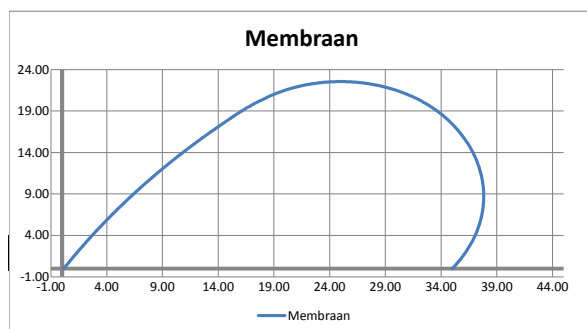
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	35.00	m
Lengte membraan	Lmin	65.10	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.60	Bar
Druk balg in meter waterkolom	H	5.97	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden	Bandbreedte		
Trek in balg	70	1000	kN
Aanvangshoek	0.15	2.5	Rad
Luchtdruk (bar)	0.01	0.6	bar

Vaste gegevens			
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschalxoord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.07	m

Doek eigenschappen			
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.13	kg
		11.06	N

**WATER/AIR****Output**

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
	Hverval	785	kN
Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	369.1	423.5	792.6 kN
RV	622.3	586.6	1'209.0 kN
Verschil verval en horizontaal klemlijn	1%		7.78 kN

Berekende waarden			
Tension	T	723.54	kN/m ²
		723542	N
Eind hoek	ϕ_B	4.18	Rad
		239.3	Graden
Start hoek	ϕ_A	59.3	Graden
		0.95	Rad
		54.2	Graden

Volumes	m ³	%
Water	522.55	86.48%
Lucht	81.71	13.52%
Totaal	604.26	

Coördinaten	X	Y
Maxima	37.83	22.55
Vast punt 1	-	-
Vast punt 2	35.00	-
Onnauwkeurigheid	0.14	-0.02
	0.14	0.00
Onnauwkeurigheid	1.45	0.06
Totaal	0.06	
Factor		0.55
Ybij X max	-	
X bij Y max		24.96

Afwijkingen	m	%
Hbalg	1.45	
X	0.14	0.40%
Y	0.02	0.10%
Hverval	7.78	6.60%
Totaal		0.50%

	Vaste input
	Vaste gegevens
	Berekende variabelen a.h.v. solver
	Berekende gegevens

Figure A.2.: Crest-height calculations with initial parameters

Coördinaten			Uitkomsten dS						Volumes						Hoek verandering						Druk						Rekenhulp									
Element	X coördinaat	Y coördinaat	1		2		3		4		5		6		af deel B	Lucht	af deel B	Hoek element		Eg doek	T	Fhor [kN]	Evert [kN]	Δdoek [kN/m²]	Δdoek [Bar]	P HW zijde [Bar]	P Balg [Bar]	P LW zijde [Bar]	Dhoek HW-WW	Dhoek HW-WL	Dhoek LL	Dhoek LW-WW	dHoek LW-LW	dHoek LW-WL	Y	X
			In S WW	DHoek HW-WL	Dhoek LL WW	WW	dHoek LW-LW	dHoek LW-WL	Water	Graden	Graden	Graden	Graden																							
1	35.00	0.00	0.07	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.945483625	54.1722213	11	3.172	2.571926163	1.8568279	48.727532	0.4872753	2.1582	2.2530753	1.7658	-	-	-	0.00	35.00	
2	35.04	0.05	0.13	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.945868	54.42341833	11	3.172	2.580042152	1.8455342	48.727532	0.4872753	2.153022107	2.2478974	1.7606221	-	-	-	0.05	35.04	
3	35.08	0.11	0.20	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.946252	54.62329456	11	3.172	2.58810855	1.834205	48.727532	0.4872753	2.147827874	2.2427032	1.7554279	-	-	-	0.11	35.08	
4	35.11	0.16	0.26	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.946582	54.82321517	11	3.172	2.5961252	1.8228406	48.727532	0.4872753	2.142617402	2.2374927	1.7502174	-	-	-	0.16	35.11	
5	35.15	0.21	0.33	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.946930	55.17700942	11	3.172	2.60409195	1.8114412	48.727532	0.4872753	2.137390791	2.2322661	1.7449098	-	-	-	0.21	35.15	
6	35.19	0.27	0.39	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.967405	55.42820645	11	3.172	2.612008645	1.8000069	48.727532	0.4872753	2.132148814	2.2270235	1.7397481	-	-	-	0.27	35.19	
7	35.23	0.32	0.46	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.971789	55.67940348	11	3.172	2.619875134	1.788538	48.727532	0.4872753	2.126889551	2.2217649	1.7344896	-	-	-	0.32	35.23	
8	35.26	0.37	0.52	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.976173	55.93060045	11	3.172	2.627691266	1.7770348	48.727532	0.4872753	2.121615126	2.2164904	1.7292151	-	-	-	0.37	35.26	
9	35.30	0.43	0.59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.980557	56.18179753	11	3.172	2.63545689	1.7654974	48.727532	0.4872753	2.116324964	2.2112003	1.723925	-	-	-	0.43	35.30	
10	35.33	0.48	0.65	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.984252	56.43299456	11	3.172	2.643171857	1.753926	48.727532	0.4872753	2.111019169	2.2058945	1.7186192	-	-	-	0.48	35.33	
11	35.37	0.54	0.72	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.988325	56.68419159	11	3.172	2.650836019	1.742321	48.727532	0.4872753	2.105697841	2.2005732	1.7132978	-	-	-	0.54	35.37	
12	35.41	0.59	0.78	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.993710	56.93538862	11	3.172	2.658449228	1.7306824	48.727532	0.4872753	2.100361084	2.1952364	1.7079611	-	-	-	0.59	35.41	
13	35.44	0.64	0.85	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	0.998094	57.18658565	11	3.172	2.666011339	1.7190106	48.727532	0.4872753	2.095080999	2.1898843	1.702609	-	-	-	0.64	35.44	
14	35.48	0.70	0.91	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.002478	57.43778268	11	3.172	2.673522205	1.7073058	48.727532	0.4872753	2.089641691	2.184517	1.6972417	-	-	-	0.70	35.48	
15	35.51	0.75	0.98	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.006863	57.68897971	11	3.172	2.680981683	1.6955681	48.727532	0.4872753	2.084259261	2.1791346	1.6918593	-	-	-	0.75	35.51	
16	35.55	0.81	1.04	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.011247	57.94017674	11	3.172	2.688389628	1.6837978	48.727532	0.4872753	2.078861813	2.1737371	1.6864618	-	-	-	0.81	35.55	
17	35.58	0.86	1.11	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.015631	58.19137376	11	3.172	2.6957459	1.6719952	48.727532	0.4872753	2.073449451	2.1683248	1.6810495	-	-	-	0.86	35.58	
18	35.62	0.92	1.17	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.020015	58.44257079	11	3.172	2.703050355	1.6601605	48.727532	0.4872753	2.06802228	2.1628976	1.6756223	-	-	-	0.92	35.62	
19	35.65	0.97	1.24	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.024399	58.69376782	11	3.172	2.710302855	1.6482938	48.727532	0.4872753	2.062580403	2.1574557	1.6701804	-	-	-	0.97	35.65	
20	35.68	1.03	1.30	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.028784	58.94496485	11	3.172	2.717503258	1.6363954	48.727532	0.4872753	2.057123925	2.1519992	1.6647239	-	-	-	1.03	35.68	
21	35.72	1.09	1.37	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.033168	59.19616188	11	3.172	2.724651428	1.6244656	48.727532	0.4872753	2.051652951	2.1465283	1.655253	-	-	-	1.09	35.72	
22	35.75	1.14	1.43	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.037552	59.44735891	11	3.172	2.731747226	1.6125046	48.727532	0.4872753	2.046167586	2.1410429	1.6537676	-	-	-	1.14	35.75	
23	35.78	1.20	1.50	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.041936	59.69855954	11	3.172	2.738790517	1.6005126	48.727532	0.4872753	2.040679935	2.1355433	1.6482679	-	-	-	1.20	35.78	
24	35.82	1.25	1.56	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.046321	59.94975297	11	3.172	2.745781165	1.5884898	48.727532	0.4872753	2.035154104	2.1300294	1.6427551	-	-	-	1.25	35.82	
25	35.85	1.31	1.63	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.004384	0.251197	1.050705	60.20091359	11	3.172	2.7527219												

128	37.79	7.67	8.33	-	-	-	0.004384	-	Dubbel	-	0.036210	Dubbel	-	0.004384	0.251197	1.502279	86.07424398	11	3.172	3.164719194	0.2171782	48.727532	0.4872753	1.405869324	1.5007446	1.0134693	-	-	-	0.6	-	7.67	37.79	
129	37.80	7.73	8.40	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.506663	-	0.004384	0.251197	1.506663	86.325441	11	3.172	3.165640932	0.2033013	48.727532	0.4872753	1.399497999	1.4943733	1.007098	-	-	-	0.6	-	7.73	37.80
130	37.80	7.80	8.46	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.511047	-	0.004384	0.251197	1.511047	86.57663803	11	3.172	3.166501822	0.1894206	48.727532	0.4872753	1.393124818	1.4880001	1.0007248	-	-	-	0.6	-	7.80	37.80
131	37.80	7.86	8.53	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.515432	-	0.004384	0.251197	1.515432	86.82783506	11	3.172	3.167301848	0.1755362	48.727532	0.4872753	1.386749904	1.4816252	0.9943499	-	-	-	0.6	-	7.86	37.80
132	37.81	7.93	8.59	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.519816	-	0.004384	0.251197	1.519816	87.07903209	11	3.172	3.168040994	0.1616484	48.727532	0.4872753	1.380373379	1.4752487	0.9879734	-	-	-	0.6	-	7.93	37.81
133	37.81	7.99	8.66	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.524200	-	0.004384	0.251197	1.524200	87.33022912	11	3.172	3.168719246	0.1477575	48.727532	0.4872753	1.373995366	1.4688707	0.9815954	-	-	-	0.6	-	7.99	37.81
134	37.81	8.06	8.71	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.528584	-	0.004384	0.251197	1.528584	87.58365461	11	3.172	3.169346584	0.1396386	48.727532	0.4872753	1.367615988	1.4524691	0.9722166	-	-	-	0.6	-	8.06	37.81
135	37.82	8.12	8.79	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.532968	-	0.004384	0.251197	1.532968	87.83262318	11	3.172	3.169893018	0.1199675	48.727532	0.4872753	1.361235367	1.4561107	0.9688354	-	-	-	0.6	-	8.12	37.82
136	37.82	8.19	8.85	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.537353	-	0.004384	0.251197	1.537353	88.08382021	11	3.172	3.170388515	0.1060689	48.727532	0.4872753	1.354853626	1.4497289	0.9624534	-	-	-	0.6	-	8.19	37.82
137	37.82	8.25	8.92	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.541737	-	0.004384	0.251197	1.541737	88.33501724	11	3.172	3.170823073	0.0921683	48.727532	0.4872753	1.348470887	1.4433462	0.9560709	-	-	-	0.6	-	8.25	37.82
138	37.82	8.32	8.98	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.546121	-	0.004384	0.251197	1.546121	88.58621426	11	3.172	3.171196683	0.0782658	48.727532	0.4872753	1.342087273	1.4369626	0.9496873	-	-	-	0.6	-	8.32	37.82
139	37.83	8.38	9.05	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.550505	-	0.004384	0.251197	1.550505	88.83741129	11	3.172	3.171509339	0.0643619	48.727532	0.4872753	1.335702907	1.4305782	0.9433029	-	-	-	0.6	-	8.38	37.83
140	37.83	9.11	-	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.554890	-	0.004384	0.251197	1.554890	89.08860832	11	3.172	3.171761035	0.0504568	48.727532	0.4872753	1.329317912	1.4241932	0.9369179	-	-	-	0.6	-	8.45	37.83
141	37.83	8.51	9.18	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.559274	-	0.004384	0.251197	1.559274	89.33980535	11	3.172	3.171951764	0.0365507	48.727532	0.4872753	1.32293241	1.4178077	0.9305324	-	-	-	0.6	-	8.51	37.83
142	37.83	8.58	9.24	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.563653	-	0.004384	0.251197	1.563653	89.59102392	11	3.172	3.172081525	0.0226438	48.727532	0.4872753	1.316546524	1.4114218	0.9241465	-	-	-	0.6	-	8.58	37.83
143	37.83	8.64	9.31	-	-	-	0.004384	-	Dubbel	-	0.004384	0.251197	1.568042	-	0.004384	0.251197	1.568042	89.84211941	11	3.172	3.172150315	0.0087366	48.727532	0.4872753	1.310160376	1.4050357	0.9177604	-	-	-	0.6	-	8.64	37.83
144	37.83	8.71	9.37	-	-	-	0.004384	-	-	-	0.000927714	-	0	-	0.004384	0.251197	1.572426	90.09339644	11	3.172	3.172158131	-0.005171	48.727532	0.4872753	1.303774091	1.3986494	0.9113741	-	-	-	0.6	-	8.71	37.83
145	37.83	8.77	9.44	-	-	-	0.004384	-	-	-	0.003448341	-	0	-	0.004384	0.251197	1.576811	90.34459347	11	3.172	3.172104974	-0.019078	48.727532	0.4872753	1.297387789	1.3922631	0.9049878	-	-	-	0.6	-	8.77	37.83
146	37.83	8.84	9.50	-	-	-	0.004384	-	-	-	0.006060606	-	0	-	0.004384	0.251197	1.581195	90.5957905	11	3.172	3.171990846	-0.032985	48.727532	0.4872753	1.291001595	1.3858769	0.8986016	-	-	-	0.6	-	8.84	37.83
147	37.83	8.91	9.57	-	-	-	0.004384	-	-	-	0.008600818	-	0	-	0.004384	0.251197	1.585579	90.84698753	11	3.172	3.171815747	-0.046891	48.727532	0.4872753	1.28461563	1.379491	0.8922156	-	-	-	0.6	-	8.91	37.83
148	37.83	8.97	9.63	-	-	-	0.004384	-	-	-	0.01123256	-	0	-	0.004384	0.251197	1.589963	91.09818455	11	3.172	3.171579682	-0.060797	48.727532	0.4872753	1.278230018	1.3731053	0.885803	-	-	-	0.6	-	8.97	37.83
149	37.83	9.04	9.70	-	-	-	0.004384	-	-	-	0.013902125	-	0	-	0.004384	0.251197	1.594347	91.34938158	11	3.172	3.171282656	-0.074701	48.727532	0.4872753	1.271844881	1.3687202	0.8794446	-	-	-	0.7	-	9.04	37.83
150	37.82	9.10	9.77	-	-	-	0.004384	-	-	-	0.016606752	-	0	-	0.004384	0.251197	1.598732	91.60057861	11	3.172	3.170924673	-0.088604	48.727532	0.4872753	1.265460342	1.3603357	0.8730603	-	-	-	0.7	-	9.10	37.82
151	37.82	9.17	9.83	-	-	-	0.004384	-	-	-	0.019349078	-	0	-	0.004384	0.251197	1.603116	91.85177564	11	3.172	3.17050574	-0.102055	48.727532	0.4872753	1.259076523	1.3539518	0.8666765	-	-	-	0.7	-	9.17	37.82
152	37.82	9.23	9.90	-	-	-	0.004384	-	-	-	0.022128133	-	0	-	0.004384	0.251197	1.607500	92.10297267	11	3.172	3.170025866	-0.116404	48.727532	0.4872753	1.252693549	1.3475689	0.8602935	-	-	-	0.7	-	9.23	37.82
153	37.82	9.30	9.96	-	-	-	0.004384	-	-	-	0.024943849	-	0	-	0.004384	0.251197	1.611884	92.3541697	11	3.172	3.169485061	-0.130301	48.727532	0.4872753	1.24631154	1.3411869	0.8539115	-	-	-	0.7	-	9.30	37.82
154	37.81	9.36	10.03	-	-	-	0.004384	-	-	-	0.027796151	-	0	-	0.004384	0.251197	1.616269	92.60536673	11	3.172	3.168883333	-0.144196	48.727532	0.4872753	1.23993062	1.3348059	0.8475306	-	-	-	0.7	-	9.36	37.81
155	37.81	9.43	10.09	-	-	-	0.004384	-	-	-	0.030684964	-	0	-	0.004384	0.251197	1.620653	92.85656376	11	3.172	3.168220696	-0.158087	48.727532	0.4872753	1.233550911	1.3284262	0.8411509	-	-	-	0.7	-	9.43	37.81
156	37.81	9.49	10.16	-	-	-	0.004384	-	-	-	0.03361021	-	0	-	0.004384	0.251197	1.625037	93.10776079	11	3.172	3.167487161	-0.1747976	48.727532	0.4872753	1.227172537	1.3220479	0.8347259	-	-	-	0.7	-	9.49	37.81
157	37.81	9.56	10.24	-	-	-	0.004384	-	-	-	0.036571806	-	0	-	0.004384	0.251197	1.62957238	93.36671273	11	3.172	3.166712743	-0.1935861	48.727532	0.4872753	1.216795561	1.3165769	0.8283956	-	-	-	0.7	-	9.56	37.81
158	37.80	9.62	10.29	-	-	-	0.004384	-	-	-	0.039569668	-	0	-	0.004384	0.251197	1.633805	93.61015484	11	3.172	3.165867456	-0.199743	48.727532	0.4872753	1.21442028	1.3092956	0.8220203	-	-	-	0.7	-	9.62	37.80
159	37.80	9.69	10.35	-	-	-	0.004384	-	-	-	0.042603709	-	0	-	0.004384	0.251197	1.638190	93.86135187	11	3.172	3.164961317	-0.213621	48.727532	0.4872753	1.208046643	1.302922	0.8156466	-	-	-	0.7	-	9.69	37.80
160	37.79	9.75	10.42	-	-	-	0.004384	-	-	-	0.045673838	-	0	-	0.004384	0.251197	1.642574	94.1125489	11	3.172	3.163994344	-0.227494	48.727532	0.4872753	1.201674831	1.2965502	0.8092748	-	-	-	0.7	-	9.75	37.79
161	37.79	9.82	10.48	-	-	-	0.004384	-	-	-	0.048779964	-	0	-	0.004384	0.251197	1.646958	94.36374593	11	3.172	3.162966554	-0.241364	48.727532	0.4872753	1.195304965	1.2901803	0.802905	-	-	-	0.7	-	9.82	37.79
162	37.78	9.88	10.55	-	-	-	0.004384	-	-	-	0.051921989	-	0	-	0.004384	0.251197	1.651342	94.61494296	11	3.172	3.161877968	-0.255229	48.727532	0.4872753	1.188937168	1.2838125	0.7965372	-	-	-	0.7	-	9.88	37.78
163	37.78	9.95	10.61	-	-	-	0.004384	-	-	-	0.055099816	-	0	-	0.004384	0.251197	1.655726	94.86613999	11	3.172	3.160728607	-0.269089	48.727532	0.4872753	1.182571563	1.2774469	0.7901718	-	-	-	0.7	-	9.95	37.78
164	37.77	10.01	10.68	-	-	-	0.004384	-	-	-																								

260	35.95	15.94	16.93	-	-	0.004384	-	-	0.507695634	0	-	0.004384	0.251197	2.080995	119.2322518	11	3.172	2.768178978	-1.549128	48.727532	0.4872753	0.594381849	0.6892572	0.2019818	-	-	-	-	-	15.94	35.95
261	35.92	16.00	16.99	-	-	0.004384	0.251197	2.085380	119.4834488	0	-	0.004384	0.251197	2.085380	119.4834488	11	3.172	2.761306083	-1.56125	48.727532	0.4872753	0.588808853	0.6836842	0.1964089	-	-	-	-	-	16.00	35.92
262	35.89	16.05	17.06	-	-	0.004384	-	2.089764	119.7346459	0	-	0.004384	0.251197	2.089764	119.7346459	11	3.172	2.754489311	-1.573341	48.727532	0.4872753	0.583249584	0.6781249	0.1908496	-	-	-	-	-	16.05	35.89
263	35.86	16.11	17.12	-	-	0.004384	-	2.094148	119.9858429	0	-	0.004384	0.251197	2.094148	119.9858429	11	3.172	2.747564994	-1.585402	48.727532	0.4872753	0.577704148	0.6725795	0.1853041	-	-	-	-	-	16.11	35.86
264	35.82	16.17	17.19	-	-	0.004384	-	2.098532	120.2370399	0	-	0.004384	0.251197	2.098532	120.2370399	11	3.172	2.740587866	-1.597433	48.727532	0.4872753	0.572127652	0.667048	0.1797727	-	-	-	-	-	16.17	35.82
265	35.79	16.22	17.25	-	-	0.004384	-	2.102916	120.4882369	0	-	0.004384	0.251197	2.102916	120.4882369	11	3.172	2.733558059	-1.609439	48.727532	0.4872753	0.566652023	0.6615305	0.1742552	-	-	-	-	-	16.22	35.79
266	35.76	16.28	17.30	-	-	0.004384	-	2.107301	120.7409942	0	-	0.004384	0.251197	2.107301	120.7409942	11	3.172	2.726475734	-1.621402	48.727532	0.4872753	0.561151907	0.6560272	0.1705186	-	-	-	-	-	16.28	35.76
267	35.73	16.34	17.38	-	-	0.004384	-	2.111685	120.990631	0	-	0.004384	0.251197	2.111685	120.990631	11	3.172	2.719340955	-1.63334	48.727532	0.4872753	0.555662869	0.6505382	0.1632629	-	-	-	-	-	16.34	35.73
268	35.69	16.39	17.45	-	-	0.004384	-	2.116069	121.241828	0	-	0.004384	0.251197	2.116069	121.241828	11	3.172	2.71215393	-1.645246	48.727532	0.4872753	0.550188195	0.6450635	0.1577882	-	-	-	-	-	16.39	35.69
269	35.66	16.45	17.51	-	-	0.004384	-	2.120453	121.4930251	0	-	0.004384	0.251197	2.120453	121.4930251	11	3.172	2.704914775	-1.657121	48.727532	0.4872753	0.544727991	0.6396033	0.152328	-	-	-	-	-	16.45	35.66
270	35.62	16.50	17.58	-	-	0.004384	-	2.124838	121.7442221	0	-	0.004384	0.251197	2.124838	121.7442221	11	3.172	2.697623627	-1.668964	48.727532	0.4872753	0.53928236	0.6341577	0.1468824	-	-	-	-	-	16.50	35.62
271	35.59	16.56	17.64	-	-	0.004384	-	2.129222	121.9954191	0	-	0.004384	0.251197	2.129222	121.9954191	11	3.172	2.690280627	-1.680775	48.727532	0.4872753	0.533851408	0.6287267	0.1414514	-	-	-	-	-	16.56	35.59
272	35.56	16.61	17.71	-	-	0.004384	-	2.133606	122.2466161	0	-	0.004384	0.251197	2.133606	122.2466161	11	3.172	2.682885917	-1.692553	48.727532	0.4872753	0.52843524	0.6233106	0.1360352	-	-	-	-	-	16.61	35.56
273	35.52	16.67	17.77	-	-	0.004384	-	2.137990	122.4978132	0	-	0.004384	0.251197	2.137990	122.4978132	11	3.172	2.67549638	-1.704299	48.727532	0.4872753	0.52303958	0.6179093	0.130634	-	-	-	-	-	16.67	35.52
274	35.49	16.72	17.84	-	-	0.004384	-	2.142374	122.7490102	0	-	0.004384	0.251197	2.142374	122.7490102	11	3.172	2.667941933	-1.716191	48.727532	0.4872753	0.517647668	0.612523	0.125477	-	-	-	-	-	16.72	35.49
275	35.45	16.78	17.90	-	-	0.004384	-	2.146759	123.0020072	0	-	0.004384	0.251197	2.146759	123.0020072	11	3.172	2.660392947	-1.727693	48.727532	0.4872753	0.512276473	0.6071518	0.1198765	-	-	-	-	-	16.78	35.45
276	35.42	16.83	17.97	-	-	0.004384	-	2.151143	123.2514043	0	-	0.004384	0.251197	2.151143	123.2514043	11	3.172	2.652792825	-1.73934	48.727532	0.4872753	0.506920475	0.6017958	0.1145205	-	-	-	-	-	16.83	35.42
277	35.38	16.89	18.03	-	-	0.004384	-	2.155527	123.5026013	0	-	0.004384	0.251197	2.155527	123.5026013	11	3.172	2.645141713	-1.750954	48.727532	0.4872753	0.501579778	0.5964551	0.1091798	-	-	-	-	-	16.89	35.38
278	35.34	16.94	18.10	-	-	0.004384	-	2.159911	123.7537983	0	-	0.004384	0.251197	2.159911	123.7537983	11	3.172	2.637439758	-1.762534	48.727532	0.4872753	0.496254485	0.5911298	0.1038545	-	-	-	-	-	16.94	35.34
279	35.31	17.00	18.16	-	-	0.004384	-	2.164295	124.0049953	0	-	0.004384	0.251197	2.164295	124.0049953	11	3.172	2.629687107	-1.77408	48.727532	0.4872753	0.490944697	0.58582	0.0985447	-	-	-	-	-	17.00	35.31
280	35.27	17.05	18.23	-	-	0.004452	-	2.168680	124.2561924	0	-	0.004452	0.255058	2.168680	124.2561924	11	3.204	2.647977535	-1.803363	49.21248	0.4921248	0.485650518	0.5853753	0.0932505	-	-	-	-	-	17.05	35.27
281	35.23	17.10	18.29	-	-	0.004499	-	2.173131	124.5112469	0	-	0.004499	0.257775	2.173131	124.5112469	11	3.238	2.668238979	-1.834601	49.740327	0.4974033	0.480372048	0.5853753	0.087972	-	-	-	-	-	17.10	35.23
282	35.20	17.16	18.36	-	-	0.004546	-	2.177630	124.7608518	0	-	0.004546	0.260483	2.177630	124.7608518	11	3.272	2.688089865	-1.866124	50.265657	0.5026567	0.475109633	0.5853753	0.0827094	-	-	-	-	-	17.16	35.20
283	35.16	17.21	18.42	-	-	0.004593	-	2.182177	125.0295077	0	-	0.004593	0.263183	2.182177	125.0295077	11	3.307	2.707553766	-1.897928	50.791177	0.5079118	0.46986355	0.5853753	0.0774635	-	-	-	-	-	17.21	35.16
284	35.12	17.26	18.49	-	-	0.004640	-	2.186770	125.292691	0	-	0.004640	0.265875	2.186770	125.292691	11	3.341	2.726594242	-1.930014	51.314124	0.5131412	0.464634078	0.5853753	0.0722341	-	-	-	-	-	17.26	35.12
285	35.09	17.32	18.55	-	-	0.004687	-	2.191410	125.5585658	0	-	0.004687	0.268558	2.191410	125.5585658	11	3.374	2.745214843	-1.96238	51.835382	0.5183538	0.4594215	0.5853753	0.0670215	-	-	-	-	-	17.32	35.09
286	35.05	17.37	18.62	-	-	0.004734	-	2.196098	125.8271234	0	-	0.004734	0.271231	2.196098	125.8271234	11	3.408	2.763409107	-1.995023	52.354922	0.5235492	0.454226099	0.5853753	0.0618261	-	-	-	-	-	17.37	35.05
287	35.01	17.42	18.68	-	-	0.004780	-	2.200831	126.0983548	0	-	0.004780	0.273869	2.200831	126.0983548	11	3.442	2.781170566	-2.027942	52.872716	0.5287272	0.449048163	0.5853753	0.0566482	-	-	-	-	-	17.42	35.01
288	34.97	17.48	18.75	-	-	0.004827	-	2.205612	126.3722509	0	-	0.004827	0.276552	2.205612	126.3722509	11	3.476	2.798692749	-2.061335	53.388734	0.5338873	0.443879891	0.5853753	0.0521488	-	-	-	-	-	17.48	34.97
289	34.93	17.53	18.83	-	-	0.004873	-	2.210495	126.6488053	0	-	0.004873	0.279196	2.210495	126.6488053	11	3.509	2.814096122	-2.09461	53.902853	0.5390285	0.438745463	0.5853753	0.0482412	-	-	-	-	-	17.53	34.93
290	34.89	17.58	18.88	-	-	0.004919	-	2.215312	126.9280001	0	-	0.004919	0.281834	2.215312	126.9280001	11	3.542	2.8317934	-2.128335	54.415327	0.5441533	0.433622049	0.5853753	0.0441222	-	-	-	-	-	17.58	34.89
291	34.85	17.63	18.94	-	-	0.004965	-	2.220230	127.2098344	0	-	0.004965	0.284461	2.220230	127.2098344	11	3.576	2.847758933	-2.162337	54.925843	0.5492584	0.428516889	0.5853753	0.0396116	-	-	-	-	-	17.63	34.85
292	34.82	17.68	19.01	-	-	0.005010	-	2.225195	127.4942956	0	-	0.005010	0.287078	2.225195	127.4942956	11	3.609	2.863259325	-2.196603	55.434466	0.5544467	0.423430665	0.5853753	0.0310307	-	-	-	-	-	17.68	34.82
293	34.78	17.74	19.07	-	-	0.005056	-	2.230206	127.7813738	0	-	0.005056	0.289685	2.230206	127.7813738	11	3.642	2.878288131	-2.231131	55.941164	0.5594116	0.418363678	0.5853753	0.0259637	-	-	-	-	-	17.74	34.78
294	34.74	17.79	19.14	-	-	0.005101	-	2.235262	128.0710591	0	-	0.005101	0.292282	2.235262	128.0710591	11	3.675	2.892838919	-2.265917	56.449909	0.5644991	0.413316231	0.5853753	0.0209162	-	-	-	-	-	17.79	34.74
295	34.70	17.84	19.20	-	-	0.005146	-	2.240363	128.3633413	0	-	0.005146	0.294869	2.240363	128.3633413	11	3.707	2.906905277	-2.299587	56.948669	0.5694867	0.40828863	0.5853753	0.0158888	-	-	-	-	-	17.84	34.70
296	34.66	17.89	19.27	-	-	0.005191	-	2.245509	128.6592101	0	-	0.005191	0.297445	2.245509	128.6592101	11	3.740	2.920480812	-2.336251	57.449414	0.5744941	0.403281184	0.5853753	0.0108812	-	-	-	-	-	17.89	34.66
297	34.61	17.94	19.33	-	-	0.005236	-	2.250701	128.9556555	0	-	0.005236	0.30001	2.250701	128.9556555	11	3.772	2.933559157	-2.371792	57.948112	0.5794811	0.398294202									

392	29.72	21.61	25.52	-	-	-	0.005267	-	-	1.023362388	-	0.278304042	-	0.005267	0.301769	2.751036	157.622739	11	3.811	1.450782062	-3.52383	58.537532	0.5853753	0.038124268	0.5853753	-0.354277	-	-	0.5	1.2	0.092538	-	-	21.61	29.72
393	29.66	21.64	25.58	-	-	-	0.005267	-	-	1.025567241	-	0.285567241	-	0.005267	0.301769	2.756303	157.924508	11	3.811	1.432202503	-3.531422	58.537532	0.5853753	0.035692978	0.5853753	-0.354277	-	-	0.5	1.2	0.092538	-	-	21.64	29.66
394	29.60	21.66	25.65	-	-	-	0.005267	-	-	1.027743645	-	0.28249032	-	0.005267	0.301769	2.761569	158.2262769	11	3.811	1.413583215	-3.538916	58.537532	0.5853753	0.033292824	0.5853753	-0.359107	-	-	0.5	1.2	0.092538	-	-	21.66	29.60
395	29.54	21.68	25.71	-	-	-	0.005267	-	-	1.02989154	-	0.284533995	-	0.005267	0.301769	2.766836	158.5280458	11	3.811	1.394924715	-3.546312	58.537532	0.5853753	0.030923874	0.5853753	-0.361476	-	-	0.5	1.2	0.092538	-	-	21.68	29.54
396	29.48	21.71	25.78	-	-	-	0.005267	-	-	1.032010866	-	0.286555355	-	0.005267	0.301769	2.772103	158.8298147	11	3.811	1.37622752	-3.55361	58.537532	0.5853753	0.028586192	0.5853753	-0.363814	-	-	0.5	1.2	0.092538	-	-	21.71	29.48
397	29.42	21.73	25.84	-	-	-	0.005267	-	-	1.034101564	-	0.288557327	-	0.005267	0.301769	2.777370	159.13115836	11	3.811	1.357492149	-3.560809	58.537532	0.5853753	0.026279844	0.5853753	-0.36612	-	-	0.5	1.2	0.092538	-	-	21.73	29.42
398	29.35	21.75	25.91	-	-	-	0.005267	-	-	1.036135377	-	0.290535357	-	0.005267	0.301769	2.782637	159.43933525	11	3.811	1.338719121	-3.569703	58.537532	0.5853753	0.024040498	0.5853753	-0.368395	-	-	0.5	1.2	0.092538	-	-	21.75	29.35
399	29.29	21.78	25.97	-	-	-	0.005267	-	-	1.038196846	-	0.329493825	-	0.005267	0.301769	2.787904	159.7351214	11	3.811	1.319908057	-3.574911	58.537532	0.5853753	0.021761404	0.5853753	-0.370639	-	-	0.5	1.2	0.092538	-	-	21.78	29.29
400	29.23	21.80	26.04	-	-	-	0.005267	-	-	1.040201317	-	0.294427903	-	0.005267	0.301769	2.793171	160.0368903	11	3.811	1.30106218	-3.581813	58.537532	0.5853753	0.019549437	0.5853753	-0.372851	-	-	0.5	1.2	0.092538	-	-	21.80	29.23
401	29.17	21.82	26.11	-	-	-	0.005267	-	-	1.042176932	-	0.296339773	-	0.005267	0.301769	2.798438	160.3386592	11	3.811	1.282179311	-3.588616	58.537532	0.5853753	0.017369055	0.5853753	-0.375031	-	-	0.5	1.2	0.092538	-	-	21.82	29.17
402	29.11	21.84	26.17	-	-	-	0.005267	-	-	1.044123638	-	0.29822638	-	0.005267	0.301769	2.803704	160.6404281	11	3.811	1.263260875	-3.595319	58.537532	0.5853753	0.015220318	0.5853753	-0.37718	-	-	0.5	1.2	0.092538	-	-	21.84	29.11
403	29.05	21.87	26.24	-	-	-	0.005267	-	-	1.046041379	-	0.300094356	-	0.005267	0.301769	2.808971	160.942197	11	3.811	1.244307396	-3.601922	58.537532	0.5853753	0.013103285	0.5853753	-0.379297	-	-	0.5	1.2	0.092538	-	-	21.87	29.05
404	28.99	21.89	26.30	-	-	-	0.005267	-	-	1.047930104	-	0.301936525	-	0.005267	0.301769	2.814238	161.2439659	11	3.811	1.225319401	-3.608426	58.537532	0.5853753	0.011018015	0.5853753	-0.381382	-	-	0.6	1.2	0.092538	-	-	21.89	28.99
405	28.93	21.91	26.37	-	-	-	0.005267	-	-	1.049785792	-	0.303754592	-	0.005267	0.301769	2.819505	161.54537348	11	3.811	1.206297415	-3.614893	58.537532	0.5853753	0.008964566	0.5853753	-0.383435	-	-	0.6	1.2	0.092538	-	-	21.91	28.93
406	28.86	21.93	26.43	-	-	-	0.005267	-	-	1.051620295	-	0.305549278	-	0.005267	0.301769	2.824772	161.8475038	11	3.811	1.187419567	-3.621133	58.537532	0.5853753	0.006942995	0.5853753	-0.385453	-	-	0.6	1.2	0.092538	-	-	21.93	28.86
407	28.80	21.95	26.50	-	-	-	0.005267	-	-	1.053421658	-	0.307311934	-	0.005267	0.301769	2.830039	162.1492727	11	3.811	1.168153586	-3.627336	58.537532	0.5853753	0.004953358	0.5853753	-0.387447	-	-	0.6	1.2	0.092538	-	-	21.95	28.80
408	28.74	21.97	26.56	-	-	-	0.005267	-	-	1.055193799	-	0.309064848	-	0.005267	0.301769	2.835306	162.4510416	11	3.811	1.1490328	-3.633438	58.537532	0.5853753	0.00299571	0.5853753	-0.389404	-	-	0.6	1.2	0.092538	-	-	21.97	28.74
409	28.68	21.99	26.63	-	-	-	0.005267	-	-	1.056936669	-	0.310785549	-	0.005267	0.301769	2.840572	162.7528105	11	3.811	1.12988014	-3.639439	58.537532	0.5853753	0.001070106	0.5853753	-0.39133	-	-	0.6	1.2	0.092538	-	-	21.99	28.68
410	28.62	22.01	26.69	-	-	-	0.005267	-	-	1.058650221	-	0.312481196	-	0.005267	0.301769	2.845839	163.0545794	11	3.811	1.110696137	-3.645339	58.537532	0.5853753	-0.000823401	0.5853753	-0.393223	-	-	0.6	1.2	0.092538	-	-	22.01	28.62
411	28.55	22.03	26.76	-	-	-	0.005267	-	-	1.060334405	-	0.314151542	-	0.005267	0.301769	2.851106	163.3563483	11	3.811	1.091481324	-3.651139	58.537532	0.5853753	-0.002684759	0.5853753	-0.395085	-	-	0.6	1.2	0.092538	-	-	22.03	28.55
412	28.49	22.05	26.82	-	-	-	0.005267	-	-	1.061989176	-	0.315796347	-	0.005267	0.301769	2.856373	163.6581172	11	3.811	1.072236234	-3.656837	58.537532	0.5853753	-0.004513916	0.5853753	-0.396914	-	-	0.6	1.2	0.092538	-	-	22.05	28.49
413	28.43	22.06	26.89	-	-	-	0.005267	-	-	1.063614488	-	0.317415372	-	0.005267	0.301769	2.861640	163.9598861	11	3.811	1.0529614	-3.662433	58.537532	0.5853753	-0.006310221	0.5853753	-0.398711	-	-	0.6	1.2	0.092538	-	-	22.06	28.43
414	28.37	22.08	26.95	-	-	-	0.005267	-	-	1.065210296	-	0.319008384	-	0.005267	0.301769	2.866907	164.261655	11	3.811	1.033637357	-3.667928	58.537532	0.5853753	-0.008075424	0.5853753	-0.400475	-	-	0.6	1.2	0.092538	-	-	22.08	28.37
415	28.30	22.10	27.02	-	-	-	0.005267	-	-	1.066776554	-	0.320575152	-	0.005267	0.301769	2.872174	164.5634239	11	3.811	1.014324641	-3.673322	58.537532	0.5853753	-0.009807676	0.5853753	-0.402208	-	-	0.6	1.2	0.092538	-	-	22.10	28.30
416	28.24	22.12	27.08	-	-	-	0.005267	-	-	1.068313221	-	0.322115452	-	0.005267	0.301769	2.877440	164.8651928	11	3.811	0.994963788	-3.678613	58.537532	0.5853753	-0.01150753	0.5853753	-0.403908	-	-	0.6	1.2	0.092538	-	-	22.12	28.24
417	28.18	22.13	27.15	-	-	-	0.005267	-	-	1.069820253	-	0.323262906	-	0.005267	0.301769	2.882707	165.1669617	11	3.811	0.975575334	-3.683802	58.537532	0.5853753	-0.013174938	0.5853753	-0.405575	-	-	0.6	1.2	0.092538	-	-	22.13	28.18
418	28.12	22.15	27.21	-	-	-	0.005267	-	-	1.071297608	-	0.325115758	-	0.005267	0.301769	2.887974	165.4687306	11	3.811	0.956159818	-3.688889	58.537532	0.5853753	-0.014809854	0.5853753	-0.40721	-	-	0.6	1.2	0.092538	-	-	22.15	28.12
419	28.05	22.17	27.28	-	-	-	0.005267	-	-	1.072745246	-	0.326575333	-	0.005267	0.301769	2.893241	165.7704996	11	3.811	0.936717779	-3.693874	58.537532	0.5853753	-0.016412232	0.5853753	-0.408812	-	-	0.6	1.2	0.092538	-	-	22.17	28.05
420	27.99	22.20	27.34	-	-	-	0.005267	-	-	1.074163126	-	0.328007575	-	0.005267	0.301769	2.898508	166.0722685	11	3.811	0.917249756	-3.698756	58.537532	0.5853753	-0.017982029	0.5853753	-0.410382	-	-	0.6	1.2	0.092538	-	-	22.19	27.99
421	27.93	22.22	27.41	-	-	-	0.005267	-	-	1.075913028	-	0.329417467	-	0.005267	0.301769	2.903775	166.3753436	11	3.811	0.897756203	-3.703546	58.537532	0.5853753	-0.019525926	0.5853753	-0.411916	-	-	0.6	1.2	0.092538	-	-	22.21	27.93
422	27.86	22.21	27.47	-	-	-	0.005267	-	-	1.076909457	-	0.330789235	-	0.005267	0.301769	2.909042	166.6758063	11	3.811	0.878237916	-3.708213	58.537532	0.5853753	-0.021032703	0.5853753	-0.413424	-	-	0.6	1.2	0.092538	-	-	22.21	27.86
423	27.80	22.23	27.54	-	-	-	0.005267	-	-	1.078237831	-	0.332128254	-	0.005267	0.301769	2.914308	166.9775752	11	3.811	0.858695183	-3.712787	58.537532	0.5853753	-0.022495496	0.5853753	-0.414895	-	-	0.7	1.2	0.092538	-	-	22.23	27.80
424	27.74	22.24	27.60	-	-	-	0.005267	-	-	1.079536295	-	0.333459139	-	0.005267	0.301769	2.919575	167.2793441	11	3.811	0.839128629	-3.717258	58.537532	0.5853753	-0.023934539	0.5853753	-0.416335	-	-	0.7	1.2	0.092538	-	-	22.24	27.74
425	27.67	22.26	27.67	-	-	-	0.005267	-	-	1.080804813	-	0.334755197	-	0.005267	0.301769	2.924842	167.581113	11	3.811	0.819538798	-3.721626	58.537532	0.5853753	-0.025340791	0.5853753	-0.417741	-	-	0.7	1.2	0.092538	-	-	22.26	27.67
426	27.61	22.27	27.7																																

524	21.31	22.00	34.11	-	-	0.005267	-	-	1.055732442	-	0.309592314	-	0.005267	0.301769	3.446261	197.4562349	11	3.811	-1.143151191	-3.6352921	58.537532	0.5853753	0.000484876	0.5853753	-0.391915	-	-	1.2	1.2	0.092538	-	22.00	21.31
525	21.24	21.98	34.18	-	-	0.005267	-	-	1.05396929	-	0.307858601	-	0.005267	0.301769	3.451528	197.7580038	11	3.811	-1.162281828	-3.629221	58.537532	0.5853753	0.002400623	0.5853753	-0.389999	-	-	1.2	1.2	0.092538	-	21.98	21.24
526	21.18	21.96	34.24	-	-	0.005267	-	-	1.052176901	-	0.306095757	-	0.005267	0.301769	3.456795	198.0597727	11	3.811	-1.181380222	-3.623049	58.537532	0.5853753	0.004348431	0.5853753	-0.388052	-	-	1.2	1.2	0.092538	-	21.96	21.18
527	21.12	21.94	34.31	-	-	0.005267	-	-	1.050355324	-	0.304308837	-	0.005267	0.301769	3.462062	198.3615416	11	3.811	-1.200445846	-3.616777	58.537532	0.5853753	0.006328244	0.5853753	-0.386072	-	-	1.2	1.2	0.092538	-	21.94	21.12
528	21.06	21.91	34.37	-	-	0.005267	-	-	1.048504611	-	0.3024978	-	0.005267	0.301769	3.467329	198.6633105	11	3.811	-1.21947817	-3.610404	58.537532	0.5853753	0.008340090	0.5853753	-0.384060	-	-	1.2	1.2	0.092538	-	21.91	21.06
529	21.00	21.89	34.44	-	-	0.005267	-	-	1.046624812	-	0.300662908	-	0.005267	0.301769	3.472596	198.9650794	11	3.811	-1.238476665	-3.603931	58.537532	0.5853753	0.010383669	0.5853753	-0.382016	-	-	1.2	1.2	0.092538	-	21.89	21.00
530	20.93	21.87	34.51	-	-	0.005267	-	-	1.044715981	-	0.298801427	-	0.005267	0.301769	3.477863	199.2744086	11	3.811	-1.257440863	-3.597358	58.537532	0.5853753	0.012451497	0.5853753	-0.379187	-	-	1.2	1.2	0.092538	-	21.87	20.93
531	20.87	21.85	34.57	-	-	0.005267	-	-	1.042778169	-	0.296922628	-	0.005267	0.301769	3.483129	199.5686172	11	3.811	-1.276370065	-3.590866	58.537532	0.5853753	0.014566447	0.5853753	-0.377834	-	-	1.2	1.2	0.092538	-	21.85	20.87
532	20.81	21.83	34.63	-	-	0.005267	-	-	1.040811431	-	0.295017284	-	0.005267	0.301769	3.488396	199.8703861	11	3.811	-1.295263918	-3.583914	58.537532	0.5853753	0.016705449	0.5853753	-0.375695	-	-	1.2	1.2	0.092538	-	21.83	20.81
533	20.75	21.81	34.70	-	-	0.005267	-	-	1.038815821	-	0.293090171	-	0.005267	0.301769	3.493663	200.172155	11	3.811	-1.314121841	-3.577042	58.537532	0.5853753	0.018876114	0.5853753	-0.373524	-	-	1.2	1.2	0.092538	-	21.81	20.75
534	20.69	21.79	34.76	-	-	0.005267	-	-	1.036791395	-	0.291140071	-	0.005267	0.301769	3.498930	200.4739239	11	3.811	-1.332943311	-3.570071	58.537532	0.5853753	0.021078383	0.5853753	-0.371322	-	-	1.2	1.2	0.092538	-	21.79	20.69
535	20.63	21.76	34.83	-	-	0.005267	-	-	1.034738208	-	0.289167766	-	0.005267	0.301769	3.504197	200.7756929	11	3.811	-1.351727804	-3.563001	58.537532	0.5853753	0.023312193	0.5853753	-0.369088	-	-	1.2	1.2	0.092538	-	21.76	20.63
536	20.57	21.74	34.89	-	-	0.005267	-	-	1.032656318	-	0.287173544	-	0.005267	0.301769	3.509464	201.0774618	11	3.811	-1.370474802	-3.555832	58.537532	0.5853753	0.025577483	0.5853753	-0.366823	-	-	1.2	1.2	0.092538	-	21.74	20.57
537	20.51	21.72	34.96	-	-	0.005267	-	-	1.030545782	-	0.285157696	-	0.005267	0.301769	3.514731	201.3792307	11	3.811	-1.389183782	-3.548565	58.537532	0.5853753	0.027874149	0.5853753	-0.364526	-	-	1.3	1.2	0.092538	-	21.72	20.51
538	20.45	21.69	35.02	-	-	0.005267	-	-	1.028406659	-	0.283120516	-	0.005267	0.301769	3.519991	201.6809996	11	3.811	-1.407854227	-3.541199	58.537532	0.5853753	0.030202251	0.5853753	-0.362028	-	-	1.3	1.2	0.092538	-	21.69	20.45
539	20.39	21.67	35.09	-	-	0.005267	-	-	1.026239008	-	0.281062301	-	0.005267	0.301769	3.525264	201.9827685	11	3.811	-1.426485619	-3.533735	58.537532	0.5853753	0.032561601	0.5853753	-0.359838	-	-	1.3	1.2	0.092538	-	21.67	20.39
540	20.33	21.64	35.15	-	-	0.005267	-	-	1.024042389	-	0.278983351	-	0.005267	0.301769	3.530531	202.2845374	11	3.811	-1.445077744	-3.526173	58.537532	0.5853753	0.034952173	0.5853753	-0.357448	-	-	1.3	1.2	0.092538	-	21.64	20.33
541	20.27	21.62	35.22	-	-	0.005267	-	-	1.021818365	-	0.276883971	-	0.005267	0.301769	3.535798	202.5863063	11	3.811	-1.463629175	-3.518513	58.537532	0.5853753	0.037373903	0.5853753	-0.355026	-	-	1.3	1.2	0.092538	-	21.62	20.27
542	20.20	21.59	35.28	-	-	0.005267	-	-	1.019565495	-	0.274764468	-	0.005267	0.301769	3.541065	202.8880752	11	3.811	-1.482140309	-3.510756	58.537532	0.5853753	0.039826723	0.5853753	-0.352573	-	-	1.3	1.2	0.092538	-	21.59	20.20
543	20.14	21.57	35.35	-	-	0.005267	-	-	1.017284342	-	0.272625151	-	0.005267	0.301769	3.546332	203.1898441	11	3.811	-1.50061033	-3.502091	58.537532	0.5853753	0.042310565	0.5853753	-0.350089	-	-	1.3	1.2	0.092538	-	21.57	20.14
544	20.09	21.54	35.41	-	-	0.005267	-	-	1.01497497	-	0.270466334	-	0.005267	0.301769	3.551599	203.491613	11	3.811	-1.519038723	-3.494949	58.537532	0.5853753	0.04482536	0.5853753	-0.347575	-	-	1.3	1.2	0.092538	-	21.54	20.09
545	20.03	21.52	35.48	-	-	0.005267	-	-	1.012637444	-	0.268288334	-	0.005267	0.301769	3.556866	203.7933819	11	3.811	-1.537424979	-3.4869	58.537532	0.5853753	0.047371037	0.5853753	-0.345028	-	-	1.3	1.2	0.092538	-	21.52	20.03
546	19.97	21.49	35.54	-	-	0.005267	-	-	1.010278186	-	0.266091127	-	0.005267	0.301769	3.562132	204.0951508	11	3.811	-1.555768587	-3.478754	58.537532	0.5853753	0.049497528	0.5853753	-0.342455	-	-	1.3	1.2	0.092538	-	21.49	19.97
547	19.91	21.46	35.61	-	-	0.005267	-	-	1.007878184	-	0.263876065	-	0.005267	0.301769	3.567399	204.3969197	11	3.811	-1.574069038	-3.470512	58.537532	0.5853753	0.052554759	0.5853753	-0.339845	-	-	1.3	1.2	0.092538	-	21.46	19.91
548	19.85	21.44	35.67	-	-	0.005267	-	-	1.005456584	-	0.261642444	-	0.005267	0.301769	3.572666	204.6986887	11	3.811	-1.592325825	-3.462173	58.537532	0.5853753	0.05519266	0.5853753	-0.337207	-	-	1.3	1.2	0.092538	-	21.44	19.85
549	19.79	21.41	35.74	-	-	0.005267	-	-	1.003007093	-	0.259390935	-	0.005267	0.301769	3.577933	205.0004576	11	3.811	-1.610538441	-3.453739	58.537532	0.5853753	0.057861156	0.5853753	-0.334539	-	-	1.3	1.2	0.092538	-	21.41	19.79
550	19.73	21.38	35.81	-	-	0.005267	-	-	1.000529778	-	0.257121871	-	0.005267	0.301769	3.583200	205.3022265	11	3.811	-1.628706382	-3.445208	58.537532	0.5853753	0.060560173	0.5853753	-0.33184	-	-	1.3	1.2	0.092538	-	21.38	19.73
551	19.67	21.35	35.87	-	-	0.005267	-	-	0.998024709	-	0.254835586	-	0.005267	0.301769	3.588467	205.6039954	11	3.811	-1.646829142	-3.436583	58.537532	0.5853753	0.063289637	0.5853753	-0.32911	-	-	1.3	1.2	0.092538	-	21.35	19.67
552	19.61	21.33	35.94	-	-	0.005267	-	-	0.995495316	-	0.252523246	-	0.005267	0.301769	3.593733	205.9037643	11	3.811	-1.66490622	-3.427861	58.537532	0.5853753	0.066049472	0.5853753	-0.326351	-	-	1.3	1.2	0.092538	-	21.33	19.61
553	19.55	21.30	35.99	-	-	0.005267	-	-	0.992921655	-	0.250217133	-	0.005267	0.301769	3.599002	206.2075319	11	3.811	-1.682971132	-3.419548	58.537532	0.5853753	0.068839602	0.5853753	-0.323538	-	-	1.3	1.2	0.092538	-	21.30	19.55
554	19.49	21.27	36.07	-	-	0.005267	-	-	0.990343674	-	0.247876706	-	0.005267	0.301769	3.604267	206.5093021	11	3.811	-1.700921233	-3.410134	58.537532	0.5853753	0.071659499	0.5853753	-0.32074	-	-	1.3	1.2	0.092538	-	21.27	19.49
555	19.44	21.24	36.13	-	-	0.005267	-	-	0.987728219	-	0.245525013	-	0.005267	0.301769	3.609534	206.811071	11	3.811	-1.718858349	-3.401128	58.537532	0.5853753	0.074510434	0.5853753	-0.31789	-	-	1.3	1.2	0.092538	-	21.24	19.44
556	19.38	21.21	36.20	-	-	0.005267	-	-	0.985085506	-	0.243157733	-	0.005267	0.301769	3.614801	207.1128399	11	3.811	-1.736747694	-3.392028	58.537532	0.5853753	0.077390979	0.5853753	-0.315009	-	-	1.4	1.2	0.092538	-	21.21	19.38
557	19.32	21.18	36.26	-	-	0.005267	-	-	0.982415397	-	0.240775295	-	0.005267	0.301769	3.620068	207.4146088	11	3.811	-1.754588863	-3.382834	58.537532	0.5853753	0.080301504	0.5853753	-0.312098	-	-	1.4	1.2	0.092538	-	21.18	19.32
558	19.26	21.15	36.33	-	-	0.005267	-	-	0.979718035	-	0.238378039	-	0.005267	0.301769	3.625335	207.7163777	11	3.811	-1.772381359	-3.373546	58.537532	0.5853753	0.083241928	0.5853753	-0.309158	-	-	1.4	1.2	0.092538	-	21.15	19.26
559	19.20																																

656	14.21	17.29	42.71	-	-	-	0.811376653	-	-	0	3.889447	222.8489106	11	0.804	-0.546667393	-0.589337	12.347818	0.1234872	0.461897141	0.5853753	0.0694971	-	0.3	1.4	1.2	0.092538	-	17.29	14.21
657	14.16	17.25	42.77	-	-	-	0.811376653	-	-	0	3.889447	222.8489106	11	0.776	-0.527439358	-0.568608	11.913506	0.1191351	0.466240263	0.5853753	0.0738403	-	0.3	1.4	1.2	0.092538	-	17.25	14.16
658	14.11	17.20	42.84	-	-	-	0.811376653	-	-	0	3.889447	222.8489106	11	0.747	-0.508211323	-0.547879	11.479194	0.1147919	0.470583384	0.5853753	0.0781834	-	0.3	1.4	1.2	0.092538	-	17.20	14.11
659	14.07	17.16	42.90	-	-	-	0.811376653	-	-	0	3.889447	222.8489106	11	0.719	-0.488983288	-0.527151	11.044882	0.1104488	0.474926505	0.5853753	0.0825265	-	0.3	1.4	1.2	0.092538	-	17.16	14.07
660	14.02	17.11	42.97	-	-	-	0.811376653	-	-	0	3.889447	222.8489106	11	0.691	-0.469755253	-0.506422	10.61057	0.1061057	0.479269626	0.5853753	0.0868696	-	0.3	1.4	1.2	0.092538	-	17.11	14.02
661	13.97	17.07	43.03	-	-	-	0.811376653	-	-	0	3.889447	222.8489106	11	0.662	-0.450527218	-0.485699	10.176257	0.1017626	0.483612748	0.5853753	0.0912127	-	0.3	1.4	1.2	0.092538	-	17.07	13.97
662	13.92	17.03	43.04	-	-	-	0.811376653	-	-	0	3.889447	222.8489106	11	0.634	-0.431299143	-0.467195	9.647949	0.0974195	0.487955869	0.5853753	0.0945556	-	0.3	1.4	1.2	0.092538	-	17.03	13.92
663	13.88	16.98	43.16	0.000854	-	-	0.809448884	-	-	0	3.889447	222.8489106	11	0.618	-0.420035707	-0.452821	9.4875322	0.0948753	0.49229899	0.5871743	0.0998999	0.0	0.3	1.4	1.2	0.092538	-	16.98	13.88
664	13.83	16.94	43.23	0.000854	-	-	0.806691318	-	-	0	3.889447	222.8489106	11	0.618	-0.420422097	-0.452463	9.4875322	0.0948753	0.496642111	0.5915174	0.1042421	0.0	0.3	1.4	1.2	0.092538	-	16.94	13.83
665	13.78	16.89	43.29	0.000854	-	-	0.803938575	-	-	0	3.889447	222.8489106	11	0.618	-0.420808018	-0.452104	9.4875322	0.0948753	0.500989228	0.5958645	0.1085892	0.0	0.3	1.4	1.2	0.092538	-	16.89	13.78
666	13.73	16.85	43.36	0.000854	-	-	0.801186666	-	-	0	3.889447	222.8489106	11	0.618	-0.421193957	-0.451744	9.4875322	0.0948753	0.505340336	0.6002157	0.1129403	0.0	0.3	1.4	1.2	0.092538	-	16.85	13.73
667	13.69	16.80	43.42	0.000854	-	-	0.798435603	-	-	0	3.889447	222.8489106	11	0.618	-0.421579427	-0.451384	9.4875322	0.0948753	0.509695434	0.6045708	0.1172954	0.0	0.3	1.4	1.2	0.092538	-	16.80	13.69
668	13.64	16.76	43.49	0.000854	-	-	0.795685397	-	-	0	3.889447	222.8489106	11	0.618	-0.421964589	-0.451024	9.4875322	0.0948753	0.514054517	0.6089298	0.1216545	0.0	0.3	1.4	1.2	0.092538	-	16.76	13.64
669	13.59	16.72	43.55	0.000854	-	-	0.792936959	-	-	0	3.889447	222.8489106	11	0.618	-0.422349444	-0.450664	9.4875322	0.0948753	0.518417583	0.6132929	0.1260176	0.0	0.3	1.4	1.2	0.092538	-	16.72	13.59
670	13.54	16.67	0.000854	-	-	-	0.790187601	-	-	0	3.889447	222.8489106	11	0.618	-0.422733992	-0.450303	9.4875322	0.0948753	0.522784628	0.6176599	0.1303844	0.0	0.3	1.4	1.2	0.092538	-	16.67	13.54
671	13.50	16.63	43.68	0.000854	-	-	0.787440033	-	-	0	3.889447	222.8489106	11	0.618	-0.423118231	-0.449942	9.4875322	0.0948753	0.527155649	0.622031	0.1347556	0.0	0.3	1.4	1.2	0.092538	-	16.63	13.50
672	13.45	16.58	43.75	0.000854	-	-	0.784693367	-	-	0	3.889447	222.8489106	11	0.618	-0.423502162	-0.449581	9.4875322	0.0948753	0.531530643	0.626406	0.1391306	0.0	0.3	1.4	1.2	0.092538	-	16.58	13.45
673	13.40	16.54	43.81	0.000854	-	-	0.781947614	-	-	0	3.889447	222.8489106	11	0.618	-0.423885784	-0.449219	9.4875322	0.0948753	0.535909607	0.6307849	0.1435096	0.0	0.3	1.4	1.2	0.092538	-	16.54	13.40
674	13.35	16.49	43.88	0.000854	-	-	0.779202786	-	-	0	3.889447	222.8489106	11	0.618	-0.424269098	-0.448857	9.4875322	0.0948753	0.540292538	0.6351679	0.1478925	0.0	0.3	1.4	1.2	0.092538	-	16.49	13.35
675	13.31	16.45	43.94	0.000854	-	-	0.776458893	-	-	0	3.889447	222.8489106	11	0.618	-0.424652102	-0.448495	9.4875322	0.0948753	0.544679432	0.6395548	0.1522794	0.0	0.3	1.4	1.2	0.092538	-	16.45	13.31
676	13.26	16.40	44.01	0.000854	-	-	0.773715948	-	-	0	3.889447	222.8489106	11	0.618	-0.425034797	-0.448132	9.4875322	0.0948753	0.549070286	0.6439456	0.1566703	0.0	0.3	1.4	1.2	0.092538	-	16.40	13.26
677	13.21	16.36	44.07	0.000854	-	-	0.770973996	-	-	0	3.889447	222.8489106	11	0.618	-0.425417182	-0.447769	9.4875322	0.0948753	0.553465098	0.6483404	0.1610551	0.0	0.3	1.4	1.2	0.092538	-	16.36	13.21
678	13.16	16.31	44.14	0.000854	-	-	0.768232942	-	-	0	3.889447	222.8489106	11	0.618	-0.425799257	-0.447406	9.4875322	0.0948753	0.557863863	0.6527392	0.1654639	0.0	0.3	1.4	1.2	0.092538	-	16.31	13.16
679	13.12	16.27	44.20	0.000854	-	-	0.765492904	-	-	0	3.889447	222.8489106	11	0.618	-0.426181022	-0.447402	9.4875322	0.0948753	0.562266578	0.6571419	0.1698666	0.0	0.3	1.4	1.2	0.092538	-	16.27	13.12
680	13.07	16.22	44.27	0.000854	-	-	0.762753859	-	-	0	3.889447	222.8489106	11	0.618	-0.426562477	-0.446678	9.4875322	0.0948753	0.566673241	0.6615486	0.1742732	0.0	0.3	1.4	1.2	0.092538	-	16.22	13.07
681	13.02	16.18	44.33	0.000854	-	-	0.760015816	-	-	0	3.889447	222.8489106	11	0.618	-0.42694362	-0.446314	9.4875322	0.0948753	0.571083489	0.6659592	0.1786838	0.0	0.3	1.4	1.2	0.092538	-	16.18	13.02
682	12.98	16.13	44.40	0.000854	-	-	0.757278788	-	-	0	3.889447	222.8489106	11	0.618	-0.427324453	-0.445949	9.4875322	0.0948753	0.575498397	0.6703737	0.1830984	0.0	0.3	1.4	1.2	0.092538	-	16.13	12.98
683	12.93	16.09	44.46	0.000854	-	-	0.754542786	-	-	0	3.889447	222.8489106	11	0.618	-0.427704794	-0.445585	9.4875322	0.0948753	0.579916883	0.6747922	0.1875169	0.0	0.3	1.4	1.2	0.092538	-	16.09	12.93
684	12.88	16.04	44.53	0.000854	-	-	0.751821621	-	-	0	3.889447	222.8489106	11	0.618	-0.428085183	-0.445219	9.4875322	0.0948753	0.584339325	0.6792146	0.1919399	0.0	0.3	1.4	1.2	0.092538	-	16.04	12.88
685	12.83	16.00	44.59	0.000854	-	-	0.749073989	-	-	0	3.889447	222.8489106	11	0.618	-0.428468136	-0.444851	9.4875322	0.0948753	0.588765656	0.683641	0.196365	0.0	0.3	1.4	1.2	0.092538	-	16.00	12.83
686	12.79	15.95	44.66	0.000854	-	-	0.746341046	-	-	0	3.889447	222.8489106	11	0.618	-0.428844666	-0.444488	9.4875322	0.0948753	0.593195936	0.6880713	0.2007953	0.0	0.3	1.4	1.2	0.092538	-	15.95	12.79
687	12.74	15.91	44.72	0.000854	-	-	0.743609258	-	-	0	3.889447	222.8489106	11	0.618	-0.429223938	-0.444122	9.4875322	0.0948753	0.597630141	0.6925055	0.2052301	0.0	0.3	1.4	1.2	0.092538	-	15.91	12.74
688	12.69	15.86	44.79	0.000854	-	-	0.740878553	-	-	0	3.889447	222.8489106	11	0.618	-0.429602898	-0.443755	9.4875322	0.0948753	0.602062627	0.6969436	0.2096683	0.0	0.3	1.4	1.2	0.092538	-	15.86	12.69
689	12.65	15.82	44.85	0.000854	-	-	0.738148941	-	-	0	3.889447	222.8489106	11	0.618	-0.429981545	-0.443388	9.4875322	0.0948753	0.606510312	0.7013856	0.2141103	0.0	0.3	1.4	1.2	0.092538	-	15.82	12.65
690	12.60	15.77	44.92	0.000854	-	-	0.735420433	-	-	0	3.889447	222.8489106	11	0.618	-0.430359879	-0.443021	9.4875322	0.0948753	0.610956272	0.7058316	0.2185563	0.0	0.3	1.4	1.2	0.092538	-	15.77	12.60
691	12.55	15.73	44.98	0.000854	-	-	0.732639041	-	-	0	3.889447	222.8489106	11	0.618	-0.430737899	-0.442659	9.4875322	0.0948753	0.615406144	0.7102815	0.2230061	0.0	0.3	1.4	1.2	0.092538	-	15.73	12.55
692	12.51	15.68	45.04	0.000854	-	-	0.729870776	-	-	0	3.889447	222.8489106	11	0.618	-0.431156675	-0.442285	9.4875322	0.0948753	0.619859925	0.7147352	0.2274596	0.0	0.3	1.4	1.2	0.092538	-	15.68	12.51
693	12.46	15.64	45.11	0.000854	-	-	0.727241649	-	-	0	3.889447	222.8489106	11	0.618	-0.431492997	-0.441917	9.4875322	0.0948753	0.624317611	0.7191929	0.2319176	0.0	0.3	1.4	1.2	0.092538	-	15.64	12.46
694	12.41	15.59	45.18	0.000854	-	-	0.724517672	-	-	0	3.889447	222.8489106	11	0.618	-0.431870074	-0.441549	9.4875322	0.0948753	0.628779199	0.7236545	0.2363792	0.0	0.3	1.4	1.2	0.092538	-	15.59	12.41
695	12.37	15.54	45.24	0.000854	-	-	0.721794855	-	-	0	3.889447	222.8489106	11	0.618	-0.432246837	-0.44118	9.4875322	0.0948753	0.6										

788	8.21	11.14	51.30	0.000854	-	-	-	-	0.475199044	-	0	0	-	0.000854	0.04891	3.996151	228.9625998	11	0.618	-0.465872975	-0.405511	9.4875322	0.0948753	1.065117971	1.1599933	0.6727218	0.1	0.3	1.4	1.2	0.092538	-	11.14	8.21
789	8.17	11.09	51.36	0.000854	-	-	-	-	0.472635358	-	0	0	-	0.000854	0.04891	3.997005	229.0115093	11	0.618	-0.466218963	-0.405114	9.4875322	0.0948753	1.069935045	1.1648104	0.677353	0.1	0.3	1.4	1.2	0.092538	-	11.09	8.17
790	8.13	11.04	51.43	0.000854	-	-	-	-	0.470073895	-	0	0	-	0.000854	0.04891	3.997858	229.0604188	11	0.618	-0.466564611	-0.404715	9.4875322	0.0948753	1.074755696	1.169631	0.6823557	0.1	0.3	1.4	1.2	0.092538	-	11.04	8.13
791	8.09	11.00	51.49	0.000854	-	-	-	-	0.467514664	-	0	0	-	0.000854	0.04891	3.998712	229.1093283	11	0.618	-0.466909919	-0.404317	9.4875322	0.0948753	1.07957992	1.1744552	0.6871799	0.1	0.3	1.4	1.2	0.092538	-	11.00	8.09
792	8.04	10.95	51.56	0.000854	-	-	-	-	0.464957677	-	0	0	-	0.000854	0.04891	3.999566	229.1582378	11	0.618	-0.467254887	-0.403918	9.4875322	0.0948753	1.084407716	1.179283	0.6920077	0.1	0.3	1.4	1.2	0.092538	-	10.95	8.04
793	8.00	10.90	51.62	0.000854	-	-	-	-	0.462402945	-	0	0	-	0.000854	0.04891	4.000419	229.2071473	11	0.618	-0.467599514	-0.403519	9.4875322	0.0948753	1.089239078	1.1841144	0.6968391	0.1	0.3	1.4	1.2	0.092538	-	10.90	8.00
794	7.96	10.85	51.65	0.000854	-	-	-	-	0.461850668	-	0	0	-	0.000854	0.04891	4.001279	229.2560668	11	0.618	-0.467943870	-0.403119	9.4875322	0.0948753	1.094074004	1.1894993	0.6976794	0.1	0.3	1.4	1.2	0.092538	-	10.85	7.96
795	7.92	10.80	51.70	0.000854	-	-	-	-	0.45730029	-	0	0	-	0.000854	0.04891	4.002127	229.3049664	11	0.618	-0.468287746	-0.40272	9.4875322	0.0948753	1.098912489	1.1937878	0.7065125	0.1	0.3	1.4	1.2	0.092538	-	10.80	7.92
796	7.87	10.75	51.82	0.000854	-	-	-	-	0.454752389	-	0	0	-	0.000854	0.04891	4.002980	229.3538759	11	0.618	-0.468631351	-0.40232	9.4875322	0.0948753	1.103754531	1.1986299	0.7113545	0.1	0.3	1.4	1.2	0.092538	-	10.75	7.87
797	7.83	10.70	51.88	0.000854	-	-	-	-	0.452206787	-	0	0	-	0.000854	0.04891	4.003834	229.4027854	11	0.618	-0.468974614	-0.40192	9.4875322	0.0948753	1.108600126	1.2034754	0.7162001	0.1	0.3	1.4	1.2	0.092538	-	10.70	7.83
798	7.79	10.65	51.95	0.000854	-	-	-	-	0.449663496	-	0	0	-	0.000854	0.04891	4.004688	229.4516949	11	0.618	-0.469317535	-0.40152	9.4875322	0.0948753	1.11344927	1.2083246	0.7210493	0.1	0.3	1.4	1.2	0.092538	-	10.65	7.79
799	7.75	10.60	52.01	0.000854	-	-	-	-	0.447122525	-	0	0	-	0.000854	0.04891	4.005541	229.5006044	11	0.618	-0.469660114	-0.401119	9.4875322	0.0948753	1.118301959	1.2131773	0.725902	0.1	0.3	1.4	1.2	0.092538	-	10.60	7.75
800	7.70	10.55	52.08	0.000854	-	-	-	-	0.444583886	-	0	0	-	0.000854	0.04891	4.006395	229.5495139	11	0.618	-0.470002351	-0.400718	9.4875322	0.0948753	1.123158191	1.2180335	0.7307582	0.1	0.3	1.4	1.2	0.092538	-	10.55	7.70
801	7.66	10.50	52.15	0.000854	-	-	-	-	0.442047074	-	0	0	-	0.000854	0.04891	4.007248	229.5994234	11	0.618	-0.470344245	-0.400317	9.4875322	0.0948753	1.128017962	1.2228933	0.735618	0.1	0.3	1.4	1.2	0.092538	-	10.50	7.66
802	7.62	10.45	52.21	0.000854	-	-	-	-	0.439513649	-	0	0	-	0.000854	0.04891	4.008102	229.6473329	11	0.618	-0.470685797	-0.399915	9.4875322	0.0948753	1.132881268	1.2277566	0.7404813	0.1	0.3	1.4	1.2	0.092538	-	10.45	7.62
803	7.58	10.40	52.28	0.000854	-	-	-	-	0.436982072	-	0	0	-	0.000854	0.04891	4.008956	229.6962425	11	0.618	-0.471027006	-0.399513	9.4875322	0.0948753	1.137748105	1.2326234	0.7453481	0.1	0.3	1.4	1.2	0.092538	-	10.40	7.58
804	7.53	10.35	52.34	0.000854	-	-	-	-	0.434452871	-	0	0	-	0.000854	0.04891	4.009809	229.745152	11	0.618	-0.471367871	-0.399111	9.4875322	0.0948753	1.142618471	1.2374938	0.7502185	0.1	0.3	1.4	1.2	0.092538	-	10.35	7.53
805	7.49	10.30	52.41	0.000854	-	-	-	-	0.431926058	-	0	0	-	0.000854	0.04891	4.010663	229.7940615	11	0.618	-0.471708393	-0.398708	9.4875322	0.0948753	1.147492361	1.2423677	0.7550924	0.1	0.3	1.4	1.2	0.092538	-	10.30	7.49
806	7.45	10.25	52.47	0.000854	-	-	-	-	0.429401642	-	0	0	-	0.000854	0.04891	4.011517	229.842971	11	0.618	-0.472048572	-0.398306	9.4875322	0.0948753	1.152369772	1.2472451	0.7599698	0.1	0.3	1.4	1.2	0.092538	-	10.25	7.45
807	7.41	10.20	52.54	0.000854	-	-	-	-	0.426879635	-	0	0	-	0.000854	0.04891	4.012370	229.8918805	11	0.618	-0.472388406	-0.397902	9.4875322	0.0948753	1.1572507	1.252126	0.7648507	0.1	0.3	1.4	1.2	0.092538	-	10.20	7.41
808	7.37	10.15	52.60	0.000854	-	-	-	-	0.424360049	-	0	0	-	0.000854	0.04891	4.013224	229.94079	11	0.618	-0.472727896	-0.397499	9.4875322	0.0948753	1.162135142	1.2570105	0.7697351	0.1	0.3	1.4	1.2	0.092538	-	10.15	7.37
809	7.32	10.10	52.67	0.000854	-	-	-	-	0.421842893	-	0	0	-	0.000854	0.04891	4.014078	229.9896995	11	0.618	-0.473087042	-0.397095	9.4875322	0.0948753	1.167023095	1.2618984	0.7746231	0.1	0.3	1.4	1.2	0.092538	-	10.10	7.32
810	7.28	10.05	52.73	0.000854	-	-	-	-	0.419328179	-	0	0	-	0.000854	0.04891	4.014931	230.0386901	11	0.618	-0.473405842	-0.396691	9.4875322	0.0948753	1.171914554	1.2667899	0.7795144	0.1	0.3	1.4	1.2	0.092538	-	10.05	7.28
811	7.24	10.00	52.80	0.000854	-	-	-	-	0.416815918	-	0	0	-	0.000854	0.04891	4.015785	230.0875186	11	0.618	-0.473744298	-0.396287	9.4875322	0.0948753	1.176809516	1.2716848	0.7844095	0.1	0.3	1.4	1.2	0.092538	-	10.00	7.24
812	7.20	9.95	52.86	0.000854	-	-	-	-	0.41430612	-	0	0	-	0.000854	0.04891	4.016638	230.1364281	11	0.618	-0.474082409	-0.395883	9.4875322	0.0948753	1.181707978	1.2765833	0.7893908	0.1	0.3	1.4	1.2	0.092538	-	9.95	7.20
813	7.16	9.90	52.93	0.000854	-	-	-	-	0.411798797	-	0	0	-	0.000854	0.04891	4.017492	230.1853376	11	0.618	-0.474420174	-0.395478	9.4875322	0.0948753	1.186609936	1.2814853	0.7942099	0.1	0.3	1.4	1.2	0.092538	-	9.90	7.16
814	7.12	9.85	52.99	0.000854	-	-	-	-	0.40929396	-	0	0	-	0.000854	0.04891	4.018346	230.2342471	11	0.618	-0.474757594	-0.395073	9.4875322	0.0948753	1.191515387	1.2863907	0.7991154	0.1	0.3	1.4	1.2	0.092538	-	9.85	7.12
815	7.07	9.80	53.06	0.000854	-	-	-	-	0.40679162	-	0	0	-	0.000854	0.04891	4.019199	230.2831566	11	0.618	-0.475094668	-0.394697	9.4875322	0.0948753	1.196424326	1.2912996	0.8040243	0.1	0.3	1.4	1.2	0.092538	-	9.80	7.07
816	7.03	9.75	53.12	0.000854	-	-	-	-	0.404291187	-	0	0	-	0.000854	0.04891	4.020053	230.3320661	11	0.618	-0.475431395	-0.394261	9.4875322	0.0948753	1.201336751	1.2962121	0.8083668	0.1	0.3	1.4	1.2	0.092538	-	9.75	7.03
817	6.99	9.70	53.19	0.000854	-	-	-	-	0.401785127	-	0	0	-	0.000854	0.04891	4.020907	230.3809772	11	0.618	-0.475768772	-0.393852	9.4875322	0.0948753	1.206253651	1.301128	0.813521	0.1	0.3	1.4	1.2	0.092538	-	9.70	6.99
818	6.95	9.65	53.25	0.000854	-	-	-	-	0.399299687	-	0	0	-	0.000854	0.04891	4.021760	230.4298852	11	0.618	-0.47610381	-0.393449	9.4875322	0.0948753	1.211172042	1.3060474	0.818772	0.1	0.3	1.4	1.2	0.092538	-	9.65	6.95
819	6.91	9.60	53.32	0.000854	-	-	-	-	0.396807442	-	0	0	-	0.000854	0.04891	4.022614	230.4787947	11	0.618	-0.476439498	-0.393043	9.4875322	0.0948753	1.216094901	1.3109702	0.8236949	0.1	0.3	1.4	1.2	0.092538	-	9.60	6.91
820	6.87	9.55	53.38	0.000854	-	-	-	-	0.394317748	-	0	0	-	0.000854	0.04891	4.023467	230.5277042	11	0.618	-0.476774838	-0.392636	9.4875322	0.0948753	1.221021231	1.3158966	0.8286212	0.1	0.3	1.4	1.2	0.092538	-	9.55	6.87
821	6.83	9.50	53.45	0.000854	-	-	-	-	0.391830617	-	0	0	-	0.000854	0.04891	4.024321	230.5766137	11	0.618	-0.47710983	-0.392229	9.4875322	0.0948753	1.225951028	1.3208263	0.833551	0.1	0.3	1.4	1.2	0.092538	-	9.50	6.83
822	6.78	9.45	53.51	0.000854	-	-	-	-	0.389346058	-	0	0	-	0.000854	0.04891	4.025175	230.6255232	11	0.618	-0.477444476	-0.391821	9.4875322	0.0948753	1.230884289	1.3257596	0.8384843	0.1	0.3	1.4	1.2	0.092538	-	9.45	6.78
823	6.74	9.40	53.58	0.000854	-																													

920	2.95	4.36	59.89	0.000854	-	-	-	-	-	0.160081188	-	0	-	0.000854	0.04891	4.108831	235.4186555	11	0.618	-0.508514753	-0.350557	9.4875322	0.0948753	1.730555749	1.8254311	1.3381557	0.2	0.3	1.4	1.2	0.092538	-	4.36	2.95
921	2.91	59.96	0.000854	-	-	-	-	-	-	0.157904367	-	0	-	0.000854	0.04891	4.109684	235.4675651	11	0.618	-0.508813814	-0.350122	9.4875322	0.0948753	1.735813733	1.8308691	1.3434137	0.2	0.3	1.4	1.2	0.092538	-	4.31	2.91
922	2.87	4.25	60.02	0.000854	-	-	-	-	-	0.155731179	-	0	-	0.000854	0.04891	4.110538	235.5164746	11	0.618	-0.509112504	-0.349688	9.4875322	0.0948753	1.74107481	1.8359501	1.3486748	0.2	0.3	1.4	1.2	0.092538	-	4.25	2.87
923	2.83	4.20	60.09	0.000854	-	-	-	-	-	0.153561634	-	0	-	0.000854	0.04891	4.111392	235.5653841	11	0.618	-0.509410824	-0.349253	9.4875322	0.0948753	1.746338975	1.8412143	1.353939	0.2	0.3	1.4	1.2	0.092538	-	4.20	2.83
924	2.80	4.14	60.15	0.000854	-	-	-	-	-	0.151395742	-	0	-	0.000854	0.04891	4.112245	235.6142936	11	0.618	-0.509708772	-0.348818	9.4875322	0.0948753	1.751606225	1.8464815	1.3592062	0.2	0.3	1.4	1.2	0.092538	-	4.14	2.80
925	2.76	4.09	60.22	0.000854	-	-	-	-	-	0.149233513	-	0	-	0.000854	0.04891	4.113099	235.6632031	11	0.618	-0.510006348	-0.348383	9.4875322	0.0948753	1.756876556	1.8517519	1.3644766	0.2	0.3	1.4	1.2	0.092538	-	4.09	2.76
926	2.72	4.04	60.28	0.000854	-	-	-	-	-	0.147074958	-	0	-	0.000854	0.04891	4.113732	235.7111128	11	0.618	-0.510303553	-0.347947	9.4875322	0.0948753	1.762149963	1.8570253	1.369756	0.2	0.3	1.4	1.2	0.092538	-	4.04	2.72
927	2.69	3.98	60.35	0.000854	-	-	-	-	-	0.144920087	-	0	-	0.000854	0.04891	4.114806	235.7610221	11	0.618	-0.510600386	-0.347512	9.4875322	0.0948753	1.767426443	1.8623018	1.3750264	0.2	0.3	1.4	1.2	0.092538	-	3.98	2.69
928	2.65	3.93	60.41	0.000854	-	-	-	-	-	0.14276891	-	0	-	0.000854	0.04891	4.115660	235.8099317	11	0.618	-0.510896847	-0.347076	9.4875322	0.0948753	1.772705993	1.8675813	1.380306	0.2	0.3	1.4	1.2	0.092538	-	3.93	2.65
929	2.61	3.88	60.48	0.000854	-	-	-	-	-	0.140621437	-	0	-	0.000854	0.04891	4.116513	235.8588412	11	0.618	-0.511192936	-0.346639	9.4875322	0.0948753	1.777988608	1.8728639	1.3855886	0.2	0.3	1.4	1.2	0.092538	-	3.88	2.61
930	2.58	3.82	60.54	0.000854	-	-	-	-	-	0.138477679	-	0	-	0.000854	0.04891	4.117367	235.9077507	11	0.618	-0.511488652	-0.346203	9.4875322	0.0948753	1.783274285	1.8781496	1.3908743	0.2	0.3	1.4	1.2	0.092538	-	3.82	2.58
931	2.54	3.77	60.61	0.000854	-	-	-	-	-	0.136337645	-	0	-	0.000854	0.04891	4.118221	235.9566602	11	0.618	-0.511783996	-0.345766	9.4875322	0.0948753	1.788563019	1.8834383	1.396163	0.2	0.3	1.4	1.2	0.092538	-	3.77	2.54
932	2.51	3.71	60.67	0.000854	-	-	-	-	-	0.134201346	-	0	-	0.000854	0.04891	4.119074	236.0055697	11	0.618	-0.512078967	-0.345329	9.4875322	0.0948753	1.793854807	1.8887301	1.4014548	0.2	0.3	1.4	1.2	0.092538	-	3.71	2.51
933	2.47	3.66	60.74	0.000854	-	-	-	-	-	0.132068792	-	0	-	0.000854	0.04891	4.119928	236.0544792	11	0.618	-0.512373564	-0.344893	9.4875322	0.0948753	1.799149565	1.894025	1.4067496	0.2	0.3	1.4	1.2	0.092538	-	3.66	2.47
934	2.43	3.61	60.80	0.000854	-	-	-	-	-	0.129939992	-	0	-	0.000854	0.04891	4.120782	236.1033887	11	0.618	-0.512667798	-0.344454	9.4875322	0.0948753	1.80444753	1.8993229	1.4120274	0.2	0.3	1.4	1.2	0.092538	-	3.61	2.43
935	2.40	3.55	60.87	0.000854	-	-	-	-	-	0.127814958	-	0	-	0.000854	0.04891	4.121635	236.1522983	11	0.618	-0.512961639	-0.344017	9.4875322	0.0948753	1.809748456	1.9046238	1.4173485	0.2	0.3	1.4	1.2	0.092538	-	3.55	2.40
936	2.36	3.50	60.93	0.000854	-	-	-	-	-	0.125693699	-	0	-	0.000854	0.04891	4.122489	236.2012078	11	0.618	-0.513255115	-0.343579	9.4875322	0.0948753	1.815052421	1.9099277	1.4226524	0.2	0.3	1.4	1.2	0.092538	-	3.50	2.36
937	2.32	3.44	61.00	0.000854	-	-	-	-	-	0.123576226	-	0	-	0.000854	0.04891	4.123342	236.2501173	11	0.618	-0.513548218	-0.34314	9.4875322	0.0948753	1.82035942	1.9152347	1.4279594	0.2	0.3	1.4	1.2	0.092538	-	3.44	2.32
938	2.29	3.39	61.06	0.000854	-	-	-	-	-	0.121462548	-	0	-	0.000854	0.04891	4.124196	236.2990268	11	0.618	-0.513840947	-0.342702	9.4875322	0.0948753	1.82566945	1.9205448	1.4332695	0.2	0.3	1.4	1.2	0.092538	-	3.39	2.29
939	2.25	3.34	61.13	0.000854	-	-	-	-	-	0.119352675	-	0	-	0.000854	0.04891	4.125050	236.3479363	11	0.618	-0.514133301	-0.342263	9.4875322	0.0948753	1.830982507	1.9258578	1.4385825	0.2	0.3	1.4	1.2	0.092538	-	3.34	2.25
940	2.22	3.28	61.19	0.000854	-	-	-	-	-	0.117246618	-	0	-	0.000854	0.04891	4.125903	236.3968458	11	0.618	-0.514425228	-0.341824	9.4875322	0.0948753	1.836298586	1.9311739	1.4438986	0.2	0.3	1.4	1.2	0.092538	-	3.28	2.22
941	2.18	3.23	61.26	0.000854	-	-	-	-	-	0.115144387	-	0	-	0.000854	0.04891	4.126757	236.4457553	11	0.618	-0.514716885	-0.341385	9.4875322	0.0948753	1.841617685	1.9364993	1.4492177	0.2	0.3	1.4	1.2	0.092538	-	3.23	2.18
942	2.14	3.17	61.32	0.000854	-	-	-	-	-	0.113045992	-	0	-	0.000854	0.04891	4.127611	236.4946648	11	0.618	-0.515008114	-0.340945	9.4875322	0.0948753	1.846693799	1.9418151	1.4545398	0.2	0.3	1.4	1.2	0.092538	-	3.17	2.14
943	2.11	3.12	61.39	0.000854	-	-	-	-	-	0.110951442	-	0	-	0.000854	0.04891	4.128464	236.5435744	11	0.618	-0.515298969	-0.340506	9.4875322	0.0948753	1.852264924	1.9471402	1.4598644	0.2	0.3	1.4	1.2	0.092538	-	3.12	2.11
944	2.07	3.06	61.45	0.000854	-	-	-	-	-	0.108860749	-	0	-	0.000854	0.04891	4.129318	236.5924839	11	0.618	-0.515589447	-0.340066	9.4875322	0.0948753	1.857593057	1.9524684	1.4651931	0.2	0.3	1.4	1.2	0.092538	-	3.06	2.07
945	2.04	3.01	61.52	0.000854	-	-	-	-	-	0.106773921	-	0	-	0.000854	0.04891	4.130171	236.6413934	11	0.618	-0.51587955	-0.339625	9.4875322	0.0948753	1.862924193	1.9577995	1.4705242	0.2	0.3	1.4	1.2	0.092538	-	3.01	2.04
946	2.00	2.96	61.58	0.000854	-	-	-	-	-	0.104669097	-	0	-	0.000854	0.04891	4.131025	236.6903029	11	0.618	-0.516169277	-0.339185	9.4875322	0.0948753	1.868258328	1.9631337	1.4758583	0.2	0.3	1.4	1.2	0.092538	-	2.96	2.00
947	1.96	2.90	61.65	0.000854	-	-	-	-	-	0.102611904	-	0	-	0.000854	0.04891	4.131879	236.7392124	11	0.618	-0.516458628	-0.338744	9.4875322	0.0948753	1.873595546	1.9684708	1.4811955	0.2	0.3	1.4	1.2	0.092538	-	2.90	1.96
948	1.93	2.85	61.71	0.000854	-	-	-	-	-	0.100536735	-	0	-	0.000854	0.04891	4.132732	236.7881219	11	0.618	-0.516747603	-0.338303	9.4875322	0.0948753	1.878935583	1.9738109	1.4865356	0.2	0.3	1.4	1.2	0.092538	-	2.85	1.93
949	1.89	2.79	61.78	0.000854	-	-	-	-	-	0.098465471	-	0	-	0.000854	0.04891	4.133586	236.8370314	11	0.618	-0.517036201	-0.337862	9.4875322	0.0948753	1.884278594	1.979154	1.4919787	0.2	0.3	1.4	1.2	0.092538	-	2.79	1.89
950	1.86	2.74	61.85	0.000854	-	-	-	-	-	0.096398124	-	0	-	0.000854	0.04891	4.134400	236.885941	11	0.618	-0.517324423	-0.33742	9.4875322	0.0948753	1.88962479	1.9845001	1.4972248	0.2	0.3	1.4	1.2	0.092538	-	2.74	1.86
951	1.82	2.68	61.91	0.000854	-	-	-	-	-	0.094334702	-	0	-	0.000854	0.04891	4.135293	236.9348505	11	0.618	-0.517612267	-0.336979	9.4875322	0.0948753	1.894973865	1.9898492	1.5025739	0.2	0.3	1.4	1.2	0.092538	-	2.68	1.82
952	1.79	2.63	61.98	0.000854	-	-	-	-	-	0.092275217	-	0	-	0.000854	0.04891	4.136147	236.98376	11	0.618	-0.517899734	-0.336537	9.4875322	0.0948753	1.900325917	1.9952012	1.5079259	0.2	0.3	1.4	1.2	0.092538	-	2.63	1.79
953	1.75	2.57	62.04	0.000854	-	-	-	-	-	0.090219678	-	0	-	0.000854	0.04891	4.137001	237.0326695	11	0.618	-0.518186824	-0.336095	9.4875322	0.0948753	1.905680941	2.0005563	1.5132809	0.2	0.3	1.4	1.2	0.092538	-	2.57	1.75
954	1.72	2.52	62.11	0.000854	-	-	-	-	-	0.088168094	-	0	-	0.000854	0.04891	4.137854	237.081579	11	0.618	-0.518477536	-0.335652	9.4875322	0.0948753	1.911038934	2.0059144	1.5186389	0.2	0.3	1.4	1.2	0.092538	-	2.52	1.72
955	1.68	2.46	62.17	0.000854	-	-	-	-	-	0.086120476	-	0	-	0.000854	0.04891</																			

Appendix B: Optimization of membrane force

Here the spread sheet is used to optimize the configurations of the inflatable rubber barrier by changing the the base width B, the circumferential membrane length or all parameters at once.

B.1. Variation of base width B

By changing the base width to $B = 34.5$ m the resulting air pressure in the barrier becomes 0.59 bar and the tension force results to 711.3 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.591057 bar Luchtdruk
34.5 m Breedte tussen klemlijnen
65.1 m Lengte membraan

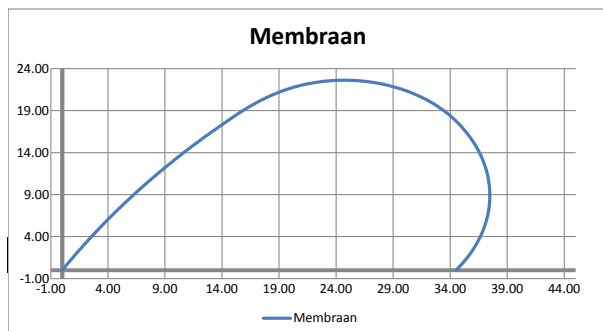
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	34.50	m
Lengte membraan	Lmin	65.10	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.59	Bar
Druk balg in meter waterkolom	H	5.91	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden		Bandbreedte	
Trek in balg		70	1000 kN
Aanvangshoek		0.15	2.5 Rad
Luchtdruk (bar)		0.01	0.6 bar

Vaste gegevens			
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschalkoord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.07	m

Doek eigenschappen			
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.13	kg
		11.06	N



WATER/ LUCHT

Output

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
Hverval		785	kN
Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	365.1	427.1	792.2 kN
RV	610.4	568.8	1'179.2 kN
Verschil verval en horizontaal klemlijn		1%	7.45 kN

Berekenende waarden			
Tension	T	711.30	kN/m ²
		711305	N
Eind hoek	ϕ_B	h1	4.17 Rad
			239.1 Graden
			59.1 Graden
Start hoek	ϕ_A	h2	0.93 Rad
			53.1 Graden

Volumes		m ³	%
Water		519.61	86.27%
Lucht		82.72	13.73%
Totaal		602.33	

Coördinaten		X	Y
Maxima		37.48	22.62
Vast punt 1		-	-
Vast punt 2		34.50	-
Onnauwkeurigheid		0.00	0.00
		0.00	0.00
Onnauwkeurigheid		1.38	0.06
Totaal		0.06	
Factor			0.55
Ybij X max		-	
X bij Y max			24.71

Afwijkingen		m	%
Hbalg		1.38	
X		0.00	0.00%
Y		0.00	0.00%
Hverval		7.45	6.31%
Totaal			0.00%

Figure B.1.: Crest-height calculations with $B = 34.5$ m

By changing the base width to $B = 34.0$ m the resulting air pressure in the barrier becomes 0.59 bar and the tension force results to 710.07 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.59208 bar Luchtdruk
34 m Breedte tussen klemlijnen
65.1 m Lengte membraan

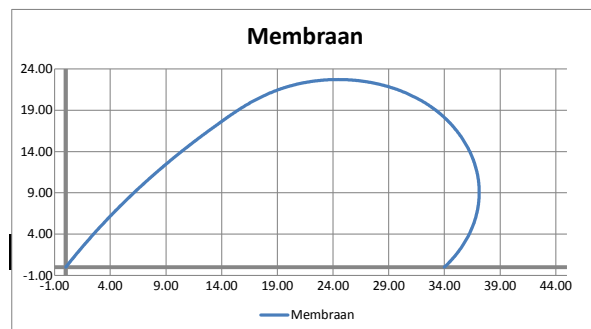
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	34.00	m
Lengte membraan	Lmin	65.10	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.59	Bar
Druk balg in meter waterkolom	H	5.92	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden		Bandbreedte	
Trek in balg		70	1000 kN
Aanvangshoek		0.15	2.5 Rad
Luchtdruk (bar)		0.01	0.6 bar

Vaste gegevens			
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschalxoord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.07	m

Doek eigenschappen			
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.13	kg
		11.06	N



WATER/AIR

Output

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
	Hverval	785	kN

Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	357.0	436.2	793.2 kN
RV	613.8	560.3	1'174.1 kN
Verscil verval en horizontaal klemlijn		1%	8.40 kN

Berekende waarden			
Tension	T	710.07	kN/m ²
Eind hoek	ϕB	h1	710070 N
			4.19 Rad
			239.8 Graden
Start hoek	ϕA	h2	59.8 Graden
			0.91 Rad
			52.1 Graden

Volumes		m ³	%
Water		516.48	85.93%
Lucht		84.54	14.07%
Totaal		601.02	

Coördinaten		X	Y
Maxima		37.12	22.72
Vast punt 1		-	-
Vast punt 2		34.00	-
Onnauwkeurigheid		0.00	-0.00
		0.01	0.00
Onnauwkeurigheid		1.28	0.05
Totaal		0.05	
Factor			0.55
Ybij X max		-	
X bij Y max			24.47

Afwijkingen		m	%
Hbalg		1.28	
X		0.00	0.02%
Y		0.00	0.02%
Hverval		8.40	7.11%
Totaal			0.03%

	Vaste input
	Vaste gegevens
	Berekende variabelen a.h.v. solver
	Berekende gegevens

Figure B.2.: Crest-height calculations with $B = 34.0$ m

By changing the base width to $B = 33.0$ m the resulting air pressure in the barrier becomes 0.59 bar and the tension force results to 709.87 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0,594911 bar Luchtdruk
33 m Breedte tussen klemlijnen
65,1 m Lengte membraan

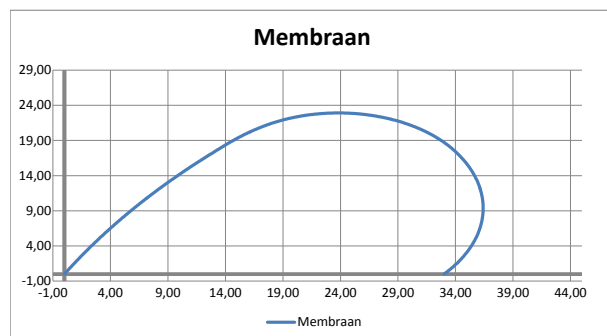
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	33,00	m
Lengte membraan	Lmin	65,10	m
Water hoogte balg	Hin	17,00	m
Luchtdruk (bar)	dP	0,59	Bar
Druk balg in meter waterkolom	H	5,95	m
Hoogte van de balg	Hbalg	24,00	m
Hoog water zijde	HW	22,00	m
Laag water zijde	LW	18,00	m

Te berekenen waardes		Bandbreedte	
Trek in balg		70	1000 kN
Aanvangshoek		0,15	2,5 Rad
Luchtdruk (bar)		0,01	0,6 bar

Vaste gegevens			
Zwaartekracht	g	9,81	m/s ²
Hoogte onbelast	Ho	22,00	m
Kruinhoogte	Hkruin	1,00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschaloord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0,07	m

Doek eigenschappen			
Breedte		1,00	m
Doek dikte		16,00	mm
Eigen gewicht doek		1082,00	kg/m ³
Elasticiteit doek		5000000,00	kN/m ²
Gewicht ds		1,13	kg
		11,06	N



WATER/ LUCHT

Output

Belastingen			
Hoogwaterzijde		2.374	kN
Laagwaterzijde		1.589	kN
Hverval		785	kN
Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	340,3	454,1	794,5 kN
RV	623,0	545,6	1.168,6 kN
Verschil verval en horizontaal klemlijn		1%	9,66 kN

Berekende waardes			
Tenstion	T	709,87	kN/m ²
		709874	N
Eind hoek	ϕB	h1	4,21 Rad
			241,4 Graden
			61,4 Graden
Start hoek	ϕA	h2	0,88 Rad
			50,2 Graden

Volumes		m ³	%
Water		510,86	85,26%
Lucht		88,35	14,74%
Totaal		599,21	

Coördinaten		X	Y
Maxima		36,40	22,91
Vast punt 1		-	-
Vast punt 2		33,00	-
Onnauwkeurigheid		0,01	0,01
		0,01	0,00
Onnauwkeurigheid		1,09	0,05
Totaal		0,05	
Factor			0,55
Ybij X max		-	-
X bij Y max		-	23,84

Afwijkingen		m	%
Hbalg		1,09	
X		0,01	0,03%
Y		0,01	0,03%
Hverval		9,66	8,19%
Totaal			0,06%

Figure B.3.: Crest-height calculations with $B = 33.0$ m

By changing the base width to $B = 32.0$ m the resulting air pressure in the barrier becomes 0.60 bar and the tension force results to 708.04 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.597803 bar Luchtdruk
32 m Breedte tussen klemlijnen
65.1 m Lengte membraan

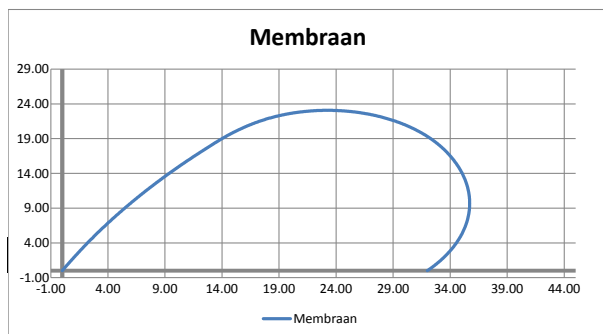
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	32.00	m
Lengte membraan	Lmin	65.10	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.60	Bar
Druk balg in meter waterkolom	H	5.98	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden		Bandbreedte	
Trek in balg		70	1000 kN
Aanvangshoek		0.15	2.5 Rad
Luchtdruk (bar)		0.01	0.6 bar

Vaste gegevens			
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschaloord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.07	m

Doek eigenschappen			
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.13	kg
		11.06	N



WATER/ LUCHT

Output

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
	Hverval	785	kN
Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	323.9	471.9	795.7 kN
RV	629.6	527.9	1'157.5 kN
Verschil verval en horizontaal klemlijn	1%	10.92	kN

Berekende waarden			
Tenstion	T	708.04	kN/m ²
		708036	N
Eind hoek	ϕB	h1	4.24 Rad
			242.8 Graden
Start hoek	ϕA	h2	62.8 Graden
			0.84 Rad
			48.2 Graden

Volumes		m ³	%
Water		505.23	84.70%
Lucht		91.28	15.30%
Totaal		596.51	

Coördinaten		X	Y
Maxima		35.71	23.07
Vast punt 1		-	-
Vast punt 2		32.00	-
Onnauwkeurigheid		-0.00	0.00
		0.00	0.00
Onnauwkeurigheid		0.93	0.04
Totaal		0.04	
Factor			0.55
Ybij X max		-	
X bij Y max			23.22

Afwijkingen		m	%
Hbalg		0.93	
X		0.00	0.00%
Y		0.00	0.00%
Hverval		10.92	9.25%
Totaal			0.00%

Figure B.4.: Crest-height calculations with $B = 32.0$ m

By changing the base width to $B = 31.0$ m the resulting air pressure in the barrier becomes 0.57 bar and the tension force results to 652.1 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.570971 bar Luchtdruk
31 m Breedte tussen klemlijnen
65.1 m Lengte membraan

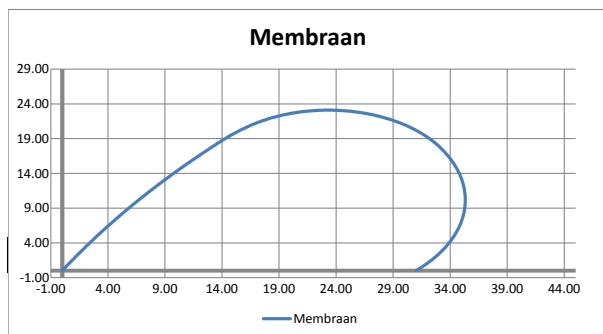
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	31.00	m
Lengte membraan	Lmin	65.10	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.57	Bar
Druk balg in meter waterkolom	H	5.71	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden		Bandbreedte	
Trek in balg		70	1000 kN
Aanvangshoek		0.15	2.5 Rad
Luchtdruk (bar)		0.01	0.6 bar

Vaste gegevens			
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschaloord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.07	m

Doek eigenschappen			
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.13	kg
		11.06	N



WATER/ LUCHT

Output

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
	Hverval	785	kN
Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	322.4	468.7	791.1 kN
RV	566.8	453.4	1'020.2 kN
Verschil verval en horizontaal klemlijn	1%	6.35	kN

Berekende waarden			
Tenstion	T	652.12	kN/m ²
		652119	N
Eind hoek	ϕB	h1	4.20 Rad
			240.4 Graden
Start hoek	ϕA	h2	60.4 Graden
			0.77 Rad
			44.0 Graden

Volumes		m ³	%
Water		496.49	84.68%
Lucht		89.81	15.32%
Totaal		586.31	

Coördinaten		X	Y
Maxima		35.32	23.10
Vast punt 1		-	-
Vast punt 2		31.00	-
Onnauwkeurigheid		-0.00	0.00
Onnauwkeurigheid		0.00	0.00
Totaal		0.04	0.04
Factor			0.55
Ybij X max		-	-
X bij Y max			23.33

Afwijkingen		m	%
Hbalg		0.90	
X		0.00	0.00%
Y		0.00	0.00%
Hverval		6.35	5.38%
Totaal			0.00%

Figure B.5.: Crest-height calculations with $B = 31.0$ m

By changing the base width to $B = 30.0$ m the resulting air pressure in the barrier becomes 0.58 bar and the tension force results to 669.4 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.58436 bar Luchtdruk
30 m Breedte tussen klemlijnen
65.1 m Lengte membraan

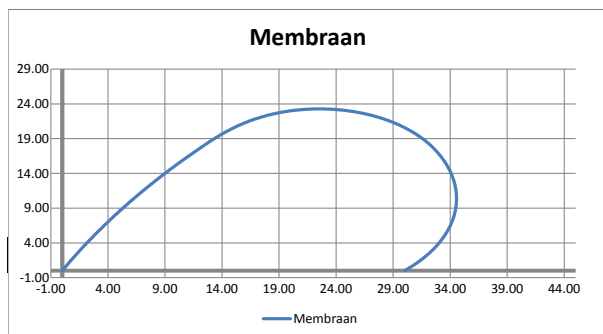
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	30.00	m
Lengte membraan	Lmin	65.10	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.58	Bar
Druk balg in meter waterkolom	H	5.84	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden		Bandbreedte	
Trek in balg		70	1000 kN
Aanvangshoek		0.15	2.5 Rad
Luchtdruk (bar)		0.01	0.6 bar

Vaste gegevens			
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschaloord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.07	m

Doek eigenschappen			
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.13	kg
		11.06	N



WATER/ LUCHT

Output

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
	Hverval	785	kN
Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	301.9	491.9	793.8 kN
RV	597.4	454.0	1'051.5 kN
Verschil verval en horizontaal klemlijn	1%	9.04	kN

Berekende waarden			
Tenstion	T	669.40	kN/m ²
		669404	N
Eind hoek	ϕB	h1	4.24 Rad
			243.2 Graden
Start hoek	ϕA	h2	63.2 Graden
			0.75 Rad
			42.7 Graden

Volumes		m ³	%
Water		493.00	84.04%
Lucht		93.63	15.96%
Totaal		586.63	

Coördinaten		X	Y
Maxima		34.56	23.28
Vast punt 1		-	-
Vast punt 2		30.00	-
Onnauwkeurigheid		-0.00	-0.00
		0.00	0.00
Onnauwkeurigheid		0.72	0.03
Totaal		0.03	
Factor			0.55
Ybij X max		-	
X bij Y max			22.52

Afwijkingen		m	%
Hbalg		0.72	
X		0.00	0.00%
Y		0.00	0.00%
Hverval		9.04	7.66%
Totaal			0.00%

Figure B.6.: Crest-height calculations with $B = 30.0$ m

B.2. Variation of circumferential membrane length

By keeping the the base width to $B = 31.0$ m and changing the circumferential length to $L = 67.0$ m the resulting air pressure in the barrier becomes 0.60 bar and the tension force results to 748.8 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.599999 bar Luchtdruk
31 m Breedte tussen klemlijnen
67 m Lengte membraan

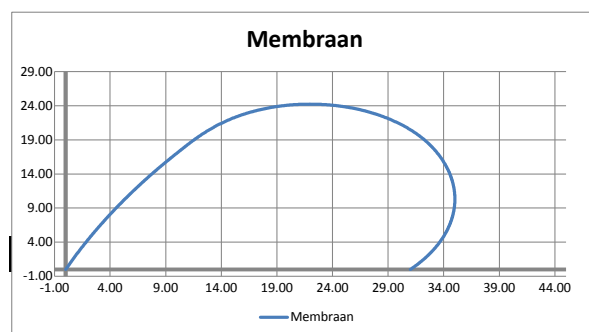
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	31.00	m
Lengte membraan	Lmin	67.00	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.60	Bar
Druk balg in meter waterkolom	H	6.00	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden		Bandbreedte
Trek in balg	70	1000 kN
Aanvangshoek	0.15	2.5 Rad
Luchtdruk (bar)	0.01	0.6 bar

Vaste gegevens		
Zwaartekracht	g	9.81 m/s ²
Hoogte onbelast	Ho	22.00 m
Kruinhoogte	Hkruin	1.00 m
Aantal delen	n	1000 st
Dichtheid water Hellevoetsluis	RH	1000 kg/m ³
Dichtheid water Goidschalxoord	RG	1000 kg/m ³
Dichtheid water	RW	1000 kg/m ³
Afmeting deel	dS	0.07 m

Doek eigenschappen		
Breedte		1.00 m
Doek dikte		16.00 mm
Eigen gewicht doek		1082.00 kg/m ³
Elasticiteit doek		5000000.00 kN/m ²
Gewicht ds		1.16 kg
		11.38 N



WATER/AIR

Output

Belastingen			
Hoogwaterzijde			2'374 kN
Laagwaterzijde			1'589 kN
		Hverval	785 kN

Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	291.9	504.2	796.2 kN
RV	689.6	553.6	1'243.2 kN
Verschil verval en horizontaal klemlijn		1%	11.36 kN

Berekende waarden			
Tenstion	T		748.82 kN/m ²
			748817 N
Eind hoek	ϕ_B	h1	4.31 Rad
			247.1 Graden
			67.1 Graden
Start hoek	ϕ_A	h2	0.83 Rad
			47.7 Graden

Volumes		m ³	%
Water		512.45	81.06%
Lucht		119.72	18.94%
Totaal		632.17	

Coördinaten		X	Y
Maxima		35.00	24.23
Vast punt 1		-	-
Vast punt 2		31.00	-
Onnauwkeurigheid		-0.00	-0.00
Onnauwkeurigheid		0.00	0.00
Onnauwkeurigheid		0.23	0.01
Totaal		0.01	
Factor			0.55
Ybij X max		-	
X bij Y max			21.97

Afwijkingen		m	%
Hbalg		0.23	
X		0.00	0.00%
Y		0.00	0.00%
Hverval		11.36	9.63%
Totaal			0.00%

	Vaste input
	Vaste gegevens
	Berekende variabelen a.h.v. solver
	Berekende gegevens

Figure B.7.: Crest-height calculations with $L = 67.0$ m

By keeping the the base width to $B = 31.0$ m and changing the circumferential length to $L = 66.0$ m the resulting air pressure in the barrier becomes 0.57 bar and the tension force results to 664.6 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.567081 bar Luchtdruk
31 m Breedte tussen klemlijnen
66 m Lengte membraan

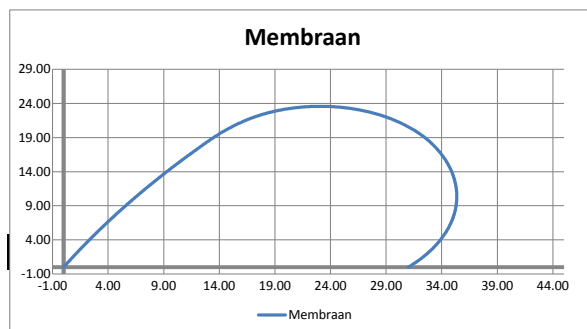
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	31.00	m
Lengte membraan	Lmin	66.00	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.57	Bar
Druk balg in meter waterkolom	H	5.67	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden		Bandbreedte	
Trek in balg	70	1000	kN
Aanvangshoek	0.15	2.5	Rad
Luchtdruk (bar)	0.01	0.6	bar

Vaste gegevens		Waarde	Grootheid
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschalkoord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.07	m

Doek eigenschappen		Waarde	Grootheid
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.14	kg
		11.21	N



WATER/AIR

Output

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
	Hverval	785	kN

Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	319.7	473.7	793.4
RV	582.7	466.2	1'048.9
Verschil verval en horizontaal klemlijn	1%		8.60

Berekende waarden			
Tension	T	664.64	kN/m ²
		664640	N
Eind hoek	ϕ_B	4.21	Rad
		241.3	Graden
Start hoek	ϕ_A	61.3	Graden
		0.78	Rad
		44.5	Graden

Volumes		m ³	%
Water		502.63	83.24%
Lucht		101.21	16.76%
Totaal		603.84	

Coördinaten		X	Y
Maxima		35.36	23.57
Vast punt 1			-
Vast punt 2		31.00	-
Onnauwkeurigheid		0.00	0.00
Onnauwkeurigheid		0.00	0.00
Totaal		0.43	0.02
Factor			0.55
Ybij X max		-	
X bij Y max			23.09

Afwijkingen		m	%
Hbalg		0.43	
X		0.00	0.00%
Y		0.00	0.00%
Hverval		8.60	7.29%
Totaal			0.00%

	Vaste input
	Vaste gegevens
	Berekende variabelen a.h.v. solver
	Berekende gegevens

Figure B.8.: Crest-height calculations with $L = 66.0$ m

By keeping the the base width to $B = 31.0$ m and changing the circumferential length to $L = 65.5$ m the resulting air pressure in the barrier becomes 0.57 bar and the tension force results to 667.6 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.574899 bar Luchtdruk
31 m Breedte tussen klemlijnen
65.5 m Lengte membraan

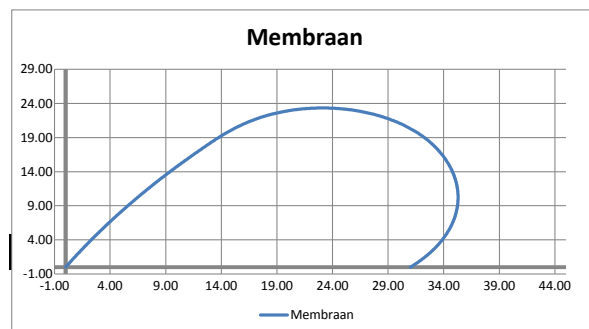
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	31.00	m
Lengte membraan	Lmin	65.50	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.57	Bar
Druk balg in meter waterkolom	H	5.75	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden		Bandbreedte	
Trek in balg		70	1000 kN
Aanvangshoek		0.15	2.5 Rad
Luchtdruk (bar)		0.01	0.6 bar

Vaste gegevens		Waarde	Grootheid
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschalkoord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.07	m

Doek eigenschappen		Waarde	Grootheid
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.13	kg
		11.12	N



WATER/AIR

Output

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
Hverval		785	kN

Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	318.6	475.2	793.8 kN
RV	586.6	468.9	1'055.5 kN
Verschil verval en horizontaal klemlijn		1%	9.03 kN

Berekende waarden			
Tension	T	667.58	kN/m ²
Eind hoek	φB	4.21	Rad
		241.5	Graden
Start hoek	φA	61.5	Graden
		0.78	Rad
		44.6	Graden

Volumes		m ³	%
Water		500.04	83.96%
Lucht		95.56	16.04%
Totaal		595.60	

Coördinaten		X	Y
Maxima		35.29	23.33
Vast punt 1			-
Vast punt 2		31.00	-
Onnauwkeurigheid		0.00	-0.00
		0.00	0.00
Onnauwkeurigheid		0.67	0.03
Totaal		0.03	
Factor			0.55
Ybij X max		-	
X bij Y max			23.10

Afwijkingen		m	%
Hbalg		0.67	
X		0.00	0.00%
Y		0.00	0.00%
Hverval		9.03	7.65%
Totaal			0.00%

	Vaste input
	Vaste gegevens
	Berekende variabelen a.h.v. solver
	Berekende gegevens

Figure B.9.: Crest-height calculations with $L = 65.5$ m

By keeping the the base width to $B = 31.0$ m and changing the circumferential length to $L = 65.0$ m the resulting air pressure in the barrier becomes 0.59 bar and the tension force results to 680.5 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

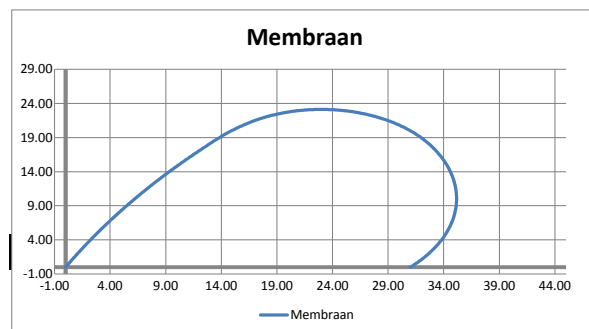
17 m Water hoogte balg
0.587795 bar Luchtdruk
31 m Breedte tussen klemlijnen
65 m Lengte membraan

Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	31.00	m
Lengte membraan	Lmin	65.00	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.59	Bar
Druk balg in meter waterkolom	H	5.88	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m
Te berekenen waarden		Bandbreedte	
Trek in balg		70	1000 kN
Aanvangshoek		0.15	2.5 Rad
Luchtdruk (bar)		0.01	0.6 bar

Vaste gegevens		Waarde	Grootheid
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschalkoord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.07	m

Doek eigenschappen		Waarde	Grootheid
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.13	kg
		11.04	N



WATER/AIR

Output

Belastingen		Waarde	Grootheid
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
	Hverval	785	kN
Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	314.6	479.5	794.1 kN
RV	603.4	482.8	1'086.2 kN
Verschil verval en horizontaal klemlijn	1%		9.33 kN

Berekende waarden		Waarde	Grootheid
Tension	T	680.47	kN/m ²
Eind hoek	ϕ_B	4.23	Rad
		242.5	Graden
Start hoek	ϕ_A	62.5	Graden
		0.79	Rad
		45.2	Graden

Volumes		m ³	%
Water		497.69	84.55%
Lucht		90.97	15.45%
Totaal		588.66	

Coördinaten		X	Y
Maxima		35.15	23.12
Vast punt 1			-
Vast punt 2		31.00	-
Onnauwkeurigheid		0.00	-0.00
		0.00	0.00
Onnauwkeurigheid		0.88	0.04
Totaal		0.04	
Factor			0.55
Ybij X max		-	
X bij Y max			23.02

Afwijkingen		m	%
Hbalg		0.88	
X		0.00	0.00%
Y		0.00	0.00%
Hverval		9.33	7.91%
Totaal			0.00%

	Vaste input
	Vaste gegevens
	Berekende variabelen a.h.v. solver
	Berekende gegevens

Figure B.10.: Crest-height calculations with $L = 65.0$ m

By keeping the the base width to $B = 31.0$ m and changing the circumferential length to $L = 64.5$ m the resulting air pressure in the barrier becomes 0.60 bar and the tension force results to 689.2 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.598628 bar Luchtdruk
31 m Breedte tussen klemlijnen
64.5 m Lengte membraan

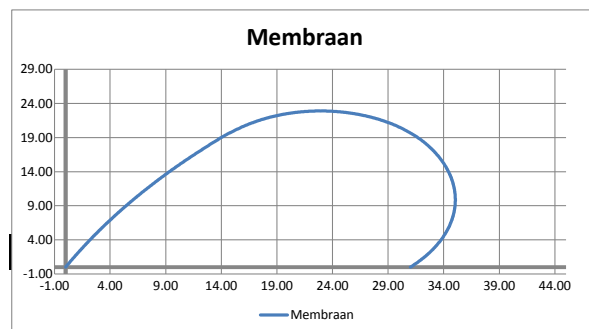
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	31.00	m
Lengte membraan	Lmin	64.50	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.60	Bar
Druk balg in meter waterkolom	H	5.99	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden		Bandbreedte	
Trek in balg		70	1000 kN
Aanvangshoek		0.15	2.5 Rad
Luchtdruk (bar)		0.01	0.6 bar

Vaste gegevens		Waarde	Grootheid
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschalkoord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.06	m

Doek eigenschappen		Waarde	Grootheid
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.12	kg
		10.95	N



WATER/AIR

Output

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
	Hverval	785	kN

Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	311.8	482.1	793.9 kN
RV	614.6	492.5	1'107.2 kN
Vershill verval en horizontal klemlijn		1%	9.06 kN

Berekende waarden			
Tension	T	689.20	kN/m ²
Eind hoek	φB	4.24	Rad
		243.1	Graden
Start hoek	φA	63.1	Graden
		0.80	Rad
		45.6	Graden

Volumes		m ³	%
Water		495.31	85.20%
Lucht		86.05	14.80%
Totaal		581.36	

Coördinaten		X	Y
Maxima		35.04	22.89
Vast punt 1			-
Vast punt 2		31.00	-
Onnauwkeurigheid		-0.00	-0.00
		0.00	0.00
Onnauwkeurigheid		1.11	0.05
Totaal		0.05	
Factor			0.55
Ybij X max		-	
X bij Y max			22.96

Afwijkingen		m	%
Hbalg		1.11	
X		0.00	0.01%
Y		0.00	0.00%
Hverval		9.06	7.68%
Totaal			0.01%

	Vaste input
	Vaste gegevens
	Berekende variabelen a.h.v. solver
	Berekende gegevens

Figure B.11.: Crest-height calculations with $L = 64.5$ m

By keeping the the base width to $B = 31.0$ m and changing the circumferential length to $L = 64.0$ m the resulting air pressure in the barrier becomes 0.59 bar and the tension force results to 691.4 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.594978 bar Luchtdruk
31 m Breedte tussen klemlijnen
64 m Lengte membraan

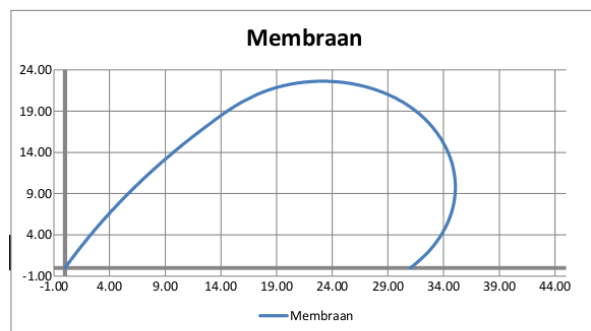
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	31.00	m
Lengte membraan	Lmin	64.00	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.59	Bar
Druk balg in meter waterkolom	H	5.95	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden		Bandbreedte	
Trek in balg		70	1000 kN
Aanvangshoek		0.15	2.5 Rad
Luchtdruk (bar)		0.01	0.6 bar

Vaste gegevens		Waarde	Grootheid
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschalkoord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.06	m

Doek eigenschappen		Waarde	Grootheid
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.11	kg
		10.87	N



WATER/AIR

Output

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
	Hverval	785	kN

Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	315.7	474.4	790.1 kN
RV	594.4	477.4	1'071.7 kN
Verschuif verval en horizontaal klemlijn		1%	5.26 kN

Berekende waarden			
Tension	T	691.39	kN/m ²
Eind hoek	φB	4.22	Rad
		242.0	Graden
Start hoek	φA	62.0	Graden
		0.79	Rad
		45.2	Graden

Volumes		m ³	%
Water		490.99	86.04%
Lucht		79.68	13.96%
Totaal		570.67	

Coördinaten		X	Y
Maxima		35.05	22.62
Vast punt 1			-
Vast punt 2		31.00	-
Onnauwkeurigheid		0.00	-0.00
		0.00	0.00
Onnauwkeurigheid		1.38	0.06
Totaal		0.06	
Factor			0.55
Ybij X max		-	
X bij Y max			23.15

Afwijkingen		m	%
Hbalg		1.38	
X		0.00	0.00%
Y		0.00	0.00%
Hverval		5.26	4.46%
Totaal			0.00%

	Vaste input
	Vaste gegevens
	Berekende variabelen a.h.v. solver
	Berekende gegevens

Figure B.12.: Crest-height calculations with $L = 64.0$ m

By keeping the the base width to $B = 31.0$ m and changing the circumferential length to $L = 63.5$ m the resulting air pressure in the barrier becomes 0.60 bar and the tension force results to 694.1 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.598461 bar Luchtdruk
31 m Breedte tussen klemlijnen
63.5 m Lengte membraan

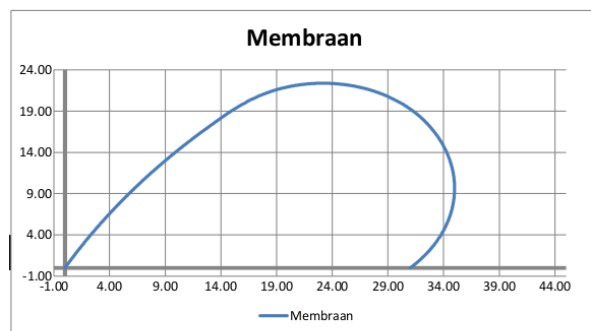
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	31.00	m
Lengte membraan	Lmin	63.50	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.60	Bar
Druk balg in meter waterkolom	H	5.98	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden		Bandbreedte	
Trek in balg		70	1000 kN
Aanvangshoek		0.15	2.5 Rad
Luchtdruk (bar)		0.01	0.6 bar

Vaste gegevens		Waarde	Grootheid
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goidschaloord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.06	m

Doek eigenschappen		Waarde	Grootheid
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.10	kg
		10.78	N



WATER/AIR

Output

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
	Hverval	785	kN

Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	316.1	471.0	787.1 kN
RV	590.5	476.2	1'066.7 kN
Verschuif verval en horizontaal klemlijn		0%	2.32 kN

Berekende waarden			
Tension	T	694.13	kN/m ²
		669788	N
Eind hoek	ϕ_B	4.22	Rad
		241.8	Graden
Start hoek	ϕ_A	61.8	Graden
		0.79	Rad
		45.3	Graden

Volumes		m ³	%
Water		487.63	86.77%
Lucht		74.36	13.23%
Totaal		561.99	

Coördinaten		X	Y
Maxima		34.98	22.38
Vast punt 1			-
Vast punt 2		31.00	-
Onnauwkeurigheid		-0.01	0.00
		0.01	0.00
Onnauwkeurigheid		1.62	0.07
Totaal		0.07	
Factor			0.55
Ybij X max		-	
X bij Y max			23.16

Afwijkingen		m	%
Hbalg		1.62	
X		0.01	0.03%
Y		0.00	0.00%
Hverval		2.32	1.97%
Totaal			0.03%

Vaste input
 Vaste gegevens
 Berekende variabelen a.h.v. solver
 Berekende gegevens

Figure B.13.: Crest-height calculations with $L = 63.5$ m

B.3. Total optimization

By varying the circumferential length L , the base width B , the internal air pressure and the tension force an optimized system can be found. The Base width becomes $B = 30.58$ m, the circumferential length $L = 65.90$ m and the internal air pressure becomes 0.56 bar. With these parameters the resulting tension force in the membrane is 651.5 kN/m'.

Met drukregulering, $\Delta F_{\text{horizontaal}}$ minimaal

17 m Water hoogte balg
0.563028 bar Luchtdruk
30.58 m Breedte tussen klemlijnen
65.9 m Lengte membraan

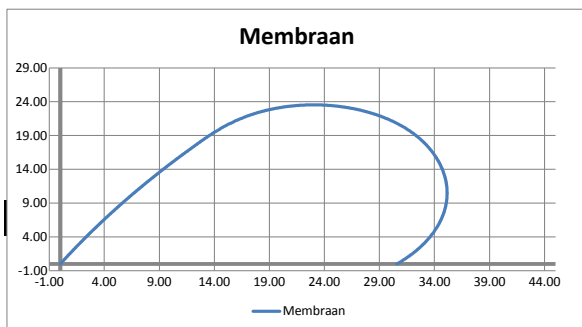
Input

Vaste gegevens		Waarde	Grootheid
Breedte tussen klemlijnen	Bb	30.58	m
Lengte membraan	Lmin	65.90	m
Water hoogte balg	Hin	17.00	m
Luchtdruk (bar)	dP	0.56	Bar
Druk balg in meter waterkolom	H	5.63	m
Hoogte van de balg	Hbalg	24.00	m
Hoog water zijde	HW	22.00	m
Laag water zijde	LW	18.00	m

Te berekenen waarden	Bandbreedte		
Trek in balg	70	1000	kN
Aanvangshoek	0.15	2.5	Rad
Luchtdruk (bar)	0.01	0.6	bar
Breedte tussen klemlijnen	30	40	m
Lengte membraan	50	80	m

Vaste gegevens		Waarde	Grootheid
Zwaartekracht	g	9.81	m/s ²
Hoogte onbelast	Ho	22.00	m
Kruinhoogte	Hkruin	1.00	m
Aantal delen	n	1000	st
Dichtheid water Hellevoetsluis	RH	1000	kg/m ³
Dichtheid water Goudschalkoord	RG	1000	kg/m ³
Dichtheid water	RW	1000	kg/m ³
Afmeting deel	dS	0.07	m

Doek eigenschappen		Waarde	Grootheid
Breedte		1.00	m
Doek dikte		16.00	mm
Eigen gewicht doek		1082.00	kg/m ³
Elasticiteit doek		5000000.00	kN/m ²
Gewicht ds		1.14	kg
		11.19	N



WATER/AIR

Output

Belastingen			
Hoogwaterzijde		2'374	kN
Laagwaterzijde		1'589	kN
	Hverval	785	kN

Belasting per klemlijn op basis van trek			
	Punt B	Punt A	totaal
RH	317.0	475.8	792.8 kN
RV	569.1	445.0	1'014.1 kN
Verschil verval en horizontaal klemlijn		1%	8.02 kN

Berekende waarden			
Tension	T	651.47	kN/m ²
		651470	N
Eind hoek	φB	h1	4.20 Rad
			240.9 Graden
Start hoek	φA	h2	60.9 Graden
			0.75 Rad
			43.1 Graden

Volumes		m ³	%
Water		499.52	83.35%
Lucht		99.76	16.65%
Totaal		599.27	

Coördinaten	X	Y
Maxima	35.15	23.54
Vast punt 1	-	-
Vast punt 2	30.58	-
Onnauwkeurigheid	0.00	0.00
Onnauwkeurigheid	0.46	0.02
Totaal	0.02	
Factor		0.55
Ybij X max	-	-
X bij Y max	-	23.00

Afwijkingen	m	%
Hbalg	0.46	
X	0.00	0.00%
Y	0.00	0.00%
Hverval	8.02	6.80%
Totaal		0.00%

	Vaste input
	Vaste gegevens
	Berekende variabelen a.h.v. solver
	Berekende gegevens

Figure B.14.: Crest-height calculations with all optimized parameters