

## Critical assessment of methodologies for operations and safety evaluations of freeway turbulence

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1 **A CRITICAL ASSESSMENT OF METHODOLOGIES FOR OPERATIONS AND**  
2 **SAFETY EVALUATIONS OF FREEWAY TURBULENCE**

3  
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## 1 **ABSTRACT**

2 Turbulence in traffic is a commonly known phenomenon, but the exact characteristics of this  
3 phenomenon are not yet clear. It reflects individual changes in speed, headways, and lanes in  
4 the traffic stream. The currently used freeway design guidelines prescribe different measures  
5 for handling turbulence, such as sufficient ramp spacing, and spacing between road disconti-  
6 nuousities. In situations where the available space between discontinuities is scarce, it might be  
7 necessary to make a trade-off between costs and safety/operation. For a valid trade off more  
8 insight is needed on the safety and operations effects when one deviates from the guidelines.  
9 A lot of research was done on the different causes of turbulence and their effect on safety and  
10 operation. This paper proposes a theoretical framework for turbulence phenomenon that fa-  
11 cilitates the comparison of the available methodologies that can be used to evaluate a freeway  
12 design on the matter of turbulence and its impact on traffic operations and safety. The main  
13 finding of this review is that the currently available methodologies lack the ability to evaluate  
14 the impact of freeway turbulence on operations and safety simultaneously. Different recom-  
15 mendations to overcome limitations of current methodologies and further research possibili-  
16 ties to improve these methodologies are given.

17  
18 *Keywords:* traffic safety; operations; turbulence; surrogate safety measures; freeway design

## 19 **INTRODUCTION**

20  
21 Entering and exiting traffic from ramps and weaving areas will affect the traffic density on the  
22 freeway. Especially on the right lane. This change in density may cause freeway traffic to re-  
23 act, for example: changing lanes to a lane with a lower traffic density. Other reactions can be  
24 decelerating or accelerating in order to increase or decrease the headway with the vehicle in  
25 front (1). This phenomena is called 'turbulence' and it is mentioned several times in literature  
26 (2-6) and in guidelines (1; 7; 8).

27  
28 The concept of turbulence is used consistently and this suggests a clear definition of turbu-  
29 lence. But neither the existing guidelines nor the literature define exactly what turbulence is.  
30 There is however a general agreement in literature on two main characteristics regarding tur-  
31 bulence: turbulence is a common phenomenon in a traffic stream (1), and will have a higher  
32 magnitude around freeway discontinuities, such as on-ramps (4), off-ramps, weaving areas,  
33 left side lane reductions, etc.. Also turbulence is stated to have a negative impact on traffic  
34 safety and traffic operations (1-3; 6).

35  
36 According to design guidelines turbulence has to be taken into account for ramp spacing (1;  
37 7; 9) and the spacing of discontinuities (8; 10). To do this guidelines prescribe certain lengths,  
38 but the scientific justification is lacking. The AASHTO for example uses a set of values for  
39 minimum ramps terminals spacing (7). The Dutch freeway guidelines (10) prescribe turbu-  
40 lence lengths for the spacing of discontinuities. In none of the guidelines the origin of the pre-  
41 scribed lengths is referenced.

42  
43 In densely populated areas, such as the Netherlands, the space for new freeways is scarce. In  
44 some freeway design cases it was decided to deviate from the guidelines in order to be able to  
45 realize the desired interchange connections. In such cases it is tempting to accept a shorter  
46 length than prescribed. However, the implications for traffic safety and operations of deviat-  
47 ing from the guidelines are not fully understood. A thorough understanding of turbulence, and  
48 its influence on traffic safety and traffic operations is critical in order to be able to make the  
49 right trade-off for the design choices in these situations.

50

1 The main aim of this paper is to review the currently available methodologies to assess the  
 2 impact of turbulence in freeway traffic on traffic safety and traffic operations. The different  
 3 methodologies are described and compared. Recommendations are given on how to use a  
 4 wide range of different existing methods, and how to combine methods when assessing de-  
 5 signs on operations and safety at the same time. The main focus of this review is turbulence in  
 6 freeway traffic around on-ramps, off-ramps and weaving areas.

7  
 8 This review starts with a background on the turbulence phenomenon and its influence on traf-  
 9 fic safety and traffic operations. The second part gives an overview of the available methods  
 10 to quantify turbulence. The available methodologies for assessing the impact of turbulence on  
 11 operations and safety are described and compared in the third and fourth part. This review  
 12 ends with conclusions and gives recommendation for further research.

## 13 BACKGROUND

14 In freeway design the use of guidelines, manuals and standards in the design process is com-  
 15 mon. Documents such as the Highway Capacity Manual (HCM) and the ‘AASHTO Green  
 16 Book’ in the USA, (7), the ‘Richtlinien für die Anlage von Autobahnen (RAA)’ (9) in Ger-  
 17 many, the ‘Design Manual for Roads and Bridges (8)’ in Great Britain and ‘Nieuwe On-  
 18 twerprijchlijnen voor Autosnelwegen (NOA)’(new design guidelines for freeways) (10) in  
 19 The Netherlands are prescribed in order to maintain consistency in road geometry and to pro-  
 20 vide safe freeways with sufficient level of service (11).

21  
 22 One of the important geometric elements in freeways is ramp spacing and the length of weav-  
 23 ing areas. The basic principle in the design of these elements is that there should be sufficient  
 24 spacing between succeeding ramps in order to cope with turbulence in the traffic stream.

25 Different approaches for dealing with turbulence are used in the different guidelines. For ex-  
 26 ample: the AASHTO Green Book uses a set of minimum values for ramp spacing and the  
 27 Dutch guidelines use a criteria called Turbulence length, which is the required length between  
 28 succeeding discontinuities. The prescribed lengths differ per type of discontinuity and also per  
 29 guideline. For example, table 1 shows the different prescribed distances between an on-ramp  
 30 followed by an off-ramp (measured from nose to nose).

31 **TABLE 1 Distance between On-Ramp and Off-Ramp Prescribed in Different Guidelines**

Country	Distance	Design criteria
The Netherlands (10)	750 m	design speed
Germany (9)	1100 m*	minimum value for isolated intersection planning
USA (7)	600 m**	road category: freeway
	480 m***	road category: freeway
UK (8), Vol.6, Sec. 2, Cpt 4.7	450 m****	3.75V, where V = design speed = 120 km/h

33 \* 250 m acceleration lane + 600 m between acceleration and deceleration lane + 250 m deceleration lane

34 \*\* system to service interchange (weaving)

35 \*\*\* service to service interchange (weaving)

36 \*\*\*\* may be increased to the minimum requirements for effective signing and motorway signaling

37  
 38 Despite the differences between the different approaches, the general concept behind ramp  
 39 spacing and weaving areas in all the above guidelines is that the traffic stream will encounter  
 40 a raised level of turbulence around freeway discontinuities. Turbulence will intensify when  
 41 the available road length for lane changing becomes shorter. This should be taken into ac-  
 42 count by applying sufficient ramp spacing. This concept is supported by literature (12; 13).

43

1 In literature and guidelines turbulence is mentioned but no explicit definition for turbulence is  
2 given. These are some examples in which turbulence is mentioned:

- 3 - “Weaving segments require intense lane-changing maneuvers as drivers must access lanes  
4 appropriate to their desired exit leg. Therefore, traffic in a weaving segment is subject to  
5 lane-changing turbulence in excess of that normally present on basic freeway segments.  
6 This additional turbulence presents operational problems and design requirements” (1);
- 7 - “Ramp-freeway junctions create turbulence in the merging or diverging traffic stream. In  
8 general, the turbulence is the result of high lane-changing rates. The action of individual  
9 merging vehicles entering the traffic stream creates turbulence in the vicinity of the ramp.  
10 Approaching freeway vehicles move toward the left to avoid the turbulence. Thus, the  
11 ramp influence area experiences a higher rate of lane-changing than is normally present on  
12 ramp-free portions of freeway” (1);
- 13 - turbulence can be captured by four variables:” (1) variation in speeds in the left and interi-  
14 or lanes, (2) variation in speed in the right lane, (3) variation in flow in the left and interior  
15 lanes, and (4) variation in flow in the right lane” (3).
- 16 - “Turbulence is (among other things) defined by headway changes and a changed distribu-  
17 tion of traffic over the different freeway lanes. Corresponding aspects of driving behavior  
18 are for example deceleration, evasive actions or (anticipating) lane changes” (10).

19  
20 Since there is no explicit definition for turbulence available, a definition is still to be suggest-  
21 ed. A non-turbulent traffic state can be considered as a state in which all vehicles on a road  
22 maintain the same relative distance and speed to others over a certain length of a road section  
23 and for a period of time. A turbulent traffic state can then be considered as the state in which  
24 speed, headway and the lateral position change over time, due to driver actions such as accel-  
25 eration, deceleration and lane-change. Since acceleration, deceleration and lane-changes are  
26 common driver actions, turbulence can be considered as always present in the traffic stream  
27 (1). Therefore, a more specific definition of turbulence in the vicinity of discontinuities (such  
28 as ramps) is proposed in this paper as following:

- 29 - Turbulence:
  - 30 · individual changes in speed, headways, and lanes (i.e. lane-changes) in a certain road  
31 segment, regardless the cause of the change;
- 32 - Level of Turbulence:
  - 33 · the frequency and intensity of individual changes in speed, headways and lane-  
34 changes in a certain road segment, over a certain period of time;

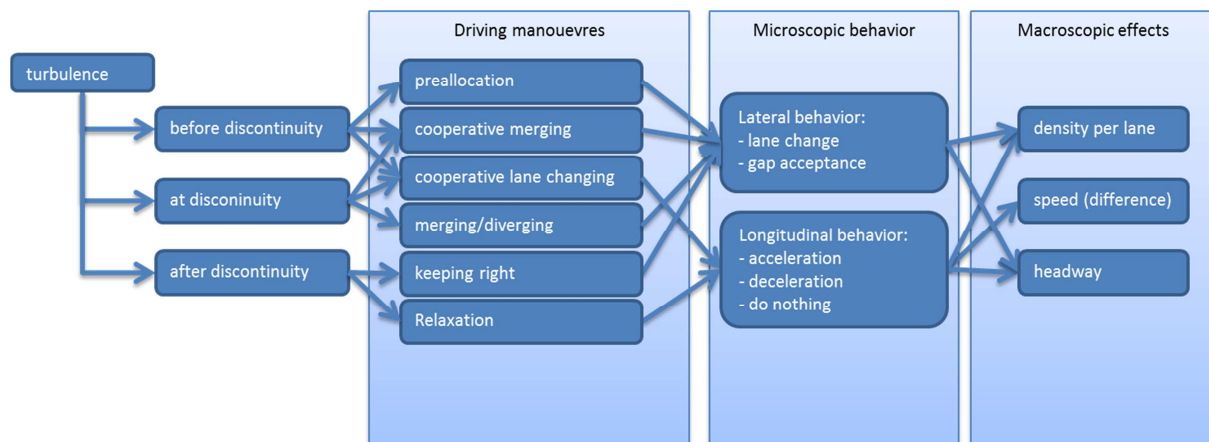
### 36 **Theoretical Structure for Turbulence**

37 The *Level of Turbulence* is expected to increase before (upstream of) and to decrease after  
38 (downstream of) a ramp or a weaving area. This phenomena is described by Hovenga (14)  
39 who found that turbulence starts more or less about 500 meter upstream and ends more or less  
40 about 800 downstream from an on-ramp nose. Kondyli and Elefteriadou (5) found that turbu-  
41 lence due to merging maneuvers initiates 110 m upstream of the nose. According to the HCM  
42 (1) the merge influence area will occur about 460 m (1.500 ft.) upstream and 460 m down-  
43 stream of the nose. To the best of our knowledge other literature that describes the start or the  
44 end of a raised level of turbulent traffic is not available.

45  
46 Based on this concept a raised level of turbulence is for this study divided in three parts:

- 47 1) Upstream of (before) the ramp;
- 48 2) At the ramp;
- 49 3) Downstream of (after) the ramp.

1 At ramps and weaving areas drivers will execute their strategic route navigation decisions,  
 2 which will lead to mandatory lane changes, in order to be able to enter or exit the freeway  
 3 (15). These lane changes make other drivers react (4), which results in turbulent traffic (1).  
 4 Figure 1 depicts the proposed structure for turbulence with consideration of the three parts.  
 5



6  
 7 **FIGURE 1 Theoretical structure for turbulence.**  
 8

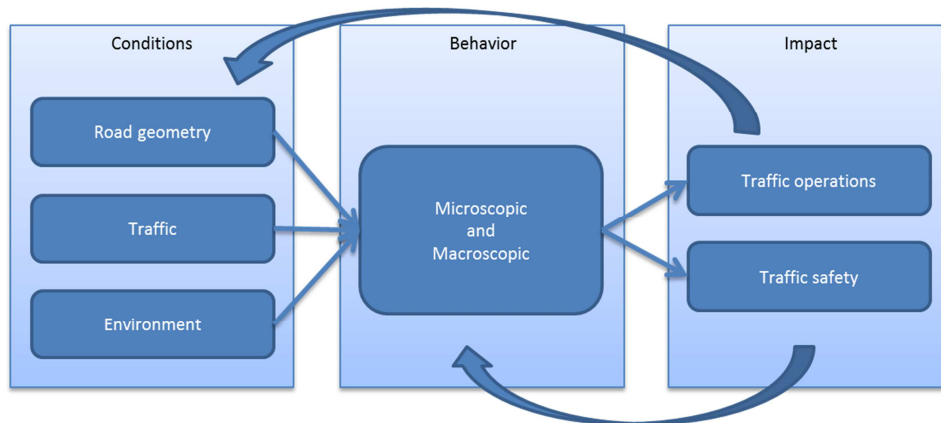
9 Lane changes upstream of a ramp are considered to be pre-allocating behavior, where the  
 10 driver chooses a lane in a tactical sense before the ramp, or cooperative behavior. Cooperative  
 11 merging is behavior where an on-ramp a driver chooses to change lanes to the left to give way  
 12 to the entering traffic (5; 16) or decelerate in order to enlarge the headway with the vehicle in  
 13 front after a new vehicle has merged in (17). Or drivers might increase their headway to give  
 14 way to entering traffic. This phenomena is called a cooperative lane change (17; 18) or cour-  
 15 tesy yielding (19). Downstream of a ramp lane changes may occur due to the right side rule,  
 16 which prescribes that drivers should change lanes to the right when possible. Downstream of  
 17 a ramp drivers might decelerate to increase the headway to their leading vehicle. This phe-  
 18 nomena is called relaxation (18; 20).  
 19

20 The different manoeuvres can be clustered in different types of microscopic behavior: lateral  
 21 or longitudinal. The first considered lateral behavior is lane change, which can be classified as  
 22 free, forced or cooperative (17). Lane changing and merging are closely related to gap ac-  
 23 ceptance and tactical lane choice. These can be considered as integrated behavior (21). Longi-  
 24 tudinal behavior is classified as acceleration, deceleration, or do-nothing (22). Lateral and  
 25 longitudinal behavior can be integrated in order to get a complete description of merging be-  
 26 havior (23; 24).  
 27

28 Microscopic behavior results in macroscopic effects. For example a lane changes will result in  
 29 a changed density per lane and a changed headway distribution. Acceleration and deceleration  
 30 might also result in a changed headway distribution, but result also in changing speed differ-  
 31 ences between different vehicles as illustrated in figure 1.  
 32

### 33 **Impact of turbulence**

34 The general hypothesis for the research on turbulence is that the level of turbulence is affected  
 35 by certain conditions, such as road design, traffic characteristics (1), environmental aspects  
 36 (such as weather and daylight), and drivers' population characteristics. These conditions af-  
 37 fect driving behavior. The resulting manoeuvres drivers take affect traffic safety and opera-  
 38 tions (1-3; 6). This principle is shown graphically in figure 2.  
 39



1  
2 **FIGURE 2** General concept of the effects of turbulence.  
3

4 Figure 2 shows that certain conditions (road design, traffic and environment) affect (micro-  
5 scopic and macroscopic characteristics reflecting results of) driver behavior (such as the  
6 choice of driving speed, headway, gap acceptance) which in turn effects the freeway opera-  
7 tions and safety. In reverse, some effects may influence driving behavior. For example, if the  
8 traffic stream becomes more turbulent, drivers may tend to drive more cautiously and lower  
9 their driving speeds. At the same time a low level of safety and operations might move the re-  
10 spective authorities to invest in improving the freeways' infrastructure by reconstructing some  
11 geometric design elements or adopt some new traffic management measures.  
12

### 13 **Problem definition**

14 It is clear that turbulence is a complex phenomenon with different causes and impacts. To the  
15 best of our knowledge no literature is available which assesses all the causes of turbulence,  
16 the influence of road design and traffic conditions on turbulence and its impact on operations  
17 and safety. Therefore, it is argued that there is a lack of knowledge with respect to under-  
18 standing the interaction of the causes and their impact on turbulence. This creates a twofold  
19 problem: (1) It is unknown if the current design guidelines lead to an optimal design; (2) It is  
20 unknown what the implication of deviating from the guidelines is and what impact this has on  
21 safety and operation.  
22

23 Therefore, there is a need for a method to assess the (expected) level of turbulence for a de-  
24 sign (only existing on paper), or an existing situation, and to evaluate the implications of de-  
25 sign decisions on traffic safety and traffic operations. This method should take into account  
26 both the geometrical road design elements as well as the traffic and driver behavioral ele-  
27 ments.  
28

### 29 **METHODOLOGIES TO COLLECT DATA RELATED TO TURBULENCE**

30 This section is dedicated to the different methods to collect data that could be used to quantify  
31 turbulence in freeway traffic. We will consider loop detectors, video cameras, driving simula-  
32 tor and instrumented vehicles.  
33

#### 34 **Loop detectors**

35 Macroscopic traffic state variables such as density, speed and headway distributions can be  
36 measured using loop detectors (25; 26). Loop detector data represents vehicle passages and,  
37 depending on the type of loop detector, information such as speed and vehicle length. The da-  
38 ta is usually aggregated to a fixed time period. Examples of chosen time periods are 30 sec-  
39 onds (27-30), 1 min (14; 31). The advantage of using loop detector data is its accessibility.  
40 Loop detector data from Dutch freeway for example can be accessed real time online. The

1 disadvantage of using loop detector data is that detailed data of individual manoeuvres, such  
2 as lane change, acceleration and deceleration, cannot be collected.  
3

#### 4 **Video cameras**

5 Video footage can be used to generate trajectory data, which gives detailed time/space infor-  
6 mation of individual vehicles. From this data turbulence related driver maneuvers such as  
7 merging, overtaking and acceleration can be studied in a detailed way. Three examples of  
8 studies on Dutch freeways are given. Daamen, Loot and Hoogendoorn (19) studied merging  
9 behavior at two Dutch on-ramps and compared the empirical results to applied theories in ex-  
10 isting microscopic simulation models. They found that that gap acceptance theories using a  
11 certain critical gap are not able to represent the observed behavior. Hoogendoorn,  
12 Hoogendoorn and Daamen (32) used the same data as (19) to propose a new approach to  
13 model and simulate car-following behavior. Marczak, Daamen and Buisson (33) combined  
14 the Dutch data with data from Grenoble (France) to study gap acceptance. They observed dif-  
15 ferences in the driver's behavior on the two locations: the merging drivers in Grenoble  
16 (France) tend to be more aggressive, i.e. accepting smaller gaps than in Bodegraven (Nether-  
17 lands).  
18

19 Cameras can be mounted on a high observation point such as a helicopter (34), a drone (35) or  
20 a building/structure (36).

21 The advantage of trajectory data is that it gives insight in the actual movements of vehicles.  
22 But it doesn't give any information about the underlying psychological driver behavior, it is  
23 relatively expensive to collect, and the data processing is time consuming. Thus, most studies  
24 that used trajectory data included limited number of sites.  
25

#### 26 **Driving simulators**

27 A driving simulator consists of a vehicle mock-up with a functional steering wheel, indica-  
28 tors, pedals and a shift stick. The simulator attempts to emulate a real driving environment.  
29 Behavioral aspects can be researched using data from a driving simulator. Two examples are  
30 given of freeway turbulence related studies. Winsum and Heino (37) studied time-headway  
31 during car-following and braking response. De Waard, Dijksterhuis and Brookhuis (38) stud-  
32 ied the impact of proportion of HGVs, length of the acceleration lane and the speed of the  
33 driver ahead on the workload of elderly drivers, and the benefits of in-car support systems,  
34 when merging into freeway traffic.  
35

36 The driving simulator has several advantages: the ability to test a wide variety of different ex-  
37 isting and non-existing road design layouts, control of the intervening variables and it is a safe  
38 environment. One of the disadvantages of driving simulators is that its measurements are tak-  
39 en from a simulated environment and does not reflect drivers' behavior exactly as in reality,  
40 since drivers do not face a real risk of a collision which might bias the observed behavior  
41 (39). There is therefore a need to validate the results from the simulator with real life data.  
42 Furthermore, the other vehicles designed in a driving scenario although designed to behave  
43 "intelligently" do not represent real behavior of humans.  
44

#### 45 **Instrumented vehicle and naturalistic driving**

46 Driver behavior data from a real life traffic environment can be acquired by the use of an in-  
47 strumented vehicle. An instrumented vehicle is equipped with sensors and radars that can rec-  
48 ord data relevant to the vehicle itself and also relative speeds and distances from other vehi-  
49 cles (40). All the behavioral aspects of the driver, such as driving speed, acceleration, deceler-  
50 ation, steering action, longitudinal and lateral position, can be measured comparable to the



1 driver simulator. Such a vehicle was assembled and used by TRG Southampton (41; 42) for  
2 studying car following on UK motorways. Another study is conducted in Germany, where tra-  
3 jectory data from a radar equipped vehicle was used to calibrate car following models (43).  
4 A drawback of using an instrumented vehicle is the experimental and non-naturalistic setting  
5 in which the data is gathered. This might have an effect on the behavior of the participants  
6 and as a result bias the data.

7  
8 As opposed to the experimental approach using an instrumented vehicle, naturalistic driving  
9 can be measured by drivers who operate daily using their own vehicles that have been  
10 equipped with specialized sensors, and recording equipment. Drivers operate their vehicle  
11 during normal driving routines while data is collected continuously. Olson et al. (44) and  
12 Blanco et al. (45) studied driver distraction in commercial motor vehicle operations and the  
13 impact of time-on-task on the risk of safety-critical events in the '100-car Naturalistic Driving  
14 Study'. Chong et al. (46) used data from naturalistic driving to propose a model to simulate  
15 driver behavior in terms of longitudinal and lateral actions in two driving situations, namely  
16 car-following situation and safety critical events. Another example is the Strategic Highway  
17 Research Program (SHRP2) Naturalistic Driving Study (NDS) project (47). The NDS data-  
18 base contains comprehensive video and vehicle sensor data collected from drivers and their  
19 vehicles over a three year period in six locations across the United States. The database con-  
20 tains data from 5.4 million trips taken by 3,147 volunteer drivers for between 4 and 24 months  
21 each nearly 50 million miles of driving (48). The advantage of naturalistic driving is that the  
22 resulting data is reliable and comes in large quantities. The disadvantage is that vehicles need  
23 to be equipped and operated. This requires a relatively big organizational effort. However, the  
24 rapid advancement in sensing and communication technologies is expected to facilitate these  
25 studies in the future.

## 26 **METHODOLOGIES TO ASSESS TRAFFIC OPERATIONS**

28 Turbulent traffic has a negative effect on road capacity (1). Numerous studies, for example  
29 (26; 49-51), have focused on explaining the mechanisms of driving in turbulent traffic and are  
30 based on traffic data such as loop detector data and individual vehicle trajectories. Traffic  
31 flow theories are derived from traffic data. These theories are used to describe traffic behavior  
32 in a mathematical sense by developing models. These models try to emulate the lateral and  
33 longitudinal behavior of drivers. A review of the lateral behavior models (lane change and gap  
34 acceptance) was made by (52), while a review on longitudinal behavior models was made by  
35 (53). Integrated models were also developed where lateral and longitudinal models are com-  
36 bined (24). These models can be used in microscopic simulation models, which simulate driv-  
37 ing behavior for certain situations.

38 Following is a summary of the two most common methodologies for analyzing the impact of  
39 turbulence on traffic operations: (ex-post) data evaluation and (ex-ante) Microscopic simula-  
40 tion models.

### 41 **Traffic data evaluation**

42 The most direct way to study traffic operations is by studying traffic data. Several examples  
43 of studies are available in the literature. Coifman, Krishnamurthy and Wang (54) used trajec-  
44 tory data to study the impact of lane change manoeuvres on congestion. Laval and Leclercq  
45 (55) used trajectory data collected from a freeway to study driver behavior to explain the for-  
46 mation and propagation of stop-and-go waves in congested freeway traffic. They found that  
47 difference in driving behavior, ranging from aggressive to timid, seems a more appropriate  
48 cause for traffic oscillations than seeking lane change opportunities or acceleration and decel-  
49

1 eration characteristics. This conclusion is also found in a follow-up study in which more tra-  
2 jectory data from multiple locations is used (56).

3 Zheng et al. (57) found that lane changing is a possible trigger for the deceleration waves in  
4 traffic at bottlenecks. They applied the Wavelet Transform method on Next Generation Simu-  
5 lation (NGSIM) empirical trajectory data. In a follow-up study in which the same method was  
6 used on a larger trajectory dataset, comparable conclusions were drawn (58).

7 Treiber, Hennecke and Helbing (26) used loop detector data from multiple German freeways  
8 to study congestion characteristics. Their data suggests that the congested states depend not  
9 only on the traffic situation but also on the specific infrastructure. Coifman, Krishnamurthy  
10 and Wang (54) used loop detector data to study traffic flow characteristics at bottle necks. In  
11 this study and a follow-up study (51) they found that the road capacity downstream of a bot-  
12 tleneck is reduced due to lane changing traffic.

### 14 **Microscopic simulation models**

15 The HCM suggests that traffic simulation can be used to assess the traffic operations perfor-  
16 mance of roads (1). A few examples of micro simulation software packages mentioned in lit-  
17 erature are: CORSIM (59), VISSIM (60), PARAMICS (61), AIMSUN (62), ARTEMiS (17),  
18 TRITONE(63) and FOSIM (64). FOSIM is the prescribed microsimulation package for free-  
19 way assessments in The Netherlands.

20  
21 The use of microscopic simulation software for evaluating a design is part of the regular free-  
22 way design process. Most of these applications do not result in scientific papers. However  
23 some examples of design evaluations, related to freeways, are found in the literature. Garber  
24 and Fontaine (65) used CORSIM to evaluate the performance of different interchange types  
25 under different magnitudes of traffic. Based on these results guidelines for intersections were  
26 developed. Wang, Hadiuzzaman and Qiu (66) used VISSIM for estimating the capacity of a  
27 weaving segment. They calibrated VISSIM with a capacity accuracy of about 90%, using 5  
28 minute aggregated data recorded by videos and data from loop detectors. Martínez, Garcia  
29 and Moreno (67) used VISSIM to elaborate recommendations about the best freeway exit  
30 ramp layout. They calibrated VISSIM for speed distributions gained from video recordings.  
31 Sharma and Chatterjee (68) used VISSIM to compare two alternative interchange designs: di-  
32 verging diamond and conventional diamond interchange to help in providing guidelines to the  
33 decision makers for selecting the best alternative.

34  
35 In the above mentioned studies microscopic simulation programs have proven to be powerful  
36 methodologies for assessing and comparing different designs on the matter of operations. Es-  
37 pecially macroscopic features are captured well. Current microscopic simulation programs  
38 however are not suitable for studying microscopic behavior and the effect of more detailed  
39 road geometry aspects, such as alignment, shoulders and super elevation. Most – if not all –  
40 microscopic models have problems in terms of their predictive validity. Research has shown  
41 that microscopic behavior, such as gap acceptance, is not simulated accurately (19). It is how-  
42 ever possible to calibrate a program, but even after calibration the results may vary up to 10%  
43 from measured data (66).

### 45 **METHODOLOGIES TO ASSESS TRAFFIC SAFETY**

46 In recent years a lot of research was done to gain more understanding about the factors that af-  
47 fect traffic safety by combining traffic flow characteristics, road characteristics and crash sta-  
48 tistics. This has resulted in multiple methodologies that can be used to assess traffic safety.

49  
50

## 1 **Crash prediction models**

2 Crash prediction models are used to study the factors that affect the number of crashes occur-  
3 ring on a specific (stretch of) road over some specified time period (week, month, year, num-  
4 ber of years). In general, the most basic crash data consist of crash location; date and time;  
5 crash severity; collision type; and basic information about the roadway, vehicles, and people  
6 involved. The HSM (69) Part C provides detailed steps for applying a predictive method for  
7 estimating expected average crash frequency of a network, facility, or individual site. The  
8 types of roads considered range from suburban arterials to rural multilane highways.

9  
10 The use of crash statistics has a number of drawbacks: 1) only available for existing roads and  
11 existing situations (61); 2) crash data are not always sufficient due to small sample sizes lead-  
12 ing to inconclusive results, and the lack of details to improve our understanding of crash fail-  
13 ure mechanism and especially the driver crash avoidance behavior (70; 71); 3) accidents are  
14 rare events, making it troublesome to base traffic safety analyses at individual sites on acci-  
15 dents only (71); 4) not all crashes are reported and the level of underreporting depends on the  
16 accident's severity and types of road users involved (71-73) and 5) the lack of details to im-  
17 prove our understanding of crash failure mechanism and especially the driver crash avoidance  
18 behavior (70).

19  
20 For the description of the relationship between different elements many different types of  
21 models were developed (69). A good overview on the available models, used data and their  
22 advantages and disadvantages is made by Lord and Mannering (74). Different data issues are  
23 mentioned. For a detailed description of these data issues and the modelling methods which  
24 were developed the reader is referred to the overview by Lord and Mannering (74).

25  
26 All models are developed using crash statistics and traffic volumes, but the use of detailed  
27 traffic data and road geometry data depends on the focus of the research. There are some ex-  
28 amples of studies that focus on estimating safety around freeway ramps and interchanges (12;  
29 13; 75). The use of large datasets with many aspects makes it possible to examine the rela-  
30 tionships between many different variables. For example Garber and Ehrhart (76) examined  
31 44 variable combinations. To avoid circumstantial correlation only evidential differing varia-  
32 bles can be chosen. This is a problem when more subtle variations, such as a slight reduction  
33 in ramp spacing, need to be investigated. The desired level of statistical validation requires  
34 sufficient data. When it comes to road geometric elements it is quite often difficult to get suf-  
35 ficient data on this (13; 75). Other models were developed to predict the crash likelihood  
36 based on real-time traffic flow variables measured from loop detectors. These studies used  
37 matched case-control methodology for the model development (28; 77; 78).

## 38 **Surrogate safety measures**

39 Because of the stated drawbacks of using crash statistics and the desire to take behavior of in-  
40 dividual drivers into account, researchers studied the possibility to replace and complement  
41 the traditional crash statistics with a surrogate (70; 71). The surrogate was found in traffic  
42 safety indicators, which increase the possibility of: 1) evaluating traffic safety changes more  
43 efficiently and in a shorter time; 2) elaborating the relation between design elements and risk  
44 3) more thoroughly understanding the relationships between behavior and risk and 4) a better  
45 understanding of the processes characterizing the normal traffic and critical situations includ-  
46 ing crashes and near crashes.

47  
48  
49 In order to do so, researchers tried to find measurable aspects in the traffic stream by which  
50 traffic safety can be quantified. The most frequently used measure is the Time To Collision

1 (TTC) value at an instant  $t$  is defined as: “*the time that remains until a collision between two*  
2 *vehicles would have occurred if the collision course and speed difference are*  
3 *maintained*”(15). Minderhoud and Bovy (15) proposed two additional safety indicators based  
4 on the TTC: the TET (Time Exposed Time-to-collision) and the TIT (Time Integrated Time-  
5 to-collision). The duration of exposition to safety-critical time-to-collision values over speci-  
6 fied time duration is used here as a safety indicator. The TET is a summation of all moments  
7 (over the considered time period) that a driver approaches a front vehicle with a TTC-value  
8 below a certain threshold value. The TIT is the integral of the time-to-collision profile. Alt-  
9 hough explicit thresholds are not mentioned, a general rule is applicable: the higher a TTC-  
10 value, the more safe the situation is (15; 79).

11  
12 In the case where the leading vehicle is faster than the following vehicle, TTC index cannot  
13 be estimated in a finite number. This is a practical weak point of TTC index because the situa-  
14 tion in which two subsequent vehicles following each other at a very close distance, can be  
15 considered as unsafe. Even if the leading vehicle drives at a slightly higher speed (80). To  
16 counter this weak point the Potential Index for Collision with Urgent Deceleration (PICUD)  
17 was proposed. This measure evaluates the possibility that two consecutive vehicles might  
18 collide, defined as the distance between the two vehicles considered when they completely  
19 stop (80; 81).

20  
21 A further variation of the Time-to-Collision concept is Post-Encroachment Time (PET). This  
22 measure is used to measure situations in which two road-users that are not on a collision  
23 course, pass over a common spatial point or area with a temporal difference that is below a  
24 predetermined threshold (73). One study is found in which the PET is calculated for a freeway  
25 (82). The PET was calculated for lane changing traffic. This research concludes that the ap-  
26 plication of extreme value theory over PETs during lane change manoeuvres provides a prom-  
27 ising approach for freeway safety evaluation.

28  
29 Two other indicators related to braking were introduced: Individual Braking Time Risk  
30 (IBTR) and Platoon Braking Time Risk (PBTR) or J-value (79). IBTR stands for the likeli-  
31 hood of a rear-end crash if the leading vehicle stops. PBTR stands for the accumulated risk of  
32 collision for each vehicle inside a platoon.

33  
34 Surrogate safety measures can be derived from trajectory data. For the validation of these  
35 measures crash statistics can be used. Also a method which does not require crash statistics is  
36 developed (82; 83). The most accurate way to derive surrogate safety measures is to use em-  
37 pirical trajectory data (73; 84). Also data from loop detectors can be used (85), but this kind  
38 of data gives less information than trajectory data. An alternative is to generate trajectories  
39 with micro simulation models (63; 79). This method however has a major drawback: the cur-  
40 rently available micro simulation programs are not suitable for safety study purposes (79)

#### 41 42 **Assessment of recorded crashes on video**

43 When video recordings of a crash are available a lot of useful information can be gained from  
44 these recordings (86; 87). Especially more insights in the conditions preceding the crash. Vid-  
45 eo footage of crashes can be used to generate individual vehicle trajectories on which micro-  
46 scopic analysis can be performed, such as deriving surrogate safety measures (86; 88; 89).  
47 The SHRP2 NDS study provided event files for approximately 700 crashes and 7,000 near-  
48 crashes. These files contain video footage, a trip summary and other data coded manually,  
49 such as driver distraction and cell phone use (48). But finding footage of specific locations  
50 will still take a lot of effort and will result in only a few number of crashes per facility.

## 1 **COMPARISON**

2 The different methods to collect data related to turbulence and the methods that assess the im-  
3 pact of turbulence on traffic operations and traffic safety, as detailed above are compared in  
4 Table 2. The table is organized based on the different aspects described in Figure 1 and 2, i.e.  
5 conditions and behavior.

6 Three different signs are used with the following interpretations: ‘+’ means that the specified  
7 method is suitable to take the considered functionality into account, ‘-’ means that the speci-  
8 fied method is **not** suitable to take the considered functionality into account and ‘+-’ means  
9 that the specified method can take the considered functionality into account but with a lack of  
10 accuracy.

11  
12 Loop detectors are very useful to acquire empirical macroscopic traffic data from which tur-  
13 bulence related aspects can be studied. However more detailed information, such as driving  
14 maneuvers and microscopic behavior cannot be measured directly. Video cameras can be used  
15 to derive empirical trajectory data which gives detailed information on driver maneuvers and  
16 macroscopic effects. Because of the level of detail of the collected information, the effects of  
17 the road geometry on turbulence can as well be studied. Trajectory data are not suitable for  
18 explaining the drivers’ decisions leading to maneuvers and turbulence. This can be studied us-  
19 ing a driver simulator or an instrumented vehicle. The advantage of the driver simulator is the  
20 controlled environment in which also new designs can be studied. The advantage of an in-  
21 strumented vehicle is that it can study actual road situation with actual traffic. Both the simu-  
22 lator and the instrumented vehicle consider only a single vehicle and its surrounding vehicles,  
23 while loop detectors and video cameras consider all vehicles. Therefore, data from loop detec-  
24 tors and video cameras are more suitable to be used for studying the macroscopic effects on  
25 turbulence, compared to data from a driver simulator or an instrumented vehicle.

26 When it comes to assessing the impact of turbulence on traffic operation, analyzing empirical  
27 traffic data is a good method for this purpose, especially when empirical trajectory data is  
28 available. For non-existing situations, such as new designs, microscopic simulation models  
29 can be used. These types of models however cannot model all aspects of the roads’ geometry  
30 and therefore cannot simulate microscopic behavior as realistically as desired.

31 Crash prediction models can be used as a method to assess the impact of turbulence on traffic  
32 safety. The drawback of this method is that large quantities of data are required to develop a  
33 model which can cope with a large set of variables. This is required when studying the effect  
34 of road and traffic characteristics on turbulence and its impact on traffic safety. Surrogate  
35 safety measures and video assessments give more traffic safety information on an individual  
36 vehicle level. Video recordings of crashes can give detailed insights on individual crashes that  
37 occurred in the past but give less insight on microscopic traffic conditions and preceding be-  
38 havior and maneuvers. Surrogate safety measures can take the microscopic traffic conditions  
39 into account, but lack the capacity of explaining behavior. Also information regarding road  
40 geometry and environment cannot be directly extracted from these methods.

**TABLE 2 Overview of Available Methodologies**

	Methodologies to collect data related to turbulence				Methodologies to assess the impact of turbulence					
	Loop detectors	Video cameras (vehicle trajectories)	Driver simulator	Instrumented vehicle / naturalistic studies	Traffic operations		Traffic safety			
					Traffic data analysis	Simulation models	crash prediction models	Surrogate safety measures	Crash video assessment	
New design	-	-	+	-	-	+	+	+	-	
Existing situation	+	+	+-	+	+	+	+	+	+	
<b>Conditions to take into account</b>	<b>Road Geometry</b>									
	Number of lanes	-	+	+	+	+	+	+	+-	+
	Shoulder width	-	+	+	+	+	-	+-	-	+
	Length of acc./dec. lane	-	+	+	+	+	+	+-	-	+
	Interchange spacing	-	+	+	+	+	+	+-	-	+
	Horizontal alignment	-	+	+	+	+	-	+-	-	+
	Vertical alignment	-	-	+-	+	+	-	+-	-	-
	Super elevation	-	-	-	+	+	-	+-	-	-
	<b>Traffic</b>									
	Average daily traffic	+	+	+-	+	+	+	+	+	+
	Speed / speed differences	+	+	+-	+	+	+	+	+	+
	Density per lane	+	+	+-	+	+	+	+	+	+
	<b>Environment</b>									
	Weather conditions	-	+-	-	+	+	-	+-	-	+
<b>Behavior</b>	Driving manoeuvres	-	+-	+	+	+	+-	-	+	+
	Microscopic behavior	-	+	+	+	+	+	+-	+	+
	Macroscopic effects	+	+	-	-	+	+	+-	+	+

## 1 CONCLUSIONS AND RECOMMENDATIONS

2  
3 Turbulence covers different elements of microscopic traffic characteristics such as lane  
4 changing, variation in speeds and headways and is the result of a complex combination of dif-  
5 ferent driving manoeuvres. Literature and freeway design guidelines agree that the level, or  
6 magnitude, of turbulence is influenced by road design, traffic volume and driver behavior, and  
7 that turbulence has an effect on traffic operations and safety. Although there seems to be an  
8 agreement on what turbulence is, there is no definition found which covers all causes and ef-  
9 fects of turbulence. Since turbulence is a commonly present in the traffic stream, two defini-  
10 tions are proposed:

- 11 - Turbulence:
  - 12 · individual changes in speed, headways, and lanes (i.e. lane-changes) in a certain road
  - 13 segment, regardless the cause of the change;
- 14 - Level of Turbulence:
  - 15 the frequency and intensity of individual changes in speed, headways and lane-
  - 16 changes in a certain road segment, over a certain period of time.

17  
18 The level of turbulence is expected to be higher around discontinuities (on freeways) com-  
19 pared to continuous road stretches. Although research and design guidelines agree on the con-  
20 cept of turbulence, a gap can be observed between guidelines and research: where guidelines  
21 frequently rely on unreferenced assumptions and rules of thumb, research tries to assess the  
22 impacts of different elements of turbulence on traffic safety and traffic operations and the in-  
23 fluence of design characteristics on these impacts. Furthermore, the results of research do not  
24 seem to fully find their way into the freeway design guidelines. One of the reasons for this  
25 may be that the currently available methodologies are not able to combine the effects of road  
26 design on turbulence with its impact on both traffic safety and traffic operations.

27  
28 Several methods to assess traffic operations and traffic safety exist today, such as the use of  
29 microscopic simulation programs, surrogate safety measures, crash prediction models and  
30 driver simulators. However, each of the methods has its own strengths and weaknesses. Con-  
31 sidering these strengths and weaknesses, combining different methods might be a potential  
32 solution for this problem, that is worth researching in the future. The overview in Table 2  
33 suggests that combining microscopic simulation software with surrogate safety measure  
34 methodologies is the most promising way forward. By doing that, road characteristics, traffic  
35 characteristics and microscopic behavior can be taken into account to evaluate the safety and  
36 capacity of a certain freeway segment. There are however a few challenges that need to be  
37 overcome.

38  
39 The first challenge is that the currently available microscopic simulation programs are not de-  
40 signed for traffic safety studies (79). These programs also do not simulate merging behavior  
41 as accurately as desired. This makes them unsuitable for generating trajectory data from  
42 which surrogate safety measures can be derived (79; 90). Also surrogate safety measures  
43 seem to be in a theoretical stage, where valid threshold values need still to be set.

44  
45 For the improvement of the existing microscopic simulation models a more realistic, mathe-  
46 matical description of merging behavior is needed. Despite the huge improvements in micro-  
47 scopic simulation models' appearance and visualization, the advancement in its traffic behav-  
48 ior performance is at a much slower pace. For example, the most recent car following model  
49 in VISSIM dates from 1999 and AIMSUN uses a car-following model based on the model  
50 developed by Gipps (1981).

1 The most important improvements should be the merging behavior in itself by using gap se-  
 2 lection instead of gap acceptance theory. Other types of behavior such as pre allocation, cour-  
 3 tesy yielding and relaxation phenomena should also be integrated more realistically. But also  
 4 unsafe situations should be possible to occur in models in order to be able to generate realistic  
 5 trajectories to derive surrogate safety measures. It is also important to develop mathematical  
 6 models that take into account, for example, different driving styles and behaviors of drivers  
 7 and account for drivers' heterogeneity. Also existing or maybe new models should be cali-  
 8 brated and validated by the use of empirical trajectory data. The behavioral aspects can be  
 9 studied by using driver simulator, instrumented vehicle or a naturalistic driver study.

10  
 11 A second challenge is that good quality empirical trajectory data is scarce. The available tra-  
 12 jectory data focusses mainly on the merging area (19; 33; 36) and not so much on the areas  
 13 upstream and downstream of the discontinuity. Therefore new data is needed.

14  
 15 A third challenge is that not all the design elements can be taken into account by available mi-  
 16 croscopic simulation models. Elements such as shoulder width, horizontal alignment, vertical  
 17 alignment and super elevation, can either not be modelled at all, using the existing simulation  
 18 programs, or do not affect the simulated driving behavior realistically. The impacts of the dif-  
 19 ferent design elements on driving behavior related to turbulence need to be studied more in  
 20 order to be able to show if and how design elements have an impact on driving behavior.  
 21 Suitable methodologies for this research are the use of a driver simulator, instrumented vehi-  
 22 cle and/or a naturalistic driver study. It is recommended to include these methodologies in fur-  
 23 ther research on turbulence and the effects on safety and operation.

## 24 25 REFERENCES

- 26  
 27 [1] HCM. Transportation Research Board of the National Academies, 2010. 2010.  
 28 [2] Abdel-Aty, and Pande. Identifying crash propensity using specific traffic speed conditions. *Journal of Safety*  
 29 *Research*, Vol. 36, 2005, p. 12.  
 30 [3] Golob, T. F., W. W. Recker, and V. M. Alvarez. Freeway safety as a function of traffic flow. *Accident*  
 31 *Analysis & Prevention*, Vol. 36, No. 6, 2004, pp. 933-946.  
 32 [4] Kondyli, A., and L. Elefteriadou. Modeling driver behavior at freeway-ramp merges. *Transportation*  
 33 *Research Record: Journal of the Transportation Research Board*, Vol. 2249, No. 1, 2011, pp. 29-37.  
 34 [5] ---. Driver behavior at freeway-ramp merging areas based on instrumented vehicle observations.  
 35 *Transportation Letters*, Vol. 4, No. 3, 2012, pp. 129-142.  
 36 [6] Lee, C., B. Hellinga, and F. Saccomanno. Proactive freeway crash prevention using real-time traffic control.  
 37 *Canadian Journal of Civil Engineering*, Vol. 30, No. 6, 2003, pp. 1034-1041.  
 38 [7] AASHTO. *A Policy on Geometric Design of Highways and Streets*, 2001.  
 39 [8] Agency, T. H., T. Scotland, T. Wales, and T. D. f. R. Development. *Design Manual for Roads and Bridges*  
 40 *(DMRB), Volume 6, Section 2, Part 4 'The design of major interchanges'*.  
 41 [9] FGSW. Richtlinie für die Anlage von Autobahnen (RAA).In, Ausgabe, 2008.  
 42 [10] Rijkswaterstaat. Nieuwe Ontwerprichtlijn Autosnelwegen.In, 2007.  
 43 [11] ---. Richtlijnen voor het Ontwerpen van Autosnelwegen.In, 1992.  
 44 [12] Bared, J. G., P. K. Edara, and T. Kim. Safety Impact of Interchange Spacing on Urban Freeways.In  
 45 *Compendium of Papers DVD, Annual Meeting of the Transportation Research Board, Washington, DC,*  
 46 *2006.*  
 47 [13] Pilko, P., J. Bared, P. Edara, and T. Kim. Safety Assessment of Interchange Spacing on Urban Freeways.  
 48 *Tech Brief*, 2007.  
 49 [14] Hovenga, M. Measuring turbulence distance of acceleration lanes.In *Transport and Planning, No. MSc,*  
 50 *Delft University of Technology, Delft, 2014.*  
 51 [15] Minderhoud, M. M., and P. H. Bovy. Extended time-to-collision measures for road traffic safety  
 52 assessment. *Accident Analysis & Prevention*, Vol. 33, No. 1, 2001, pp. 89-97.  
 53 [16] Kondyli, A., and L. Elefteriadou. Driver behavior at freeway-ramp merging areas. *Transportation Research*  
 54 *Record: Journal of the Transportation Research Board*, Vol. 2124, No. 1, 2009, pp. 157-166.



- 1 [17] Hidas, P. Modelling vehicle interactions in microscopic simulation of merging and weaving. *Transportation*  
2 *Research Part C: Emerging Technologies*, Vol. 13, No. 1, 2005, pp. 37-62.
- 3 [18] Schakel, W. J., V. L. Knoop, and B. van Arem. Integrated lane change model with relaxation and  
4 synchronization. *Transportation Research Record: Journal of the Transportation Research Board*, Vol.  
5 2316, No. 1, 2012, pp. 47-57.
- 6 [19] Daamen, W., M. Loot, and S. P. Hoogendoorn. Empirical analysis of merging behavior at freeway on-ramp.  
7 *Transportation Research Record: Journal of the Transportation Research Board*, Vol. 2188, No. 1, 2010,  
8 pp. 108-118.
- 9 [20] Laval, J. A., and L. Leclercq. Microscopic modeling of the relaxation phenomenon using a macroscopic  
10 lane-changing model. *Transportation Research Part B: Methodological*, Vol. 42, No. 6, 2008, pp. 511-522.
- 11 [21] Toledo, T., C. F. Choudhury, and M. E. Ben-Akiva. Lane-changing model with explicit target lane choice.  
12 *Transportation Research Record: Journal of the Transportation Research Board*, Vol. 1934, No. 1, 2005,  
13 pp. 157-165.
- 14 [22] Koutsopoulos, H. N., and H. Farah. Latent class model for car following behavior. *Transportation research*  
15 *part B: methodological*, Vol. 46, No. 5, 2012, pp. 563-578.
- 16 [23] Toledo, T., H. N. Koutsopoulos, and M. Ben-Akiva. Integrated driving behavior modeling. *Transportation*  
17 *Research Part C: Emerging Technologies*, Vol. 15, No. 2, 2007, pp. 96-112.
- 18 [24] ---. Estimation of an integrated driving behavior model. *Transportation Research Part C: Emerging*  
19 *Technologies*, Vol. 17, No. 4, 2009, pp. 365-380.
- 20 [25] Xu, C., P. Liu, W. Wang, and Z. Li. Evaluation of the impacts of traffic states on crash risks on freeways.  
21 *Accident Analysis & Prevention*, Vol. 47, 2012, pp. 162-171.
- 22 [26] Treiber, M., A. Hennecke, and D. Helbing. Congested traffic states in empirical observations and  
23 microscopic simulations. *Physical Review E*, Vol. 62, No. 2, 2000, p. 1805.
- 24 [27] Abdel-Aty, M., N. Uddin, and A. Pande. Split models for predicting multivehicle crashes during high-speed  
25 and low-speed operating conditions on freeways. *Transportation Research Record: Journal of the*  
26 *Transportation Research Board*, Vol. 1908, No. 1, 2005, pp. 51-58.
- 27 [28] Abdel-Aty, M., N. Uddin, A. Pande, F. M. Abdalla, and L. Hsia. Predicting freeway crashes from loop  
28 detector data by matched case-control logistic regression. *Transportation Research Record: Journal of the*  
29 *Transportation Research Board*, Vol. 1897, No. 1, 2004, pp. 88-95.
- 30 [29] Abdel-Aty, M. A., and R. Pemmanaboina. Calibrating a real-time traffic crash-prediction model using  
31 archived weather and ITS traffic data. *Intelligent Transportation Systems, IEEE Transactions on*, Vol. 7,  
32 No. 2, 2006, pp. 167-174.
- 33 [30] abdel-Aty, M., A. Pande, L. Y. Hsia, and F. Abdalla. The Potential for Real-Time Traffic Crash Prediction.  
34 *ITE Journal*, 2005, p. 7.
- 35 [31] Piao, J., and M. McDonald. Safety impacts of variable speed limits-a simulation study. In *Intelligent*  
36 *Transportation Systems, 2008. ITSC 2008. 11th International IEEE Conference on*, IEEE, 2008. pp. 833-  
37 837.
- 38 [32] Hoogendoorn, S., R. G. Hoogendoorn, and W. Daamen. Wiedemann Revisited. *Transportation Research*  
39 *Record: Journal of the Transportation Research Board*, Vol. 2260, No. 1, 2011, pp. 152-162.
- 40 [33] Marczak, F., W. Daamen, and C. Buisson. Key variables of merging behaviour: empirical comparison  
41 between two sites and assessment of gap acceptance theory. *Procedia-Social and Behavioral Sciences*, Vol.  
42 80, 2013, pp. 678-697.
- 43 [34] Hoogendoorn, S. P., H. Van Zuylen, M. Schreuder, B. Gorte, and G. Vosselman. Traffic data collection  
44 from aerial imagery. In *CTS2003, 10th IFAC symposium on control in transportation systems*, 2003. pp. 1-  
45 6.
- 46 [35] Voorrips, R. Freeway Work Zone Driving Behaviour - The Influence of Work Zone Configurations. In *Citg*,  
47 *No. MSc*, TUDelft, 2013.
- 48 [36] NGSIM. *NGSIM website*. <http://ngsim-community.org/>. Accessed 22-06-2015.
- 49 [37] Winsum, W. v., and A. Heino. Choice of time-headway in car-following and the role of time-to-collision  
50 information in braking. *Ergonomics*, Vol. 39, No. 4, 1996, pp. 579-592.
- 51 [38] De Waard, D., C. Dijksterhuis, and K. A. Brookhuis. Merging into heavy motorway traffic by young and  
52 elderly drivers. *Accident Analysis & Prevention*, Vol. 41, No. 3, 2009, pp. 588-597.
- 53 [39] Farah, H., S. Bekhor, and A. Polus. Risk evaluation by modeling of passing behavior on two-lane rural  
54 highways. *Accident Analysis & Prevention*, Vol. 41, No. 4, 2009, pp. 887-894.
- 55 [40] McDonald, M., J. Wu, and M. Brackstone. Development of a fuzzy logic based microscopic motorway  
56 simulation model. *Proceedings of Intelligent Transportation Systems Council, Institute of Electrical and*  
57 *Electronics Engineers*, 1997.

- 1 [41] Brackstone, M., B. Sultan, and M. McDonald. Motorway driver behaviour: studies on car following.  
2 *Transportation Research Part F: Traffic Psychology and Behaviour*, Vol. 5, No. 1, 2002, pp. 31-46.
- 3 [42] Wu, J., M. Brackstone, and M. McDonald. The validation of a microscopic simulation model: A  
4 methodological case study. *Transportation Research Part C: Emerging Technologies*, Vol. 11, No. 6, 2003,  
5 pp. 463-479.
- 6 [43] Kesting, A., and M. Treiber. Calibrating car-following models by using trajectory data: Methodological  
7 study. *Transportation Research Record: Journal of the Transportation Research Board*, Vol. 2088, No. 1,  
8 2008, pp. 148-156.
- 9 [44] Olson, R. L., R. J. Hanowski, J. S. Hickman, and J. L. Bocanegra. Driver distraction in commercial vehicle  
10 operations. In, 2009.
- 11 [45] Blanco, M., R. J. Hanowski, R. L. Olson, J. F. Morgan, S. A. Soccolich, S.-C. Wu, and F. Guo. The impact  
12 of driving, non-driving work, and rest breaks on driving performance in commercial motor vehicle  
13 operations. In, 2011.
- 14 [46] Chong, L., M. M. Abbas, A. Medina Flintsch, and B. Higgs. A rule-based neural network approach to  
15 model driver naturalistic behavior in traffic. *Transportation Research Part C: Emerging Technologies*, Vol.  
16 32, No. 0, 2013, pp. 207-223.
- 17 [47] Antin, J. F. *Design of the in-vehicle driving behavior and crash risk study: in support of the SHRP 2*  
18 *naturalistic driving study*. Transportation Research Board, 2011.
- 19 [48] NDS. *NDS website*. <http://www.trb.org/StrategicHighwayResearchProgram2SHRP2/Blank2.aspx>. Accessed  
20 12-10-2015.
- 21 [49] Cassidy, M. J., S. B. Anani, and J. M. Haigwood. Study of freeway traffic near an off-ramp. *Transportation*  
22 *Research Part A: Policy and Practice*, Vol. 36, No. 6, 2002, pp. 563-572.
- 23 [50] Chung, K., J. Rudjanakanoknad, and M. J. Cassidy. Relation between traffic density and capacity drop at  
24 three freeway bottlenecks. *Transportation Research Part B: Methodological*, Vol. 41, No. 1, 2007, pp. 82-  
25 95.
- 26 [51] Coifman, B., and S. Kim. Extended bottlenecks, the fundamental relationship, and capacity drop on  
27 freeways. *Transportation Research Part A: Policy and Practice*, Vol. 45, No. 9, 2011, pp. 980-991.
- 28 [52] Rahman, M., M. Chowdhury, Y. Xie, and Y. He. Review of Microscopic Lane-Changing Models and  
29 Future Research Opportunities. 2013.
- 30 [53] Hoogendoorn, S. P., and P. H. Bovy. State-of-the-art of vehicular traffic flow modelling. *Proceedings of the*  
31 *Institution of Mechanical Engineers, Part I: Journal of Systems and Control Engineering*, Vol. 215, No. 4,  
32 2001, pp. 283-303.
- 33 [54] Coifman, B., S. Krishnamurthy, and X. Wang. Lane-change maneuvers consuming freeway capacity. In  
34 *Traffic and Granular Flow'03*, Springer, 2005. pp. 3-14.
- 35 [55] Laval, J. A., and L. Leclercq. A mechanism to describe the formation and propagation of stop-and-go waves  
36 in congested freeway traffic. *Philosophical Transactions of the Royal Society of London A: Mathematical,*  
37 *Physical and Engineering Sciences*, Vol. 368, No. 1928, 2010, pp. 4519-4541.
- 38 [56] Laval, J. A. Hysteresis in traffic flow revisited: An improved measurement method. *Transportation*  
39 *Research Part B: Methodological*, Vol. 45, No. 2, 2011, pp. 385-391.
- 40 [57] Zheng, Z., S. Ahn, D. Chen, and J. Laval. Applications of wavelet transform for analysis of freeway traffic:  
41 Bottlenecks, transient traffic, and traffic oscillations. *Transportation Research Part B: Methodological*,  
42 Vol. 45, No. 2, 2011, pp. 372-384.
- 43 [58] ---. Freeway traffic oscillations: Microscopic analysis of formations and propagations using Wavelet  
44 Transform. *Transportation Research Part B: Methodological*, Vol. 45, No. 9, 2011, pp. 1378-1388.
- 45 [59] Sun, D. J., and A. Kondyli. Modeling Vehicle Interactions during Lane-Changing Behavior on Arterial  
46 Streets. *Computer-Aided Civil and Infrastructure Engineering*, Vol. 25, No. 8, 2010, pp. 557-571.
- 47 [60] Chih-Sheng, C., and A. P. Nichols. Deriving a surrogate safety measure for freeway incidents based on  
48 predicted end-of-queue properties. *Intelligent Transport Systems, IET*, Vol. 9, No. 1, 2015, pp. 22-29.
- 49 [61] Dijkstra, A. *En route to safer roads: how road structure and road classification can affect road safety*.  
50 University of Twente, 2011.
- 51 [62] Young, W., A. Sobhani, M. G. Lenné, and M. Sarvi. Simulation of safety: A review of the state of the art in  
52 road safety simulation modelling. *Accident Analysis & Prevention*, Vol. 66, 2014, pp. 89-103.
- 53 [63] Astarita, V., G. Guido, A. Vitale, and V. Giofré. A new microsimulation model for the evaluation of traffic  
54 safety performances. 2012.
- 55 [64] Dijker, T., and P. Knoppers. Fosim 5.0 user manual. *Technical specification report, Delft University of*  
56 *Technology, Transport and Planning Department, Netherlands, ISSN, No. 0920-0592, 2004.*
- 57 [65] Garber, N. J., and M. Fontaine. Final Report-Guidelines for Preliminary Selection of the Optimum  
58 Interchange Type for a Specific Location. *Virginia Transportation Research Council*, 1999.

- 1 [66] Wang, X., M. Hadiuzzaman, and T. Z. Qiu. Analyzing sensitivity of freeway capacity at a complex weaving  
2 segment. *TC*, Vol. 1033, 2012, p. 1.
- 3 [67] Martínez, M. P., A. Garcia, and A. T. Moreno. Traffic Microsimulation Study to Evaluate Freeway Exit  
4 Ramps Capacity. *Procedia - Social and Behavioral Sciences*, Vol. 16, No. 0, 2011, pp. 139-150.
- 5 [68] Sharma, S., and I. Chatterjee. Performance Evaluation of the Diverging Diamond Interchange In  
6 Comparison With the Conventional Diamond Interchange. In *Transportation Scholars Conference, Iowa*  
7 *State University, Ames*, 2007.
- 8 [69] HSM. American Association of state highway and Transportation Officials (AASHTO). *Washington, DC*,  
9 Vol. 10, 2010.
- 10 [70] Tarko, A., G. Davis, N. Saunier, T. Sayed, and S. Washington. Surrogate measures of safety. *White paper*,  
11 *ANB20 (3) Subcommittee on Surrogate Measures of Safety*, 2009.
- 12 [71] Laureshyn, A., Å. Svensson, and C. Hydén. Evaluation of traffic safety, based on micro-level behavioural  
13 data: Theoretical framework and first implementation. *Accident Analysis & Prevention*, Vol. 42, No. 6,  
14 2010, pp. 1637-1646.
- 15 [72] Anastasopoulos, P. C., A. P. Tarko, and F. L. Mannering. Tobit analysis of vehicle accident rates on  
16 interstate highways. *Accident Analysis & Prevention*, Vol. 40, No. 2, 2008, pp. 768-775.
- 17 [73] Archer, J. Indicators for traffic safety assessment and prediction and their application in micro-simulation  
18 modelling: A study of urban and suburban intersections. 2005.
- 19 [74] Lord, D., and F. Mannering. The statistical analysis of crash-frequency data: a review and assessment of  
20 methodological alternatives. *Transportation Research Part A: Policy and Practice*, Vol. 44, No. 5, 2010,  
21 pp. 291-305.
- 22 [75] Guo, Y. Q. The Relationship between Freeway Safety and Geometric Design Elements. *Advanced Materials*  
23 *Research*, Vol. 424, 2012, pp. 215-219.
- 24 [76] Garber, N. J., and A. A. Ehrhart. Effect of speed, flow, and geometric characteristics on crash frequency for  
25 two-lane highways. *Transportation Research Record: Journal of the Transportation Research Board*, Vol.  
26 1717, No. 1, 2000, pp. 76-83.
- 27 [77] Roshandel, S., Z. Zheng, and S. Washington. Impact of real-time traffic characteristics on freeway crash  
28 occurrence: Systematic review and meta-analysis. *Accident Analysis & Prevention*, Vol. 79, 2015, pp. 198-  
29 211.
- 30 [78] Shi, Q., and M. Abdel-Aty. Big Data applications in real-time traffic operation and safety monitoring and  
31 improvement on urban expressways. *Transportation Research Part C: Emerging Technologies*, Vol. 58,  
32 Part B, 2015, pp. 380-394.
- 33 [79] Bevrani, K., and E. Chung. An examination of the microscopic simulation models to identify traffic safety  
34 indicators. *International Journal of Intelligent Transportation Systems Research*, Vol. 10, No. 2, 2012, pp.  
35 66-81.
- 36 [80] Uno, N., Y. Iida, S. Itsubo, and S. Yasuhara. A microscopic analysis of traffic conflict caused by lane-  
37 changing vehicle at weaving section. In *Proceedings of The 13th Mini-Euro Conference " Handling*  
38 *Uncertainty in Transportation Analysis of Traffic and Transportation Systems*, 2002. pp. 143-148.
- 39 [81] Bin, M., N. Uno, and Y. Iida. A study of lane-changing behavior model at weaving section considering  
40 conflicts. *Journal of the Eastern Asia Society for Transportation Studies*, Vol. 5, 2003, pp. 2039-2052.
- 41 [82] Zheng, L., K. Ismail, and X. Meng. Freeway safety estimation using extreme value theory approaches: A  
42 comparative study. *Accident Analysis & Prevention*, Vol. 62, 2014, pp. 32-41.
- 43 [83] Tarko, A. P. Use of crash surrogates and exceedance statistics to estimate road safety. *Accident Analysis &*  
44 *Prevention*, Vol. 45, 2012, pp. 230-240.
- 45 [84] Louah, G., D. Daucher, P. Conde-Céspedes, F. Bosc, and J.-P. Lhuillier. Traffic Operations at an Entrance  
46 Ramp of a Suburban Freeway First Results. *Procedia-Social and Behavioral Sciences*, Vol. 16, 2011, pp.  
47 162-171.
- 48 [85] Li, Ahn, Chung, Ragland, Wang, and Yu. Surrogate safety measure for evaluating rear-end collision risk  
49 related to kinematic waves near freeway recurrent bottlenecks. *Accident Analysis and Prevention*, Vol. 64,  
50 2014, p. 10.
- 51 [86] Davis, G. A., and T. Swenson. Collective responsibility for freeway rear-ending accidents?: An application  
52 of probabilistic causal models. *Accident Analysis & Prevention*, Vol. 38, No. 4, 2006, pp. 728-736.
- 53 [87] Bonneson, J., and J. Ivan. Theory, Explanation, and Prediction in Road Safety: Promising Directions.  
54 *Transportation Research E-Circular*, No. E-C179, 2013.
- 55 [88] Oh, C., and T. Kim. Estimation of rear-end crash potential using vehicle trajectory data. *Accident Analysis*  
56 *& Prevention*, Vol. 42, No. 6, 2010, pp. 1888-1893.

- 1 [89] Guido, G., F. Saccomanno, A. Vitale, V. Astarita, and D. Festa. Comparing safety performance measures  
2 obtained from video capture data. *Journal of transportation engineering*, Vol. 137, No. 7, 2010, pp. 481-  
3 491.
- 4 [90] Gettman, D., and L. Head. Surrogate safety measures from traffic simulation models. *Transportation*  
5 *Research Record: Journal of the Transportation Research Board*, Vol. 1840, No. 1, 2003, pp. 104-115.  
6