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8 An integrated workflow for stress and flow

⁹ modelling using outcrop-derived discrete

¹⁰ fracture networks

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17 fracture networks; fracture mechanics; outcrop analogue modelling; Brazil

18 Abstract

19 Fluid flow in naturally fractured reservoirs is often controlled by subseismic-scale fracture 20 networks. Although the fracture network can be partly sampled in the direct vicinity of wells, 21 the inter-well scale network is poorly constrained in fractured reservoir models. Outcrop 22 analogues can provide data for population of domains of the reservoir model where no direct 23 measurements are available. However, extracting relevant statistics from large outcrops 24 representative of inter-well scale fracture networks remains challenging. Recent advances in 25 outcrop imaging provide high-resolution datasets that can cover areas of several hundred by 26 several hundred meters, i.e. the domain between adjacent wells, but even then, data from the

27 high-resolution models is often upscaled to reservoir flow grids, resulting in loss of accuracy. 28 We present a workflow that uses photorealistic georeferenced outcrop models to construct 29 geomechanical and fluid flow models containing thousands of discrete fractures covering 30 sufficiently large areas, that does not require upscaling to model permeability. This workflow 31 seamlessly integrates geomechanical Finite Element models with flow models that take into account stress-sensitive fracture permeability and matrix flow to determine the full 32 permeability tensor. The applicability of this workflow is illustrated using an outcropping 33 carbonate pavement in the Potiguar basin in Brazil, from which 1082 fractures are digitised. 34 The permeability tensor for a range of matrix permeabilities shows that conventional 35 36 upscaling to effective grid properties leads to potential underestimation of the true 37 permeability and the orientation of principal permeabilities. The presented workflow yields 38 the full permeability tensor model of discrete fracture networks with stress-induced apertures, 39 instead of relying on effective properties as most conventional flow models do.

40

1. Introduction

41 Natural fracture networks are multiscale systems that develop through a combination of 42 mechanisms that are only partly understood (Olson et al., 2009; Philip et al., 2005). Understanding the multiscale distribution of fracture networks in the subsurface allows for 43 44 optimisation of fractured reservoir development (Nelson, 2001). However, limited 45 observations from seismic and wells do not provide the complete fracture network geometry 46 and associated flow properties, particularly of the subseismic fracture network (Fabuel-Perez 47 et al., 2010; Martinez-Landa et al., 2016). Outcrops are the only source to obtain realistic descriptions of fracture networks, as no models exist that can create realistic fracture 48 49 networks on the scale of fractured reservoirs. To derive lessons for fractured reservoirs, we 50 need outcropping datasets that contain at least several hundred fractures covering several 51 orders of magnitude in spacing and length, without suffering from censoring and truncation

52 artefacts, over an area that covers at least several grid blocks in conventional reservoir flow 53 models (Bonnet et al., 2001; Ortega et al., 2006). Such datasets are not easily obtained from 54 conventional outcrop photographs or scanline studies, as these methods capture only a limited 55 number of scales (Bisdom et al., 2014).

56 Photogrammetry, particularly Structure from Motion (SfM) Multiview stereo (MVS), is an inexpensive and fast method to accurately map 3-D structures from 2-D images taken 57 58 from different positions (Harwin and Lucieer, 2012; Ullman, 1979). In recent years, this 59 method has been embraced by geologists to create digital outcrop models as an alternative to 60 the more expensive and less flexible LiDAR (Light Image Detection And Ranging) methods (Hodgetts, 2013; Mahmud et al., 2015; Reif et al., 2011; Rotevatn et al., 2009; Tavani et al., 61 62 2014; Wilson et al., 2011). Partly overlapping images are aligned by identifying and 63 extracting common points, which can be positioned in 3-D space to reconstruct the outcrop 64 geometry (Bemis et al., 2014; James and Robson, 2012). The resulting models provide a 65 complete and unobstructed viewpoint of the outcrop that can be changed and adjusted for any purpose (Tavani et al., 2016). 66

67 As this approach requires that the outcrop is fully covered by images with an overlap 68 of at least 50%, Unmanned Aerial Vehicles (UAVs or drones), equipped with a camera and 69 positioning sensors, are best suited to acquire the images required for photogrammetry modelling (e.g. Bernis et al., 2014; Bond et al., 2015; Hodgetts, 2013; James and Robson, 70 71 2012; Tavani et al., 2014; Vasuki et al., 2014; Vollgger and Cruden, 2016). Fracture 72 geometries can be extracted from the resulting georeferenced models in 2-D or 3-D (Duelis 73 Viana et al., 2016; Hardebol and Bertotti, 2013; Tavani et al., 2014). Extraction of 2-D data 74 from a 3-D photogrammetry model is more accurate than fracture interpretation from 75 conventional 2-D images, as the photogrammetry model is accurately orthorectified and the 76 multiple viewpoints allow for more precise digitisation of fracture geometry. Irrespective of whether the fracture data is used for 2-D or 3-D analysis, 3-D outcrop models provide ahigher accuracy.

79 The second challenge is to obtain realistic aperture predictions from outcropping 80 geometries. At depth, permeability is a function of aperture, which is partly controlled by the in-situ stresses (Baghbanan and Jing, 2008; Lei et al., 2015; Tao et al., 2009; Zoback, 2007), 81 but pressure relief during exhumation and weathering dissolves cements and changes 82 83 aperture. Outcropping apertures are therefore not representative, unless it can be proven that 84 fractures have not been reactivated during exhumation. This is typically assumed to be the 85 case for veins (e.g. Hooker et al., 2014), but preserved veins are relatively rare. Alternatively, aperture is modelled as a function of stress, using subcritical crack growth as defined by 86 87 Linear Elastic Fracture Mechanics (LEFM) or conductive shearing defined by Barton-Bandis 88 (Barton, 1982; Barton et al., 1985; Barton and Bandis, 1980; Lawn and Wilshaw, 1975; 89 Olson, 2003; Pollard and Segall, 1987; Vermilye and Scholz, 1995). These models require 90 the local stress state, which is typically derived from Finite Element (FE) models with 91 explicit fracture representations (Barton, 2014; Bisdom et al., 2016b; Lei et al., 2016, 2014; 92 Nick et al., 2011).

93 The third challenge is modelling permeability through fractured rocks, taking into 94 account the coupled flow through fractures and matrix (Belayneh et al., 2009; Geiger et al., 95 2013; Lang et al., 2014). Conventional reservoir simulation tools scale up fracture density, porosity and permeability to effective grid properties in dual-porosity dual-permeability 96 97 grids, resulting in a significantly simplified flow model (Cottereau et al., 2010; Geiger and 98 Matthäi, 2012; Jonoud and Jackson, 2008). Methods exist to model flow through discrete 99 fracture-matrix models without requiring upscaling, making use of a Finite-Element Finite-100 Volume (FE-FV) approach, but the use of these methods is often limited to relatively small-101 scale synthetic fracture networks (Lei et al., 2014; Matthäi and Belayneh, 2004).

102 These individual problems have been studied extensively, focusing on 3-D outcrop 103 modelling (Hodgetts, 2013; Tavani et al., 2014; Vasuki et al., 2014), meshing (Karimi-Fard 104 and Durlofsky, 2016; Nejati et al., 2016; Nick and Matthäi, 2011a; Paluszny et al., 2007) and 105 flow modelling (Lang et al., 2014; Nick and Matthäi, 2011b), but integrating these 106 components remains a challenge. Our aim is to present an integrated workflow for modelling the complete permeability tensor of large-scale 107 fracture networks with apertures 108 representative of in-situ stress conditions by combining fast data acquisition using a UAV with outcrop modelling using photogrammetry (Figure 1). This workflow builds upon the 109 stress-aperture modelling approach presented in Bisdom et al. (2016b), making use of the 110 111 geometrical aperture approximation from Bisdom et al. (2016d), and the modelling of 112 permeability for a range of aperture definitions presented in (Bisdom et al., 2016d). The 3-D 113 outcrop models are used to accurately digitise fracture patterns in 2-D, which form the basis for stress, aperture and equivalent permeability (i.e. combined matrix and fracture 114 115 permeability) models. The main result is a discrete fracture-matrix model consisting of an 116 unstructured mesh with discrete fractures, from which the full permeability tensor is 117 calculated. The aim of this workflow is to improve the representativeness of outcrops as a 118 proxy for flow in naturally fractured reservoirs, by capturing larger-scale high-resolution 119 fracture patterns covering distances comparable to well spacing in fractured reservoirs, 120 followed by modelling of aperture and flow representative of subsurface conditions. We 121 illustrate the effectiveness of the workflow using an example of 2-D fracture patterns in 122 outcropping carbonates in the Potiguar Basin, NE Brazil (Bertotti et al., accepted; de Graaf et 123 al., 2017).

125

2. Quantitative outcrop modelling using a UAV and photogrammetry

126 2.1. Image acquisition with a UAV

127 We use a multi-rotor UAV (Figure 2) to acquire images of multiscale fracture patterns over 128 an area that covers several reservoir simulation grid blocks, which are subsequently merged 129 into 2-D georeferenced outcrop models. To ensure that an area is fully covered by images 130 with constant overlap, flight paths are programmed prior to flights (Figure 3). The 131 programmed flights are automatically executed and controlled using a GNSS sensor (2 m 132 accuracy) for horizontal positioning and a temperature-compensated barometer (dm-133 accuracy) for vertical positioning. A magnetic compass provides the absolute heading and 134 accelerometers and a gyroscope further control the position of the UAV and ensure stable 135 operation. Flight and environment conditions are continuously monitored and stored during 136 flights for quality-check and diagnostic purposes.

Outcrop images are taken with a 14-megapixel compact camera in a motorised mount attached to the UAV. The resulting image resolution depends on the altitude and camera specifications. A distance of 50 m between the UAV and the surface of interest yields a resolution R of 1.44 cm/px, which changes linearly with changing distance H (in m):

141
$$R = \frac{100w_s H}{w_i F_r} , \qquad (1.1)$$

142 where w_s is the camera sensor width (in mm), w_i is the image width (in pixels) and F_r is the 143 real focal length (in mm) for the focal length in a 35 mm equivalent (*F35*):

144
$$F_r = \frac{F_{35} W_s}{34.6} . \tag{1.2}$$

Depending on outcrop size and required resolution, most flights are between 20-100 m altitude, corresponding to resolutions of 0.6-2.9 cm/px respectively. For each image, the corresponding UAV position (horizontal coordinates and altitude) and orientation are determined using the GNSS sensor and barometer. To further constrain the outcrop position, brightly marked GCPs (Ground Control Points) are positioned on the outcrop surface, which are georeferenced with a GPS, and we measure the distance between these points using a hand-held laser range finder for further scaling of the model.

152 2.2. Outcrop model construction using photogrammetry

Using Agisoft PhotoScan®, we process the UAV images and location data into georeferenced 2-D and 3-D outcrop models. This workflow consists of image alignment, point cloud generation, surface meshing and texturing (Figure 4 and Figure 5).

156 2.2.1. Image alignment

The approximate position from where each image is taken, is used to identify the image pairs that are likely to overlap. Common points are identified and extracted and positioned in 3-D in a *sparse point cloud* (Figure 4a). The vertical position of a point is determined from the relative change in position in the partly overlapping images, where one point is imaged from different angles.

162

2.2.2. Point cloud generation

163 Once images are aligned, all points from the aligned images are extracted and positioned in 3-164 D to generate a *dense point cloud* (Figure 4b and Figure 5a). Depending on flight altitude, the 165 point cloud can have a resolution that is similar to LiDAR datasets. A single flight at 50 m 166 altitude, capturing 100 images covering an area of 200 m \times 200 m, results in a dense point 167 cloud of 1.4×10^7 points (35 points per m²; Figure 4b). Higher-detail models, for example 168 captured from an altitude of 3m, yield point clouds of 8.6×10^7 points for an area of 20 m \times 30 169 m (1.4 \times 10⁴ points per m²; Figure 5a).

When the images are georeferenced, aligning and dense point cloud generation are relatively fast processes that can be completed during a fieldwork campaign, providing an almost immediate data check to analyse whether data acquisition was successful or additional flights are required. Furthermore, having already a high-resolution point cloud in the field can be useful in identifying interesting features or sections of the outcrop that warrant further inspection, or additional higher-resolution flights.

176

2.2.3. Surface generation

Interpretation of outcropping features requires converting the point cloud into a meshed 177 178 surface consisting of triangulated elements (Figure 5). The meshed surface can have the same 179 resolution as the dense point cloud, but for sub-horizontal pavements a downsampled mesh is typically sufficient. For this example, the point cloud consists of 8.6×10^7 points whereas the 180 resulting surface is downsampled to 2×10^6 elements (Figure 5b,c). This surface has a 181 182 sufficiently high resolution for 2-D fracture interpretation. After surface generation, the outcrop model can be exported as a 3-D textured geometry to geological modelling software 183 184 or as 2-D georeferenced orthomosaic images to GIS-based software.

185 2.3. Fracture digitisation

Fractures in the 2-D orthomosaic images are digitised in GIS-tools such as DigiFract (Hardebol and Bertotti, 2013). Fracture lengths are manually traced, and attributes including orientation and infill can be assigned. Manual interpretation is time-intensive, but automatic tracking methods are not sufficiently sophisticated to replace manual interpretation, and require time-intensive quality-checking and manual corrections (Duelis Viana et al., 2016; Lin et al., 2015; Vasuki et al., 2014).

192 After digitisation, orientation, spacing and length distributions are calculated (Figure 193 6). Length or height is plotted using frequency distributions (Figure 6c,d). The orientation 194 distribution is visualised using rose diagrams or stereoplots (Figure 6b). Fracture spacing is 195 calculated using a combination of methods, where P_{10} intensity, which is the 1-D fracture 196 density measured along a line (Dershowitz, 1985), is calculated using a large amount of 197 closely-spaced scanlines, which are projected along the entire height or length of an outcrop (Hardebol and Bertotti, 2013). Alternatively, spacing is defined by P_{21} intensity, which is the 198 199 total fracture length versus outcrop area (Dershowitz, 1985), calculated by summing the total 200 length or height of fractures within the entire outcrop or in individual beds as a function of 201 outcrop area (Wu and Pollard, 2002). The P_{21} method is sensitive to boundary effects, as the 202 length of fractures that intersect the boundary cannot be fully quantified, resulting in a 203 potential underestimation of length (Mauldon, 1998; Pahl, 1981; Zhang and Einstein, 1998). Using circular sampling windows and correction methods, this can be compensated for 204 205 (Mauldon et al., 2001), but alternatively fractures that intersect the sampling boundary can be 206 identified and excluded from the length analysis entirely.

207 The full spacing distribution is analysed using box-counting methods (Bonnet et al., 2001), where the outcrop is either divided into boxes with a constant area, in which P_{21} 208 209 intensity is calculated (Figure 6e), or the P_{21} intensity is calculated within a circle with a fixed 210 centre and an increasing radius (Bonnet et al., 2001; Reith, 2015). The latter method also 211 provides a means for characterising the Representative Elementary Area (REA), which is an 212 indication for the optimal grid cell size for upscaled fracture flow modelling (Dershowitz and Doe, 1997; Long and Witherspoon, 1985). These methods help to identify whether the 213 214 digitised networks follow power-law scaling trends (Bonnet et al., 2001; Bour and Davy, 1997; Davy et al., 1990). 215

216 The size and spacing distributions are corrected for censoring artefacts, which result 217 from fractures that are not fully captured in the model such that their true length or height is 218 unknown (Bonnet et al., 2001; Ortega et al., 2006). We correct for this by filtering all 219 fractures that intersect the user-defined outcrop boundaries and by manually identifying and 220 excluding areas obscured by for example trees. Truncation artefacts, which are related to the resolution limit of the outcrop model such that the smallest fractures are typically under-221 222 represented, cannot be corrected automatically. Truncated length and spacing scales need to 223 be identified by the user, based on the image resolution limit.

224

3. Finite Element meshing and stress modelling

The 2-D fracture networks are meshed for mechanical and flow modelling, using unstructured FE meshes with explicit fractures. The meshing and the subsequent geomechanical simulations are done using ABAQUS CAE® (Dassault Systèmes®). Compared to other meshing tools, we find that this tool can handle meshing of more complex geometries, with minimal pre- and postprocessing.

3.1. Meshing of discrete fractures

231 Accurate representation of fracture connectivity and topology is essential, particularly when 232 the matrix is close to impermeable (Figure 7) (Hardebol et al., 2015; Sanderson and Nixon, 233 2015). To ensure that fracture connections and intersections are correct, most GIS tools have 234 manual or automatic snapping options that can be used. The topology is converted to a CAD 235 file that forms the basis for the FE model. To avoid boundary effects, rectangular model 236 boundaries are used with an intact rock buffer zone between the fracture network and model 237 boundary (Figure 8). The model is meshed using quadratic plane strain elements, with refinements along the fractures (Figure 9). To avoid singularity issues, we further refine the 238 239 mesh around fracture tips. To model fracture opening and closing, fractures are represented

as seams in the mesh, using a set of post-processing functions written in Python that update the mesh to generate seams (Figure 10). A seam is generated by duplicating nodes along a seam and splitting the mesh (Figure 11). The individual seams are generated sequentially, taking into account existing seams. Except for the intersecting seams, the output mesh is identical to the input mesh, and can be read directly into the FE simulator for analysis of the stress distribution in a complex fracture network.

246 3.2. Geomechanical FE model set-up

247 Using the ABAQUS Implicit solver, the local stress state is modelled, from which fracture 248 apertures can be derived using stress-aperture relations (Bisdom et al., 2016b). These models 249 take into account a far-field differential stress applied to the boundaries as pressure loads (Figure 8). Maximum and minimum principle stresses are applied perpendicular to the model 250 251 boundaries in a stress initialisation step, during which movement of fracture planes and 252 boundaries is constrained. After successful stress initialisation, the displacement conditions 253 are released to let the model equilibrate (Figure 8), solving for the stress tensor σ in fully 254 elastic rocks (Nick et al., 2011):

255
$$\boldsymbol{\sigma} = \boldsymbol{D}_{C} \left(\boldsymbol{\varepsilon} - \boldsymbol{\varepsilon}_{0} \right) + \boldsymbol{\sigma}_{0}, \qquad (1.3)$$

where ε and ε_0 are the strain and initial strain vectors, σ_0 is the initial stress vector and D_C is the material stiffness matrix:

258
$$D_{c} = \frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1-\nu & \nu & 0\\ \nu & 1-\nu & 0\\ 0 & 0 & 1-2\nu \end{bmatrix}, \qquad (1.4)$$

259 with Young's modulus *E* and Poisson's ratio *v*.

260 The slip tendency of fractures can be defined by a linear Mohr-Coulomb friction 261 coefficient or non-linear behaviour, e.g. Barton-Bandis conductive shearing (Bisdom et al., 262 2016b), which can be defined by functions or look-up tables. A heterogeneous or constant 263 pore pressure distribution can also be taken into account, as well as single-phase flow 264 injection to take into account localised changes in pore pressure over time (Bisdom et al., 265 2016a).

266 *3.3. Aperture modelling*

267 For each fracture node, stress-dependent apertures are calculated from the local normal and 268 shear stresses in the geomechanical FE model (Figure 12). Different stress-aperture relations 269 can be used to calculate the corresponding spatial aperture distribution, capturing small-scale 270 variations along individual fractures (Appendix A; Bisdom et al., 2016d). Aperture 271 definitions that are not a function of stress, such as power-law scaling, are calculated using Python functions in a GIS environment, where aperture is calculated for each fracture 272 273 segment based on the geometrical properties of that segment. Using the x, y-positions, these 274 values are translated from the segments to the nodes of the mesh (Bisdom et al., 2016c, 275 2016d). Four commonly-used aperture definitions have been implemented into the workflow, 276 but other definitions can be easily added. These definitions are (sub-)linear length aperture 277 scaling predicted by LEFM, power-law scaling and Barton-Bandis conductive shearing 278 derived from either FE models or geometrical approximations (Appendix A).

279

4. Flow modelling

To construct the flow model, we use the workflow from (Bisdom et al., 2016b) summarised below. Here, we extend this workflow from calculating only equivalent permeability parallel to the edges of the model to calculating the full permeability tensor to derive the principal maximum and minimum permeability values.

Flow is modelled using the same FE mesh used for the geomechanical models, where the seams in the mesh have been replaced by lower-dimensional elements to which modelled

fracture apertures are assigned (Bisdom et al., 2016b). In addition, the buffer zone added to avoid stress boundary effects is removed such that fractures intersect the edges of the model. We use a hybrid Finite-Element Finite-Volume (FE-FV) approach implemented in the Complex Systems Modelling Platform to solve the flow equations (Matthäi et al., 2007).

Similar to Durlofsky (1991), the full equivalent permeability tensor (**k**) is computed by solving the steady state continuity equation for flow in different directions using a far-field pressure gradient applied in both horizontal directions of the rectangular 2-D pavements. Note that the superscripts 1 and 2 are used for these two problems in Eq. (1.5) and a constant viscosity (μ) is assumed. This is followed by volume-averaging (for both problems $\langle \rangle^1$, $\langle \rangle^2$) of resulted fluid velocities (u_x and u_y) and pressure gradients (∇p_x , ∇p_y) to solve for equivalent tensor permeability through:

$$297 \qquad \langle \nabla p \rangle \mathbf{k} = -\mu \langle u \rangle \Rightarrow \begin{bmatrix} \langle \nabla p_x \rangle^1 & \langle \nabla p_y \rangle^1 & 0 & 0 \\ 0 & 0 & \langle \nabla p_x \rangle^1 & \langle \nabla p_y \rangle^1 \\ \langle \nabla p_x \rangle^2 & \langle \nabla p_y \rangle^2 & 0 & 0 \\ 0 & 0 & \langle \nabla p_x \rangle^2 & \langle \nabla p_y \rangle^2 \end{bmatrix} \begin{bmatrix} k_{xx} \\ k_{xy} \\ k_{yx} \\ k_{yy} \end{bmatrix} = -\mu \begin{bmatrix} \langle u_x \rangle^1 \\ \langle u_y \rangle^1 \\ \langle u_x \rangle^2 \\ \langle u_y \rangle^2 \\ 0 \end{bmatrix}. \quad (1.5)$$

298 The maximum and minimum principal permeability values (k_{max}, k_{min}) as well as the 299 principal direction (θ) can be calculated.

5. Application

The integrated workflow is applied to model permeability through an outcropping network of fractures in the Jandaíra Formation, which is a carbonate formation that crops out in large parts of the Potiguar Basin in NE Brazil. Extensive fracture networks were formed predominantly during burial in a compressional setting (Bertotti et al., **accepted**; de Graaf et 305 al., 2017). The sub-horizontal position of the rocks provides excellent exposures of multiscale 306 fracture patterns covering areas of several hundred by several hundred meters, which is 307 comparable to the spacing of wells in a fractured reservoir (Bisdom et al., accepted). In 308 conventional reservoir models, these areas are typically populated with stochastic fractures 309 whose distributions are derived from well data or small outcrops. We use our workflow to 310 capture and create a deterministic discrete fracture flow model, focusing on a rectangular area 311 of a pavement in the western part of the basin in which there is a minimal impact of censoring caused by a few trees (Figure 13). This study area is ideal for 2-D geomechanical and flow 312 313 analysis, as all fractures dip perpendicular to the sub-horizontal bedding planes (Bisdom et 314 al., accepted). Hence, spacings and lengths interpreted on the pavements do not require any 315 Terzaghi corrections, and the complete orientation distribution can be quantified by digitising 316 fracture strikes. Geometrical analysis of fractures in the Potiguar basin is outside the scope of this work, but presented elsewhere (Bertotti et al., accepted; Bisdom et al., accepted; de 317 318 Graaf et al., 2017).

319

5.1. Fracture network geometry

320 The area of interest was imaged with the UAV at an altitude of 50 m above the ground, 321 resulting in 90 images with a resolution of 1.44 cm/px. The model was accurately 322 georeferenced using several GCPs, for which we measured the absolute position and the distance between the GCPs. The resulting dense point cloud covers an area of 4.1×10^4 m² 323 with a point density of 284 m⁻¹. The georeferenced orthomosaic has the same resolution as 324 the individual images (1.44 cm/px; Figure 13). Using DigiFract, we digitised 1082 fractures 325 326 in a rectangular area of $150 \text{ m} \times 142 \text{ m}$ (Figure 14b). Three orientation families were 327 identified with size and spacing distributions that follow power-law scaling trends (Bisdom et 328 al., accepted). Weathering has affected apertures and limits the view of the smallest fracture 329 scales (i.e. smaller than 1 m), but these smaller length scales have only a relatively small

impact on permeability compared to the large connected system of fractures. Recentweathering also created dendritic dissolution patterns, which are excluded from the analysis.

332 5.2. *Fracture aperture distribution*

333 Most fractures are open and free from cement, i.e. barren, but this is associated with 334 exhumation and weathering (Bertotti et al., accepted). To define apertures representative of 335 subsurface conditions, we use a sublinear length-aperture scaling model defined by Linear 336 Elastic Fracture Mechanics (LEFM; Atkinson, 1984; Bisdom et al., 2016c; Lawn and 337 Wilshaw, 1975; Olson, 2003). The far-field stress is defined by a 30 MPa σ_1 applied as 338 pressure loads perpendicular to the north and south boundaries and a σ_3 of 10 MPa oriented 339 E-W. These stress directions are comparable to the paleostress directions under which most 340 of the fractures were formed (Bertotti et al., accepted). In the absence of measurements of the 341 elastic rock properties, the rock is assumed fully elastic with a Poisson's ratio of 0.3 and a Young's modulus of 50 GPa. The model mesh consists of 5.1×10^5 triangulated elements 342 343 (Figure 14a).

The resulting aperture scales with length and stress (Figure 14b). Aperture follows a lognormal distribution with a maximum of 2.5 mm and an average of 0.5 mm. One percent of fractures is hydraulically closed, but the majority of the network is permeable (Figure 14c,d).

347

5.3. Equivalent permeability

For a 1 mD matrix, the pressure gradient in the x- and y-directions is highly heterogeneous, particularly in the x-direction (Figure 14c,d). We quantify permeability as the ratio between equivalent and matrix permeability, which quantifies the contrast between matrix and fractures (Figure 15). The ratio is high for a low matrix permeability, as most flow is carried by the fractures, and decreases for increasing matrix permeability. For a low permeability matrix (1 mD), fracture flow in the *y*-direction is more dominant than the *x*-direction (Figure 15a,b), but remarkably this is reversed for higher matrix permeabilities (Figure 15c,d).

This reversal is better quantified using the fluid velocities, which show that one large fracture percolates through the entire model in the *y*-direction, creating a flow pathway even when matrix permeability is low (Figure 15a,b). There are several large E-W striking fractures with large apertures, but they do not fully percolate the model, limiting their impact in a low-permeability matrix.

360 This change in permeability is better explained by calculating the full permeability 361 tensor (Figure 16). For matrix permeabilities below 100 mD, maximum permeability is in a 362 NE-SW direction. In this domain, permeability is controlled by fracture flow. For increasing 363 matrix permeability, flow is carried by a mix of fractures and matrix, and the maximum permeability rotates to ENE-WSW, remaining anisotropic. Only when matrix permeability is 364 365 larger than several Darcy, flow is fully carried by the matrix and permeability becomes isotropic. However, for most models, the highly-connected high-intensity fracture network 366 367 controls flow either completely or partly.

368 **6. Discussion**

369 6.1. From outcrops to representative subsurface flow models

Contrary to other studies, the presented workflow uses only the outcropping network geometry as input for deterministic flow models, not taking into account outcropping apertures. Instead, we use geomechanical FE models to solve the stress state around the fracture network, based on estimates of subsurface stress conditions and rock properties. These geomechanical parameters can typically be derived from subsurface datasets, albeit with uncertainty ranges. However, the applied methodology is fast, allowing the inclusion of uncertainty ranges. The resulting stress states are used to calculate aperture, using different

377 stress-aperture relations (Bisdom et al., 2016d). This combination of outcropping geometries 378 and subsurface stress states and aperture distributions results in models that are more 379 representative of fractured reservoir permeability compared to analogue studies that use 380 apertures of exhumed barren fractures or assume a constant aperture for the entire network 381 (Makedonska et al., 2016).

382 Laser scanning of outcrops is an alternative method that provides deterministic 383 representations of entire outcrops, but photogrammetry offers more flexibility (Hodgetts, 384 2013). Through the use of deterministic 2-D patterns rather than stochastic fracture networks 385 derived from 1-D distributions, more realistic estimates of permeability can be made. Stochastic models typically contain mutually crosscutting networks of fractures resulting in 386 387 highly connected networks with consistently high permeabilities, which does not correspond 388 to observations of permeability heterogeneity typically observed in fractured reservoirs. The 389 studied deterministic pattern better represents natural fracture topology with terminating 390 rather than fully crosscutting fractures (Hardebol et al., 2015; Sanderson and Nixon, 2015).

391 6.2. Lessons for reservoir-scale flow modelling

392 The studied fracture network from the Potiguar basin contains predominantly N-S and E-W 393 striking fractures that form an orthogonal pattern. Orthogonal patterns are observed in many 394 fractured outcrops and are assumed to be present in many subsurface reservoirs (Bai et al., 395 2002). In reservoir-scale flow models, these patterns are upscaled to effective properties in 396 the two dominant fracture directions that are assumed to be representative of fracture 397 permeability, comparable to the equivalent permeability in the x- and y-directions. However, 398 by only considering flow in two directions, permeability is underestimated in this example, as for a 1 mD matrix permeability, the permeability ratio in x- and y-directions is 1.9×10^3 and 399 2.2×10^3 respectively, whereas the maximum ratio is 3.9×10^3 in the NE-SW direction. Even 400 401 for these relatively homogeneous orthogonal networks, the absolute maximum permeability

402 cannot be accurately determined using conventional upscaling. The outcrop-derived 2-D 403 permeability tensor models can be used to determine the principal permeabilities of discrete 404 fracture networks covering several grid cells, as a more accurate alternative to quantify 405 permeability compared to ODA upscaling (Oda, 1985). These flow models also help to 406 identify different fracture-matrix permeability domains, which can be used to better 407 characterise fractured reservoir flow domains. To further bridge the gap between discrete fracture models and reservoir-scale continuum models, hybrid upscaling techniques can be 408 409 used (Egya et al., 2016; Shah et al., 2016).

410

7. Conclusions

411 Outcrops provide a wealth of data for studying and modelling of fracture networks, which 412 cannot be fully captured with 1-D scanlines, as these only capture spacing and aperture of one 413 orientation set. LiDAR on the other hand captures entire outcrops at a high resolution, but 414 this method has limited flexibility in terms of the type of outcrops it can be applied to and in 415 terms of processing (Hodgetts, 2013). The presented workflow enables fast generation of 416 highly detailed realistic fracture networks for use of geomechanical and flow modelling, 417 variations of which have been applied to study different aspects of fracture and fracture-418 matrix flow (e.g. Aljuboori et al., 2015; Arnold et al., 2016; Bisdom et al., 2016c; Egya et al., 419 2016; Muhammad, 2016; Shah et al., 2016).

We use fracture patterns derived from these models for geomechanical and flow modelling of discrete fractures on a scale that is representative of part of a fractured reservoir, spanning an area of several conventional reservoir grid cells. The geomechanical model solves the local stress state within the fracture network, which is used to model aperture using a range of stress-aperture relations. The flow models consider matrix and fracture flow. Although the fluid pressure in the *x*- and *y*-directions of these models can be used to quantify

relative permeability differences between different models, it is not representative of the true principal permeabilities, even in an orthogonal network where fractures strike mainly parallel to the x- and y-directions. The presented workflow allows for fast quantification of the full permeability tensor in domains covering several conventional simulator grid cells using realistic fracture patterns digitised from outcrops, with minimal pre-processing and no upscaling.

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718 Appendix A – Summary of aperture methods

719 (Sub-)linear length-aperture scaling

Linear Elastic Fracture Mechanics (LEFM) predicts that aperture scales (sub-)linearly with length during propagation of fractures (Atkinson, 1984; Olson, 2003; Pollard and Segall, 1987). Maximum opening at the centre of the fracture, d_{max} , is defined by fracture toughness K_C, the Poisson's ratio v, Young's modulus E and fracture length L:

724
$$d_{\max} = \frac{K_C \left(1 - v^2\right)}{E \sqrt{\frac{\pi}{8}}} \sqrt{L},$$

725 where K_C is a function of driving stress $\Delta \sigma_I$ and fracture length:

726
$$K_c = \Delta \sigma_I \sqrt{\pi L/2}$$
.

Discussion remains on whether aperture scales linear or sublinear with length, which has large implications for apertures of relatively large fractures (Olson, 2003; Olson and Schultz, 2011; Scholz, 2011; Vermilye and Scholz, 1995). The impact of linear versus sublinear scaling on permeability is investigated in (Bisdom et al., 2016d).

731 *Power-law scaling*

Outcrop studies typically find that fracture lengths follow power-law scaling distributions (e.g. Bonnet et al., 2001; Bour and Davy, 1997). This, combined with the linear lengthaperture scaling model, implies that aperture also follows power-law scaling relations. Power-law scaling of apertures is commonly observed in outcrops (Hooker et al., 2014, 2009; Ortega et al., 2006), although the relation with length is rarely studied in outcrops. Instead, the aperture distributions are defined independent of other geometrical or geomechanical parameters, through a power-law function:

$$739 \qquad F = aX^{-b},$$

where F is the cumulative frequency, a is a density constant and b is the power-law scaling exponent (Bonnet et al., 2001). Power-law aperture distributions are the preferred method of defining aperture in industry fractured reservoir models, but they are rarely related to any other geometrical parameter. As a result, short fractures may be assigned unrealistically large apertures (Bisdom et al., 2016d).

745

Barton-Bandis

Whereas (sub-)linear length-aperture scaling predicts opening during propagation over geological times, Barton-Bandis describes present-day opening in the current stress field, assuming that fractures have irregular walls that result in conductive fractures when shearing occurs, even when fluid pressures are low (Bandis, 1980; Bandis et al., 1983; Barton, 1982; Barton and Bandis, 1980; Barton and Choubey, 1977). Mechanical aperture E_n is a function of an intrinsic initial aperture E_0 , maximum closure v_m , toughness K_{ni} and driving stress $\Delta \sigma_I$ (e.g. Barton, 2014).

753
$$E_n = E_0 - \left(\frac{1}{v_m} + \frac{K_{ni}}{\Delta \sigma_I}\right)^{-1}.$$

Hydraulic aperture *e* is a function of mechanical aperture, the Joint Roughness Coefficient (JRC) and the ratio between shear (u_{geom}) and peak shear (u_{peak}) displacement (Olsson and Barton, 2001).

757
$$e = \begin{cases} \frac{E_n^2}{JRC^{2.5}} & \text{for } \frac{u_{geom}}{u_{peak}} \le 0.75\\ \sqrt{E_n} JRC_{mob} & \text{for } \frac{u_{geom}}{u_{peak}} \ge 1 \end{cases}$$

This aperture model is strongly dependent on the local normal and shear stress acting on each fracture segment, which is most accurately determined using geomechanical Finite-Element models with discrete fractures (Bisdom et al., 2016b; Lei et al., 2014).

Alternatively, an approximation of Barton-Bandis apertures can be made without use of numerical models. Using a far field stress and fracture geometry, aperture can be approximated (Bisdom et al., 2016c). This model is strongly dependent on stress angle α between fracture strike and σ_1 :

765 $\sigma_{n,\text{angle}} = 0.0054\sigma_1 \alpha + 1.5186\sigma_1^{0.723}$.

766 Normal stress is further corrected for length L and spacing S:

767 $\sigma_{n,geom} = \sigma_{n,angle} (-0.083 \ln L + 1.055) \cdot 0.996 \cdot S^{-0.008}$.

768 Shear displacement is also defined as a function of length and stress angle:

769 $u_{scom} = L \cdot \sigma_1 \cdot \alpha \left(-9.07 \cdot 10^{-8} \alpha + 8.1 \cdot 10^{-6} \right).$

770 Figures



Figure 1 Workflow for obtaining flow-based principal permeability from outcropping
fracture networks that are representative of subsurface reservoir stress and pressure
conditions, by taking into account the impact of stress on aperture and flow. See Appendix A
for details on the aperture models.



Figure 2 Overview of the UAV equipment in the field: a) UAV (microdrones md4-200) with
compact camera; b) Base station with receivers and tablet to receive and store flight data
and video; c) Preparation for UAV deployment in the field. White case is the UAV transport
case; d) Use of the UAV to image carbonate pavements.



Figure 3 Programming and visualising GNSS-steered flights: a) Top view of flight path (red)
and imaged area (blue) in Google Earth; b) 3-D of the flight path, allowing for checking the
programmed flight altitude with respect to ground level.



Figure 4 Generating a 3-D outcrop model from georeferenced photographs using
photogrammetry: a) Alignment of the images (rectangles) based on their GNSS position and
common points extracted from the images. The image name is shown for each image (small
texts); b) 3-D high-resolution point cloud of the outcrop.





Figure 5 Constructing a 3-D meshed surface from the point cloud: a) Detail view of the dense
point cloud with a resolution of 1.4x10⁴ points per m², showing a compass and pen for scale;
b) Triangular mesh constructed for the area from (a). The mesh has a lower resolution than
the point cloud, but does still indicate the main discontinuities, such as the fracture within the
red dashed area; c) Texture extracted from the original photographs, projected onto the
mesh.



Figure 6 Fracture digitisation and analysis in DigiFract using a 2-D orthomosaic of a carbonate pavement, constructed from 400 photographs taken from an altitude of 50 m, resulting in a orthomosaic resolution of 1.44 cm/px: a) The orthomosaic used for fracture digitisation; b) The digitised fracture network; c) Fracture length distribution; d) Fracture spacing distribution calculated using a box-counting method; e) Spatial fracture intensity calculated using box-counting.



805 Figure 7 Removal of minor gaps and overlaps to accurately represent the network topology:
806 a) Three small fractures terminating against one larger fracture, with incorrect connections;

807 b) Detail showing one overlapping segment and one segment with a gap; c) Correct fracture

808 *network interpretation using snapping.*



810 Figure 8 Set-up of an elastic mechanical fracture network, using a 50 m × 50 m fracture 811 pattern from a carbonate outcrop in central Tunisia (Bisdom et al., 2016b). Maximum 812 horizontal stress σ_1 (30 MPa) is applied in the y-direction, resulting in a σ_3 of 10 MPa in the 813 x-direction, for a Poisson's ratio of 0.3. We apply displacement boundary conditions on the 814 centre points of each boundary to ensure symmetrical deformation.



Figure 9 Converting a deterministic fracture network into a triangular mesh: a) 150 m × 142
m section extracted from an outcropping carbonate pavement in the Potiguar basin (NE
Brazil); b) Meshed fracture network geometry; c) detail of the mesh showing refinement
around the fracture terminations and intersections (location indicated by the white square in
(b).



Figure 10 Aperture in the FE models is modelled by representing the fractures as seams in
the mesh, which can open or close as a function of local stress: a) Input mesh with two

- 824 fractures indicated by the thick lines; b) Result after simulation, showing the stresses in the
- 825 *mesh and the resulting fracture opening along the seams.*



Figure 11 Example illustrating the process of generating complex fracture intersections for three fractures that share a single intersection. The original node at this intersection, with identifier 1, is duplicated several times to generate the intersecting seams: a) Original mesh with fractures indicated by bold lines; b) Generating the first fracture by splitting nodes – node 1 is duplicated to 101; c) Second fracture requires duplication of both node 1 and 101 (1 \rightarrow 201, 101 \rightarrow 202); d) The third intersecting fracture requires duplication of the last two nodes formed (201 \rightarrow 301, 202 \rightarrow 302).



835 Figure 12 Stress-induced aperture modelling: a) Mechanical aperture defined by Barton-836 Bandis, calculated from normal and shear stresses acting on each fracture segment, under a 837 N-S regional σ_1 of 30 MPa and an E-W σ_3 of 10 MPa; b) Identification of hydraulically 838 conductive fractures using the Barton-Bandis model, for the same stress boundary 839 conditions.



841 Figure 13 High-resolution orthomosaic (1.44 cm/px resolution) of part of an outcrop in the
842 Potiguar basin (lat/long: -5.53092°, -37.6283°) constructed from 90 georeferenced images.

843 The white boundary indicates the domain that is considered for stress and flow modelling.



Figure 14 Aperture and permeability results: a) 2-D mesh with 5.1×10^5 triangular matrix elements and 3.3×10^4 linear fracture elements; b) Aperture distribution derived from the local stress state assuming sublinear length-aperture scaling relations with a maximum horizontal stress oriented in the y-direction; c) Pressure gradient in the x-direction (indicated by arrow) for a 1 mD matrix permeability; d) Fluid pressure in the y-direction.



851 Figure 15 Fluid velocity magnitudes under far-field pressure gradient in the x-direction (a, c,

e) and y-direction (b, d, f) for different matrix permeabilities.



854

855 Figure 16 Maximum permeability versus matrix permeability for a range of matrix 856 permeabilities. The direction of maximum permeability is indicated by the ellipses and θ , 857 measured from the East. The ratio between minimum and maximum permeability remains 858 relatively constant except for Darcy-scale flow, where permeability is completely controlled 859 by matrix flow.